

Z



NOTE: IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO CONTACT ALL RELEVANT SERVICE AUTHORITIES TO VERIFY THE LOCATION OF ALL EXISTING SERVICES PRIOR TO START OF CONSTRUCTION.

RP DESCRIPTION	ULDA REF:	PSM
LOTS: 22 ON SP709373 505 ON RP88A348 504 ON PR35A348	DEU2010/710 DEU2010/701	132061 BL 9.252 BELLVISTA BLVD GATON, AND

DATE ISSUED	7/11/68
DRAWING INDEX & LOCALITY PLAN	
PROJECT NUMBER	NH026-100
SECTION	

PLANS AND DOCUMENTS
referred to in the ULDA
APPROVAL dated 26 / 9 / 11



DATE: 10/10/10 DRAWN: 10/10/10 CHECKED: 10/10/10 APPROVED: 10/10/10		PROJECT NO: N11028		CLIENT: STOCKLAND CALOUNDRRA DOWNS P.L.		CONSULTANT: BROWN		PROJECT TITLE: HAUL ROAD SITE PLAN	
SCALE: 1:5000 (UNREduced)		PROJECT NO: N11028		CLIENT: STOCKLAND CALOUNDRRA DOWNS P.L.		CONSULTANT: BROWN		PROJECT TITLE: HAUL ROAD SITE PLAN	
1:5000 (UNREduced)		PROJECT NO: N11028		CLIENT: STOCKLAND CALOUNDRRA DOWNS P.L.		CONSULTANT: BROWN		PROJECT TITLE: HAUL ROAD SITE PLAN	

PLANS AND DOCUMENTS
 referred to in the ULDA
 APPROVAL dated 16/1/11

NOTE: FINAL ALIGNMENT OF PRIMARY HAUL ROAD TO BE CONFERRED ON SITE PRIOR TO CONSTRUCTION TO SUIT CONTRACTORS REQUIREMENTS. FINAL LOCATION TO UTILISE EXISTING HAUL TRACKS AND EXISTING CLEARED AREAS TO MINIMISE POTENTIAL DAMAGE TO EXISTING VEGETATION.

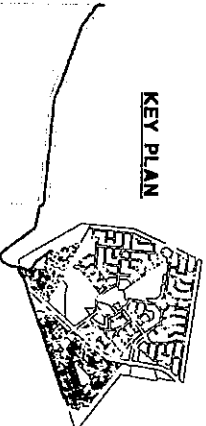
PRIMARY HAUL ROAD SETOUT TABLE

EXISTING HAIL TRACK					EXISTING HAIL TRACK TO BE REPERATED				
PT	CHAINAGE	EASTING	NORTHING	BEARING	RAD/SPR	TANGENT	DEF. ANGLE	ARC LEN	
IP1	0.000	6215.860	35655.852	181.411.57°	50.000	28.927	60.006.98°	52.449	
IP2	26.889	6212.474	35625.933	197.016.37°	50.000	28.927	60.006.98°	52.449	
IP3	53.858	6208.824	35601.231	197.016.37°	50.000	28.927	60.006.98°	52.449	
IP4	79.282	6203.881	35583.769	127.608.49°	50.000	22.716	17.013.21°	45.088	
IP5	296.411	6405.089	35452.550	109.055.28°	50.000	22.716	17.013.21°	45.088	
IP6	318.956	6462.445	35431.813	109.055.28°	50.000	22.716	17.013.21°	45.088	
IP7	357.992	6462.299	35373.622	109.055.28°	50.000	22.716	17.013.21°	45.088	
IP8	671.571	6716.824	35323.551	126.057.50°	50.000	22.716	17.013.21°	45.088	
IP9	671.572	6716.956	35323.557	126.057.50°	50.000	22.716	17.013.21°	45.088	
IP10	829.753	6732.753	35268.495	106.059.50°	50.000	22.716	17.013.21°	45.088	
IP11	829.753	6732.753	35268.495	106.059.50°	50.000	22.716	17.013.21°	45.088	
IP12	862.946	6942.541	35257.238	113.010.18°	50.000	22.716	17.013.21°	45.088	
IP13	7072.050	35199.341	113.010.18°	159.000	22.716	17.013.21°	45.088		
IP14	7072.050	35199.341	113.010.18°	159.000	22.716	17.013.21°	45.088		
IP15	7072.050	35199.341	113.010.18°	159.000	22.716	17.013.21°	45.088		
IP16	7072.050	35199.341	113.010.18°	159.000	22.716	17.013.21°	45.088		
IP17	7072.050	35199.341	113.010.18°	159.000	22.716	17.013.21°	45.088		
IP18	7072.050	35199.341	113.010.18°	159.000	22.716	17.013.21°	45.088		
IP19	7072.050	35199.341	113.010.18°	159.000	22.716	17.013.21°	45.088		
IP20	7072.050	35199.341	113.010.18°	159.000	22.716	17.013.21°	45.088		
IP21	7072.050	35199.341	113.010.18°	159.000	22.716	17.013.21°	45.088		

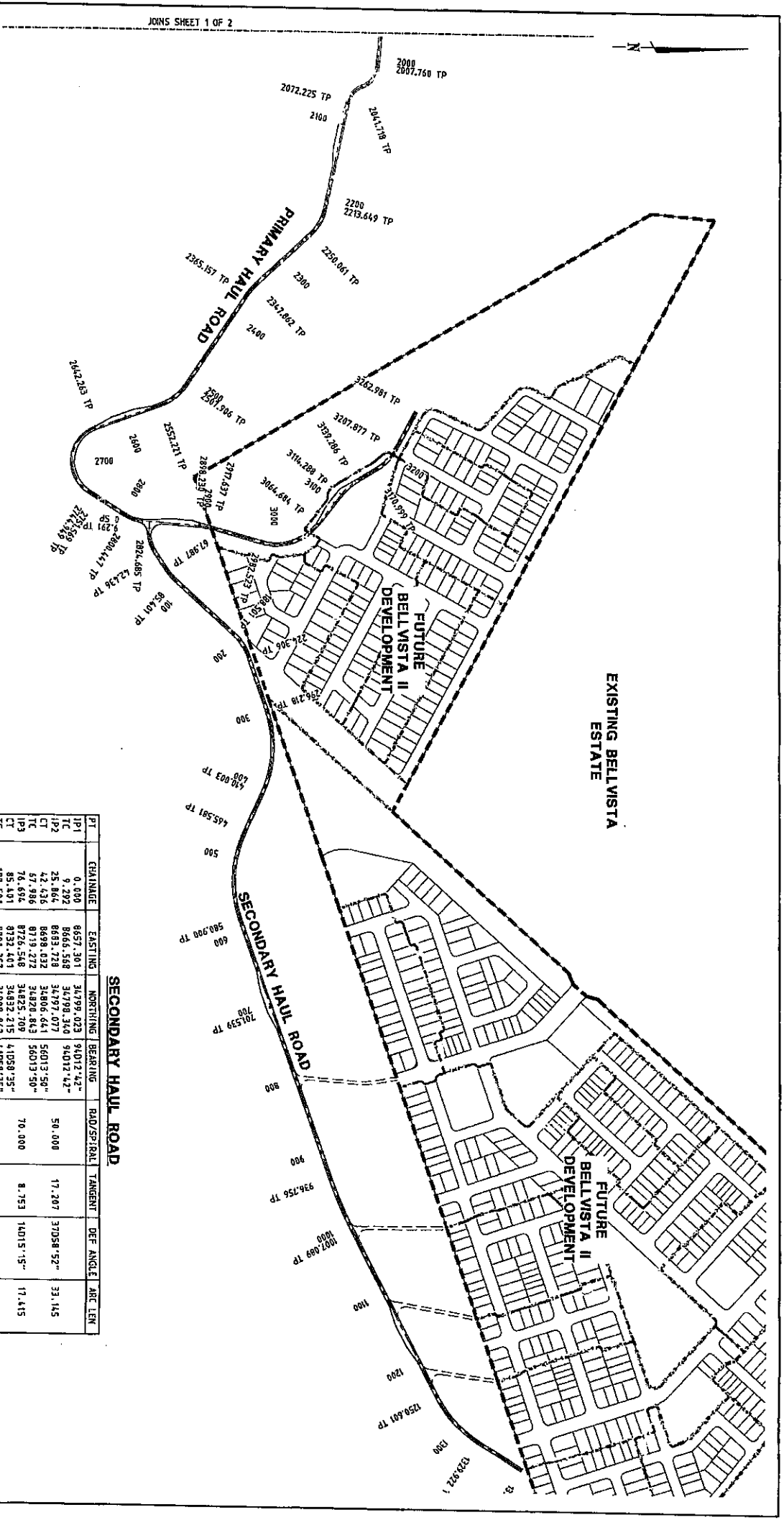
PRIMARY HAUL ROAD SETOUT TABLE (CONTINUED)

PT	CHAINAGE	EASTING	NORTHING	BEARING	RAD/SPR	TANGENT	DEF. ANGLE	ARC LEN
IP21	2713.630	8249.718	35022.556	102.014.16°	50.000	18.786	34.066.13°	36.411
IP22	2231.835	8266.077	35008.853	137.000.30°	50.000	18.786	34.066.13°	36.411
IP23	2250.041	8347.897	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP24	2346.312	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP25	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP26	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP27	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP28	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP29	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP30	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP31	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP32	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP33	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP34	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP35	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP36	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP37	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP38	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP39	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP40	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP41	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP42	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP43	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP44	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP45	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP46	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP47	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP48	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP49	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP50	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP51	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP52	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP53	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP54	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP55	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP56	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP57	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP58	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP59	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP60	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP61	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP62	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP63	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP64	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP65	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP66	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP67	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP68	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP69	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP70	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP71	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP72	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP73	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP74	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP75	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP76	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP77	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP78	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP79	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP80	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP81	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP82	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP83	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP84	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP85	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP86	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP87	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP88	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP89	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP90	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP91	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP92	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP93	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP94	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP95	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP96	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP97	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP98	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP99	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411
IP100	2356.492	8353.581	34936.973	137.000.30°	50.000	18.786	34.066.13°	36.411

KEY PLAN



PROJECT NAME		PROJECT NO.		PROJECT DATE	
STOCKLAND CALOUNDRRA DOWNS P.L.		N1026		1/10/2010	
PROJECT LOCATION		PROJECT SCALE		PROJECT STATUS	
BELLARINA II HAUL ROAD		1:1000		PROPOSED	
PROJECT CLIENT		PROJECT ENGINEER		PROJECT CHECKER	
BROWN ENGINEERING		BROWN ENGINEERING		BROWN ENGINEERING	
BROWN ENGINEERING		BROWN ENGINEERING		BROWN ENGINEERING	
BROWN ENGINEERING</					



LEGEND

ROAD CONTROL LINE

EXISTING TRUCK TO BE UPGRADED TO CONTRACTOR'S REQUIREMENTS

NEW HAUL ROAD

INDICATIVE FUTURE CONSTRUCTION ACCESS

PROPOSED TEMPORARY BRIDGE CROSSING

SECONDARY HAUL ROAD

PT	CHAINAGE	EASTING	NORTHING	BEARING	RAD/SP/IRAIL	TANGENT	DEF ANGLE	ARC LEN
PI1	0.000	8657.301	34799.023	94.012° 42"				
TC1	9.292	8666.568	34798.340	94.012° 42"				
IP2	25.864	8683.728	34797.077		50.000	17.207	37058° 52"	33.145
CT1	42.436	8698.632	34806.641	56.013° 50"				
TC2	67.986	8719.272	34820.843	56.013° 50"	70.000	8.753	14.015° 15"	17.415
IP3	76.694	8726.568	34825.709					
CT2	85.401	8732.401	34832.215	4.1058° 35"				
TC3	188.503	8801.357	34906.862	4.1058° 35"	70.000	18.303	28.018° 25"	35.805
IP4	206.568	8813.559	34922.469					
CT3	224.306	8830.324	34928.342	7.1016° 40"				
TC4	238.118	8852.443	34970.403	7.1016° 40"	150.000	59.787	43.027° 45"	113.785
IP5	323.111	8952.663	34945.577	11.4044° 45"				
CT4	410.003	9009.964	34945.577	11.4044° 45"	150.000	66.678	44.002° 54"	115.310
TC5	465.581	9060.461	34896.913					
IP6	523.241	9115.547	34916.970	7.0041° 51"				
CT5	580.808	9172.813	34916.970	7.0041° 51"	500.000	35.224	8.003° 34"	78.332
TC6	936.756	9508.666	35034.601	7.0041° 51"				
IP7	971.923	9541.910	35045.245					
CT6	1007.089	9573.193	35062.434	62.038° 16"				
TC7	1250.661	9789.461	35174.355	62.038° 16"	150.000	42.611	30.017° 54"	78.321
IP8	1289.261	9825.529	35193.021					
CT7	1339.722	9861.723	35227.333	32.020° 73"				
IP9	1396.247	9885.003	35285.060					

SCALE: 1:2500

DATE: 15/05/2011

PROJECT NO: M1026

DESIGNED BY: [Signature]

CHECKED BY: [Signature]

APPROVED BY: [Signature]

STOCKLAND CALOUNDRIA DOWNS P.L.

BELVISTA II HAUL ROAD

BROWN

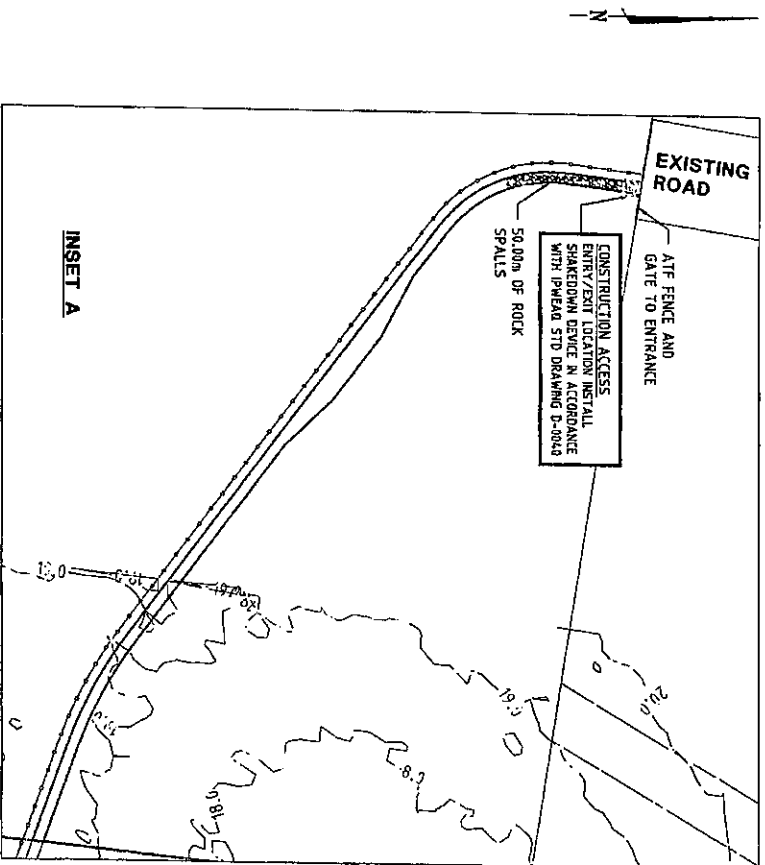
CONSULTANTS

HAUL ROAD SURVEY SETOUT PLAN SHEET 2 OF 2

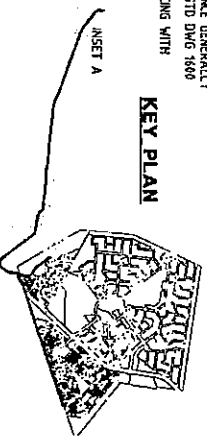
PLANS AND DOCUMENTS

referred to in the ULDA

APPROVAL dated 16/1/11



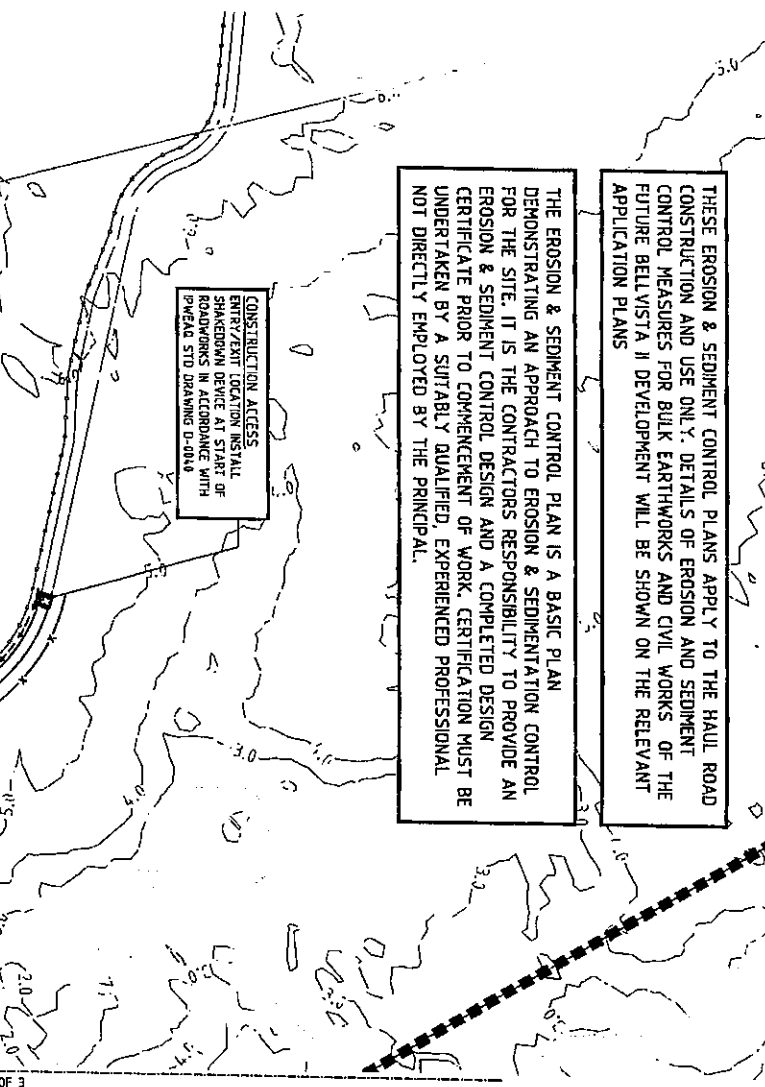
- LEGEND**
- PROPOSED STAGE BOUNDARY
 - DUTTY WATER DIVERSION DRAIN
 - NATURAL CONTOURS (1000m INTERVALS)
 - PROPOSED SEDIMENT FENCE
 - FUTURE OPEN DRAINAGE CHANNEL
 - RCP CULVERT CROSSING & HEADWALLS
 - INDICATIVE EXISTING CREEK ALIGNMENT
 - 100mm CHIP SEAL (ALL AREAS BELOW D10 FLOOD LEVEL OF 3.15m)
 - ROCK SPALLS (50m EITHER SIDE OF CHIP SEAL)
 - PROPOSED TEMPORARY BRIDGE CROSSING
 - 3 STRAND BARBED WIRE FENCE GENERALLY IN ACCORDANCE WITH DWR STD DWG 1460
 - ORANGE BARRIER MESH FENCING WITH "NO GO AREA" SIGNAGE



TITLE: CONCEPTUAL EROSION AND SEDIMENT CONTROL PLANS DATE: 11/15/2011 DRAWN BY: [Signature] CHECKED BY: [Signature] APPROVED BY: [Signature]	
PROJECT NO.: N11026-104 SHEET NO.: 1 OF 3	CLIENT: STOCKLAND CALONDRADA DOWNS P.L. PROJECT: BELLYSTA II HAUL ROAD
PREPARED BY: BROWN CONSULTANTS PHONE: 07 5436 7888 FAX: 07 5436 6530 EMAIL: info@brownconsultants.com.au	PROJECT NO.: N11026-104 SHEET NO.: 1 OF 3

CONCEPTUAL SEDIMENT MANAGEMENT PROGRAM PHASE 1

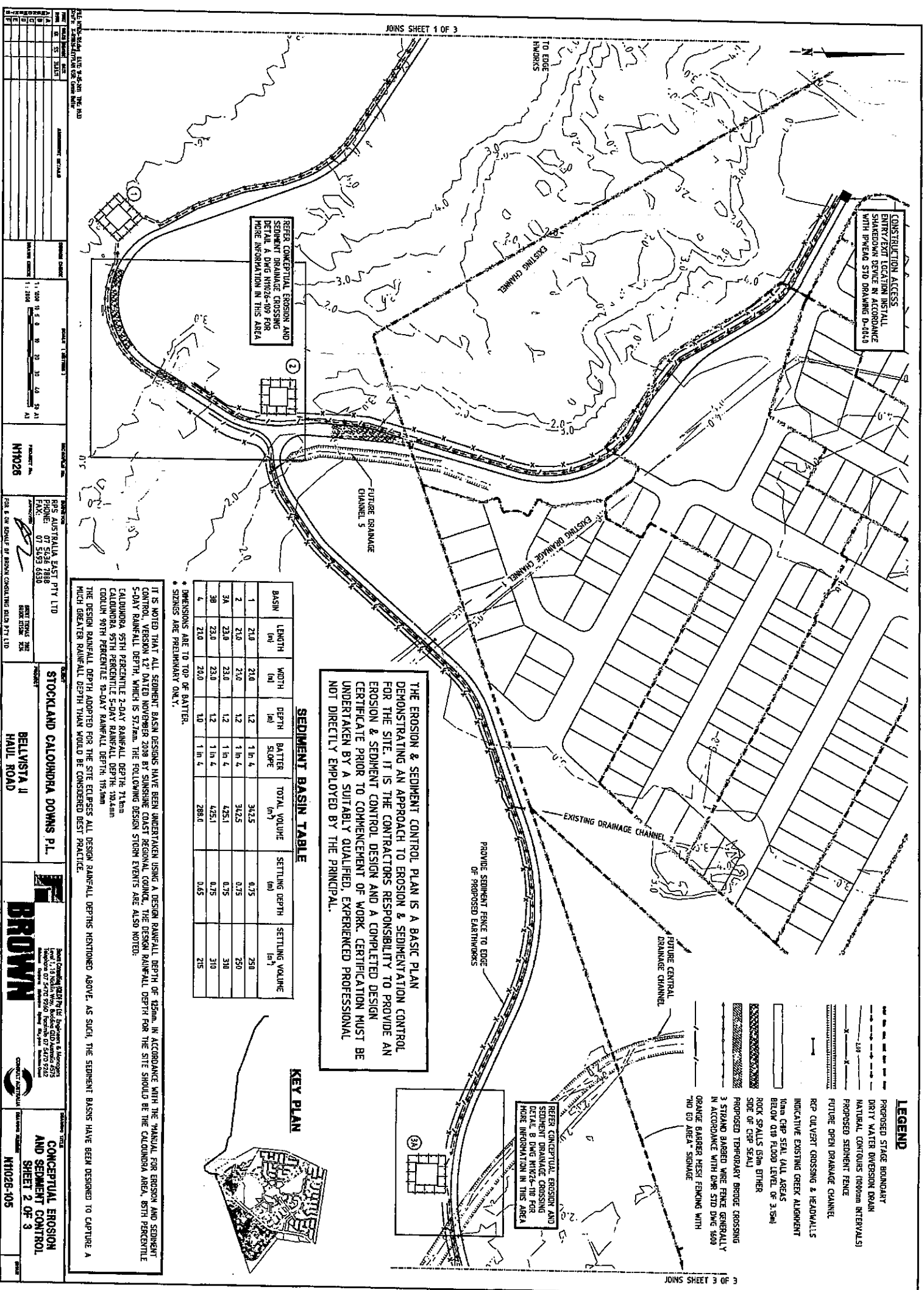
- DESCRIPTION OF WORKS:**
- PRIOR TO CONSTRUCTION
 - SEDIMENT FENCE, SAND BAGS AND EARTH BILLS TO BE ERECTED AS DIRECTED.
 - CONTRACTOR TO DISPLAY APPROPRIATE WORKPLACE HEALTH AND SAFETY SIGNAGE DETAILING THE AREA BEYOND IS A CONSTRUCTION SITE AND ONLY AUTHORIZED PERSONNEL CAN ENTER.
 - CONSTRUCT ENTRY AND EXIT POINT AS INDICATED.
 - CONSTRUCT VEHICLE WASHDOWN AREA AND ASSOCIATED SEDIMENT MANAGEMENT DEVICES.
 - CONSTRUCT SITE OFFICE AND STORAGE COMPOUND AREA.
 - SEDIMENT BASINS TO BE CONSTRUCTED.
 - CUTOFF DRAINS (CLEAN AND DIRTY) TO BE CONSTRUCTED.
 - CLEANING
 - EXISTING GRASSED VEGETATED AREAS TO BE RETAINED WHERE POSSIBLE AND EXPOSED DISTURBED AREA MINIMIZED.
 - EARTHWORKS
 - SUPERINTENDENT TO CONFIRM EXTENT OF CLEARING/EARTHWORKS TO CONTRACTOR PRIOR TO COMMENCEMENT OF WORKS.
 - EFFECTIVELY STABILISE ALLOTMENTS PROGRESSIVELY, AS REQUIRED.
 - SEDIMENT FENCES TO BE RE-ERECTED TO ALLOTMENTS WHEN REQUIRED. ANY STOCKPILES, TEMPORARY STOCKPILES AND EARTHWORKS NOT INTENDED TO BE WORKED FOR MORE THAN 10 DAYS ARE TO BE EFFECTIVELY STABILISED.
 - EARTHWORKS & RETAINING WALLS TO BE COMPLETED AND EXPOSED AREAS EFFECTIVELY STABILISED PRIOR TO WORKS PROCEEDING IN OTHER AREAS.
 - CONTRACTOR TO LIMIT AREA EXPOSED AT ANY ONE POINT TO LESS THAN 20m² UNO.
 - MAINTENANCE
 - SEDIMENT FENCES AND EARTH BILLS WITHIN ROADS TO BE ERECTED AS INDICATED OR REQUIRED.
 - SEDIMENT FENCES TO BE INSPECTED WEEKLY AND FOLLOWING RAINFALL EVENTS. ANY REPAIRS REQUIRED ARE TO BE EFFECTED IMMEDIATELY.
 - SEDIMENT AFTER RAIN IS TO BE CLEANED FROM STREETS AND ALLOTMENTS IMMEDIATELY AND CORRECTIVE ACTION TAKEN TO AVOID A RE-OCCURRENCE OF THE FAILURE.
 - SEDIMENT BASINS TO BE FLOCCULATED AND PUMPED OUT WITHIN 3 DAYS OF ALL RAIN EVENTS.

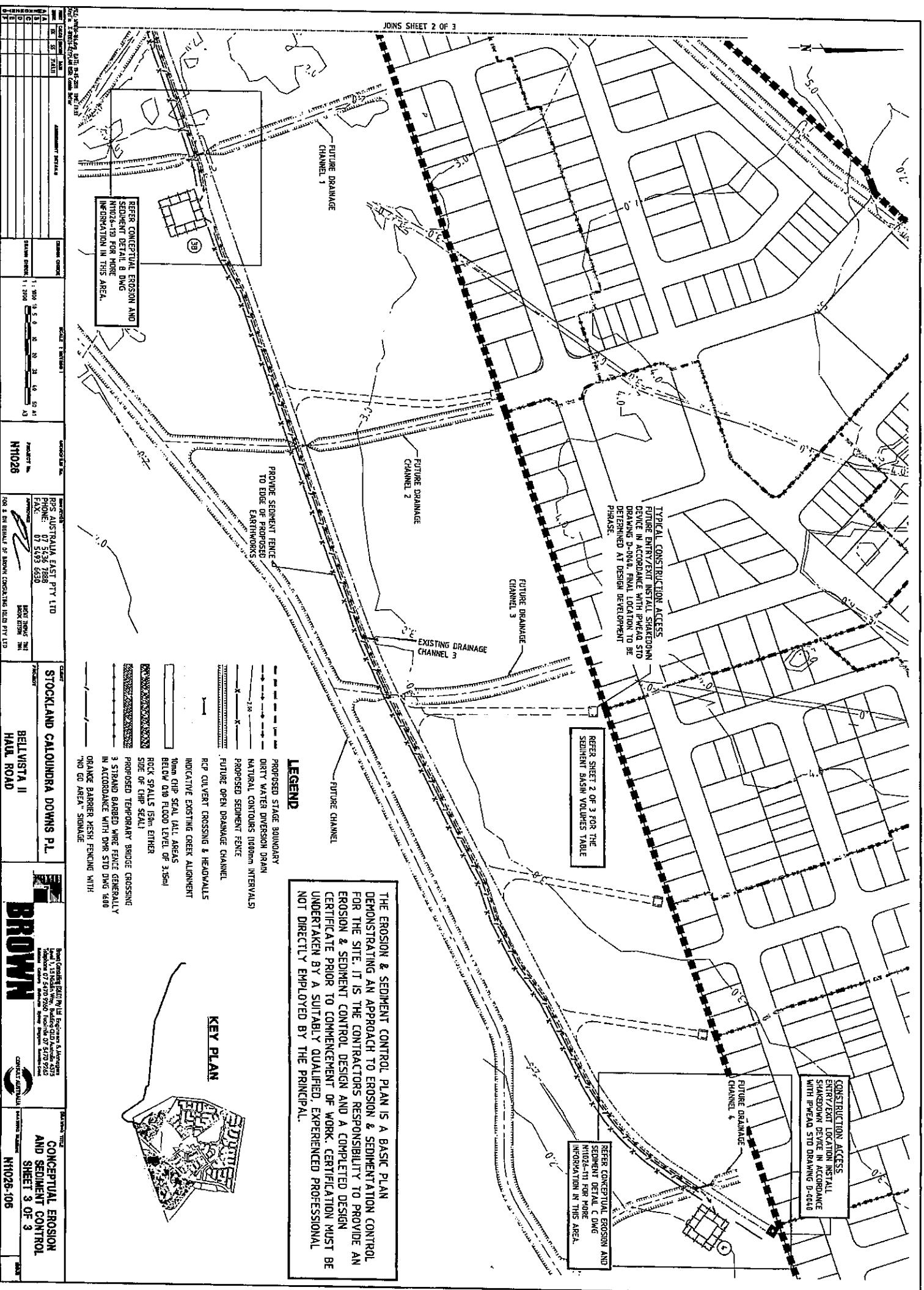


THESE EROSION & SEDIMENT CONTROL PLANS APPLY TO THE HAUL ROAD CONSTRUCTION AND USE ONLY. DETAILS OF EROSION AND SEDIMENT CONTROL MEASURES FOR BULK EARTHWORKS AND CIVIL WORKS OF THE FUTURE BELLYSTA II DEVELOPMENT WILL BE SHOWN ON THE RELEVANT APPLICATION PLANS.

THE EROSION & SEDIMENT CONTROL PLAN IS A BASIC PLAN DEMONSTRATING AN APPROACH TO EROSION & SEDIMENTATION CONTROL FOR THE SITE. IT IS THE CONTRACTORS RESPONSIBILITY TO PROVIDE AN EROSION & SEDIMENT CONTROL DESIGN AND A COMPLETED DESIGN CERTIFICATE PRIOR TO COMMENCEMENT OF WORK. CERTIFICATION MUST BE UNDERTAKEN BY A SUITABLY QUALIFIED, EXPERIENCED PROFESSIONAL NOT DIRECTLY EMPLOYED BY THE PRINCIPAL.

PLANS AND DOCUMENTS
referred to in the ULDA
APPROVAL dated 16/9/11





PLANS AND DOCUMENTS
referred to in the ULDA
APPROVAL dated 16/11/11

GENERAL NOTES

1. THIS DESIGN FOR EROSION AND SEDIMENT CONTROL IS CONCEPTUAL ONLY. THE CONTRACTOR SHALL MODIFY OR INSTALL ADDITIONAL/ ALTERNATIVE MEASURES DURING THE CONSTRUCTION AND MAINTENANCE PERIOD IN ORDER TO COMPLY WITH BEST PRACTICE STANDARDS IN ACCORDANCE WITH BUT NOT LIMITED TO BROWN CONSULTANTS' SPECIFICATION 17, "THE MANUAL FOR EROSION AND SEDIMENT CONTROL (V12) AND ALL STATUTORY REQUIREMENTS".
2. PREPARED LATER CONTAMINANTS HAS BEEN IN THE ENVIRONMENTAL PROTECTION ACT 1994 WHICH REQUIRES THE EROSION AND SEDIMENT CONTROL MEASURES TO BE RAISED TO RAINFALL OCCUR UNLESS IT IS REASONABLE TO BELIEVE THAT IT IS NOT PRACTICAL TO PREVENT OR MINIMISE THE RELEASE AND CONCENTRATION OF CONTAMINATION. THESE MEASURES MUST INCLUDE AS A MINIMUM, BUT ARE NOT LIMITED TO, THE FOLLOWING:
 - A. ENSURE NO ESSENTIAL NECESSARY OF SOIL IS PREVENTED BY RESTRICTING THE EXTENT OF CLEARING TO THAT NECESSARY FOR ACCESS TO, AND SAFE CONSTRUCTION OF, THE APPROVED WORKS; PROTECTING VEGETATION IN ALL OTHER AREAS OF THE SITE; AND BY MINIMISING THE DURATION OF SOIL EXPOSURE BY:
 - STAKING THE WORKS TO MINIMISE THE AREA OF SOIL EXPOSED AT ANY ONE TIME;
 - EFFECTIVELY STABILISING CLEARED AREAS PRIOR TO RAINFALL IF WORKS ARE DELAYED OR INTERRUPTED FOR ANY REASON;
 - EFFECTIVELY STABILISING AREAS AT FINISHED LEVEL WITHOUT DELAY AND PRIOR TO RAINFALL; AND
 - EFFECTIVELY STABILISING STEEP AREAS, SUCH AS STOCKPILES, BATTERS AND EMBANKMENTS.
 - B. WHERE IT IS NOT FEASIBLE TO EFFECTIVELY STABILISE CLEARED AREAS OF EXPOSED SOIL, SUCH AS AREAS BEING ACTIVELY WORKED, IMPLEMENT A FULL SUITE OF EROSION AND SEDIMENT CONTROL TO MAXIMISE SEDIMENT CAPTURE IN THOSE AREAS AND TO MINIMISE EROSION SUCH THAT EROSION BY ALL FORMS OTHER THAN SPLASH (RANDOM) IMPACT EROSION AND SHEET EROSION DOES NOT OCCUR; AND
 - C. IN AREAS WHERE IT IS NOT FEASIBLE TO EITHER EFFECTIVELY STABILISE THE SURFACE OR WHERE IT IS NOT PRACTICAL TO PREVENT OR MINIMISE THE CONCENTRATION OF CONTAMINATION, THE AREAS BEING ACTIVELY WORKED AND WHERE THE IMPLEMENTATION OF SOME EROSION AND SEDIMENT CONTROLS WOULD IMPERE CONSTRUCTION ACTIVITIES, ENSURE CONTINGENCY MEASURES ARE AVAILABLE ON SITE AND ARE IMPLEMENTED PRIOR TO RAIN, TO MAXIMISE SEDIMENT CAPTURE AND TO MINIMISE EROSION SUCH THAT EROSION BY ALL FORMS OTHER THAN SPLASH (RANDOM) IMPACT EROSION AND SHEET EROSION DOES NOT OCCUR.
 - D. EFFECTIVELY STABILISE ALL STOCKPILES, BATTERS AND EMBANKMENTS WITHOUT DELAY, WHERE IT IS NOT FEASIBLE TO EFFECTIVELY STABILISE A STOCKPILE, BATTER OR EMBANKMENT, SUCH AS AREAS BEING ACTIVELY WORKED, ENSURE THAT SEDIMENT EROSION AND SEDIMENTATION OF STOCKPILES, BATTERS OR EMBANKMENTS IS NOT CAUSED BY CONCENTRATED CLEARED FLOWS.
 - E. ENSURE CLEAN STORMWATER IS DIVERTED OR HANDLED AROUND OR THROUGH THE SITE WITHOUT INCREASING THE CONCENTRATION OF TOXIC SUSPENDED SOLIDS OR OTHER CONTAMINANTS IN THE FLOW AND WITHOUT CAUSING EROSION ON SITE OR OFF SITE. IF IT IS NOT FEASIBLE TO DIVERT ALL AREAS DISCHARGING CLEAN STORMWATER AROUND OR THROUGH THE SITE, MANAGE THE CLEAN STORMWATER USING AS FOR CONTAMINATED STORMWATER RUNOFF, AND ENSURE THAT SEDIMENT BATTERS ARE SIZED TO ACCOMMODATE THE ADDITIONAL VOLUME OF RUNOFF FLEE EAST ADVICE NOTE 21.
 - F. ENSURE SHEET FLOWS OF STORMWATER ARE MANAGED SUCH THAT SHEET AND LINE EROSION, SUCH AS THAT CAUSED BY STORMWATER FLOWS INCLUDING DRAINAGE LINES, DIVERSION DRAINS, CHANNELS AND BATTER CHUTES ARE MANAGED ON TO AND AROUND THE AVERAGE REGURGITATION INTERVAL, (ARI) EVENT OF 1 IN 2 YEAR, ARI WITHOUT CAUSING WATER CONTAMINATION, SHEET, RILL OR GULLY EROSION, SEDIMENTATION, OR DAMAGE TO STRUCTURES OR PROPERTY.
 - G. ENSURE MEASURES HAVE BEEN IMPLEMENTED SUCH THAT THE RUNOFF FROM ALL DISTURBED AREAS FLOWS TO A SEDIMENT BASIN OR BATTERS, WHERE IT IS NOT FEASIBLE TO DIVERT RUNOFF FROM DISTURBED AREAS OF THE SITE TO A SEDIMENT BASIN.
 - H. IMPLEMENT CONTINGENCY EROSION AND DRAINAGE CONTROLS PRIOR TO RAINFALL TO PREVENT OR MINIMISE EROSION OF THOSE AREAS DOES NOT OCCUR, INCLUDING EROSION CAUSED BY SPLASH (RANDOM) IMPACT, SHEET, RILL OR GULLY EROSION PROCESSES (SEE EAST ADVICE NOTE 21).
 - I. ENSURE EACH SEDIMENT BASIN HAS THE CAPACITY TO TREAT FLOWS TO EXISTING BEST PRACTICE STANDARDS (SEE EAST ADVICE NOTE 4) AND AS A MINIMUM TO CHAIN ALL THE STORMWATER RUNOFF FROM THE NORTH PERCENTILE 5 DAY RAINFALL DEPTH AND STORE 6 MONTHS SEDIMENT FROM THE RECEIVING CATCHMENT, AS DETERMINED USING THE REVISED UNIVERSAL SOIL LOSS EQUATION.

1. ENSURE SEDIMENT BASINS ARE MAINTAINED WITH SUFFICIENT STORAGE CAPACITY TO CAPTURE AND TREAT THE RUNOFF FOR THE DESIGN RAINFALL DURATION OR EVENT, WHERE SEDIMENT BASINS ARE PROPOSED TO BE OVERSIZED FOR STORAGE OF CAPTURED WATER FOR RE-USE, INSTALL SURVEY MARKERS IN EACH SUCH BASIN TO INDICATE THE LEVEL THAT WATER WHEN THE BASIN MUST BE LOWERED TO, IN ORDER TO MEET THE STORAGE CAPACITY SPECIFIED IN THE ABOVE REQUIREMENT.
 - K. DESIGN SEDIMENT BASINS ARE DERATED AS SOON AS PRACTICABLE AFTER EACH MAINTENANCE EVENT.
 - L. ENSURE THAT DURING OPERATING, THE CONCENTRATION OF TOTAL SUSPENDED SOLIDS (TSS) DISCHARGED DOES NOT EXCEED 350 MG/L AND THAT PH IS WITHIN THE RANGE OF 6.5-8.5. THE CONCENTRATION OF TSS RELEASED BY DEWATERING MAY NOT EXCEED 500 MG/L WHERE IT CAN BE DEMONSTRATED AND SUPPORTED THROUGH DOCUMENTATION THAT:
 - FURTHER SIGNIFICANT RAINFALL IS FORECAST TO OCCUR BEFORE THE TSS CONCENTRATION IS
 - LIKELY TO BE REDUCED TO 50 MG/L ; AND
 - RELISING A HIGHER CONCENTRATION OF TOTAL SUSPENDED SOLID WILL RESULT IN A BETTER ENVIRONMENTAL OUTCOME BY PROVIDING STORAGE FOR THE CAPTURED AND TREATMENT OF SOLIDS.
 - M. RUNOFF FROM THE IMMINENT RAINFALL AND RUNOFF: AN
 - FLOCCULANT HAS BEEN APPLIED AND THE CONCENTRATION OF TSS IN THE CAPTURED WATER
 - HAS ALREADY SIGNIFICANTLY DECREASED.
 - N. ENSURE SEDIMENT BASINS AND ASSOCIATED STRUCTURES SUCH AS INLETS, OUTLETS AND SPILLWAYS ARE STRUCTURALLY SOUND FOR 10 YEAR AIR RAINFALL EVENT.
 - O. ENSURE ACCUMULATED SEDIMENT FROM BASINS AND OTHER CONTROLS IS REMOVED AND DISPOSED OF APPROPRIATELY WITHOUT CAUSING WATER CONTAMINATION.
 - P. ENSURE SEDIMENT DOES NOT LEAVE THE SITE ON THE TYRES OF VEHICLES.
3. THE ENVIRONMENTAL PROTECTION ACT 1984 STATES THAT A PERSON MUST NOT CARRY OUT ANY ACTIVITY THAT CAUSES, OR IS LIKELY TO CAUSE, ENVIRONMENTAL HARM UNLESS THAT PERSON TAKES ALL REASONABLE AND PRACTICAL PRECAUTIONS TO PREVENT OR MINIMIZE THE HARM.
- ENVIRONMENTAL HARM INCLUDES ENVIRONMENTAL DAMAGE IN REGARD PERSONS AND ENTITIES INVOLVED IN THE CIVIL, ENGINEERING AND CONSTRUCTION PHASES OF THIS DEVELOPMENT, ARE TO ADOPT THE FOLLOWING GENERAL ENVIRONMENTAL DUTY TO MINIMIZE THE RISK OF CAUSING ENVIRONMENTAL HARM.
- ENVIRONMENTAL HARM IS DEFINED BY THE ACT AS ANY ADVERSE AFFECT, OR POTENTIAL ADVERSE AFFECT, WHETHER TEMPORARY OR PERMANENT AND OF WHATEVER NATURE/DURATION OR FREQUENCY, ON AN ENVIRONMENTAL VALUE AND INCLUDES ENVIRONMENTAL DAMAGE, THEREFORE, A PERSON SHOULD TAKE ALL INTERFERE WITH THE ENVIRONMENT OF THE PROJECT, INCLUDING BUT NOT LIMITED TO, THE FOLLOWING:
- NOISE, VIBRATION, STAY, SMELL, ASH, DUST, LOST WATER, WASTE PRODUCTS, OIL, OR OILSPILLS, OR CAUSE HAZARDS LIKELY IN THE CONDON OF THE ADMINISTERING AUTHORITY TO CAUSE UNWANTED DISTURBANCE OR ANNOYANCE TO PERSONS OR AFFECT PROPERTY NOT CONNECTED WITH THE USE.
4. THE CONTRACTOR IS TO TAKE ALL NECESSARY PRECAUTIONS TO CONTROL EROSION AND DOWNSIDE/AN CONTEMPORATION DURING ALL STAGES OF CONSTRUCTION INCLUDING THE MAINTENANCE PERIOD.
5. WHERE IT IS REQUIRED TO SLASH EXISTING VEGETATION EITHER PRIOR TO THE COMMENCEMENT OF CONSTRUCTION, OR DURING THE MAINTENANCE PERIOD, THE CONTRACTOR SHALL BE OBLIGED TO MINIMIZE THE RISK OF EROSION BY ASSIST WITH THE RETENTION OF SOILS ON SITE (I.E. ASSIST IN THE PREVENTION OF EROSION).
6. WHERE THE EXISTING VEGETATION WITH THE PROPOSED LOTS AND / OR PARKLAND IS DISBURSED AS A RESULT OF THE CONSTRUCTION WORKS, SAND EARTHWORKS ARE TO BE TOPSOILED AND EFFECTIVELY STABILIZED WITHIN FIVE (5) DAYS, (EITHER IF RAIN EXPECTED) OR FIRMAL ALTERNATIVE EARTHWORKS, AN EFFECTIVELY STABILIZED SURFACE IS DEFINED AS ONE THAT DOES NOT HAVE
 - VISIBLE EVIDENCE OF SOIL LOSS CAUSED BY SHEET, RILL OR GULLY EROSION
 - OR
 - LEAD TO SEDIMENTATION, OR
 - LEAD TO WATER CONTAMINATION.
7. ALL CONSTRUCTION VEHICLES ARE TO ACCESS THE SITE VIA A SINGLE POINT OF ACCESS: THE POINT OF ACCESS, TOGETHER WITH THE MEASURES TO BE PREVENTED, ARE TO BE AGREED WITH CONSULT'S DESIGNED REPRESENTATIVE ON SITE. THE PRACTICE AM OF THE MEASURES TO BE PREVENTED IS / ARE TO LIMIT THE TRAFFIC OF DETERIOROUS MATERIALS AROUND THE SUSPENDING ROAD NETWORK.

8. THE CONTRACTOR SHALL PROVIDE GULLEY INLET PROTECTION TO ALL GULLEY INLET STRUCTURES LOCATED DIRECTLY DOWNSTREAM OF THE PROPOSED DEVELOPMENT WORKS.
9. APPROPRIATE PROVISIONS ARE TO BE PROVIDED TO THE INTERFACET BETWEEN THE EXISTING ROADWAY PARAPETS AND THE NEW ROADWORKS CONSTRUCTION. THE PROVISIONS SHALL ADDRESS WORKPLACE HEALTH AND SAFETY CONCERNS (I.E. RESTRICTING ACCESS BY THE GENERAL PUBLIC TO THE SITE).
10. THE LOCATION OF THE CONSTRUCTION VEHICLE COMPOUND, SITE OFFICE AND THE VEHICLE STAGING AREA SHALL BE AGREED WITH COUNCIL'S DESIGNATED REPRESENTATIVE ON SITE, PRIOR TO THE COMMENCEMENT OF WORKS.
11. CLEARED VEGETATION IS TO NOT BE BURNED ON SITE, ALL VEGETATIVE WASTE(S) SHALL BE MULCHED AND THEREAFTER RETAINED ON SITE FOR USE AS PART OF THE EROSION AND SEDIMENTATION CONTROL STRATEGY OR THE LANDSCAPING / REVEGETATION WORKS. ALL STOPS AND / OR OTHER DRAINAGE MATTER NOT SUITABLE FOR MULCHING SHALL BE DISPOSED OF AT AN APPROVED WASTE DISPOSAL FACILITY.
12. STORMWATER FENCE AND TURNING RUNNING DOWNSLOPES SHALL HAVE REGULAR FLOW DISSEPARATORS AT 65 TO SLOPE AS DIRECTED CROSSING OF SAND BAGS OR SIMILAR AS REQUIRED.
13. DURING THE CONSTRUCTION PROCESS INCLUDING THAT PERIOD DURING WHICH THE WORKS ARE "ON MAINTENANCE" SHOULD COUNCIL'S DESIGNATED REPRESENTATIVE REQUEST ADDITIONAL EROSION AND SEDIMENT CONTROL MEASURES BE IMPLEMENTED, SAID MEASURES SHALL BE IMPLEMENTED AT THE EARLIEST TIME POSSIBLE. NOTWITHSTANDING THE ABOVE REQUIREMENT ANY REASONS REQUESTED TO BE IMPLEMENTED BY COUNCIL'S DESIGNATED REPRESENTATIVE SHALL BE IMPLEMENTED WITHIN 24 HOURS OF THE TIME OF THE REQUEST.
14. ALL ROADWAY(S) / SEWER RETICULATION TRENCHES EITHER ADJACENT TO EXISTING DEVELOPMENTS OR PERPENDICULAR TO THE CROSSEVAL OF THE LAND ARE TO BE TOPSOILED (TERRAIN MINIMING) AND TIERED, FOR A MINIMUM 50MM MIDFILL.
15. THE CONTRACTOR SHALL CONSTRUCT LINED CUTOFF DRAINS IN WORK AREAS SO AS TO LIMIT SLOPE LENGTHS TO A MAXIMUM OF 60%.
16. THE CONTRACTOR IS RESPONSIBLE FOR ENSURING THAT NO RELEASE OR FLOW IS PERMITTED FROM THE SITE, THROUGHOUT THE EARTHWORKS AND CONSTRUCTION PERIOD TO ANY WATERWAYS OR STORMWATER DRAINAGES LEADING TO A WATERWAY OR AREA OF NATIVE VEGETATION UNLESS THE LEVELS OF TOTAL SUSPENDED SOLIDS DOES NOT EXCEED A CONCENTRATION OF 50 MG/L.
17. ALL SEDIMENT CONTROL DEVICES SHALL BE MONITORED, CLEANED AND/OR REPAIRED WHENEVER THE ACCUMULATED SEDIMENT REDUCES THE CAPACITY BY 50%.
18. ALL PERMEETER BANK/SWALE SHALL HAVE UNINTERRUPTED POSITIVE GRADE TO AN OUTLET.
19. AT ALL TIMES THE CONTRACTOR SHALL MONITOR THE PREVAILING WEATHER CONDITIONS AND PROTECT OR STABILISE ANY DOWNSTREAM CONSTRUCTION AND GULLEY INLETS.
20. CLEANING OF SITE AND STOCK THE AREAS TO BE AS DIRECTED BY THE SUPERVISING ENGINEER.
21. WHERE PRACTICABLE THE CONTRACTOR SHALL DIVERT CLEAN WATER ENTERING THE SITE FROM EXTERNAL CATCHMENTS AND DIRECTED TO THE STORMWATER SYSTEM, THIS DISCHARGE POINT SHOULD BE ROCK LINED, REGULAR ROCK CHECK DAMS SHOULD BE POSITIONED ALONG THE VEGETATED DAMAGE LINE LEADING TO THIS DISCHARGE POINT.
22. REGULAR INSPECTIONS AND MAINTENANCE OF VEHICLE WASHDOWN AREA, SITE AND STORAGE COMPOUND TO BE CARRIED OUT BY CONTRACTOR.
23. AREAS USED FOR STORAGE OF CHEMICALS USED FOR CONSTRUCTION PURPOSES SHALL HAVE STORMWATER CONTROL DEVICES INSTALLED ADJACENT TO THEM, I.E. EARTH DAM AND SEDIMENT FENCES, UPON COMPLETION OF ROADWORKS WASTE PRODUCTS ARE TO BE DISPOSED OF AS PER LOCAL AUTHORITY GUIDELINES AND TEMPORARY DEVICES ARE TO BE REMOVED AND AREA REMEDIATED.

FILE #100-37409 DATE 10-05-2011 TIME 09:33
 2007a X-TRACK-177 USE Camtek buffer

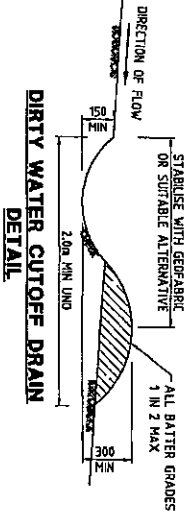
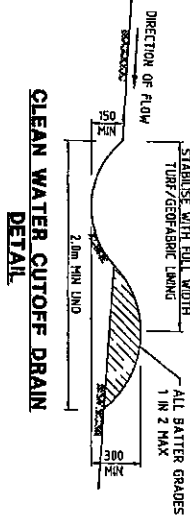
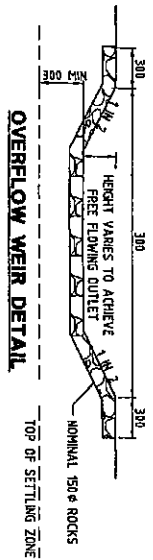
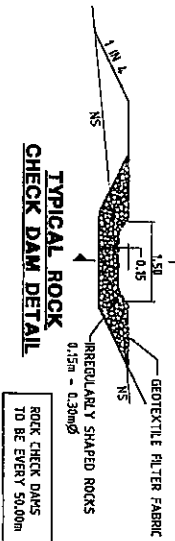
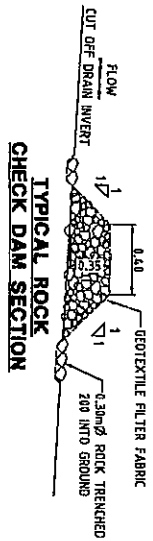
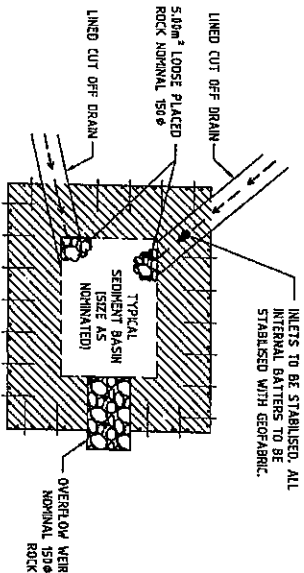
FILE #100-37469 DATE 08-25-2011
XERO X-1000-171 1000 Cheetah 0011

[illegible]

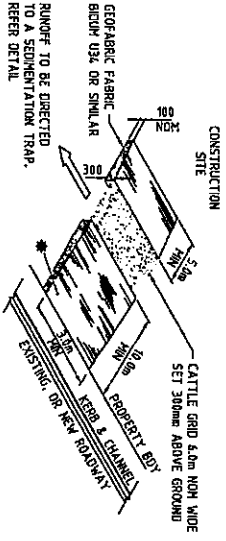
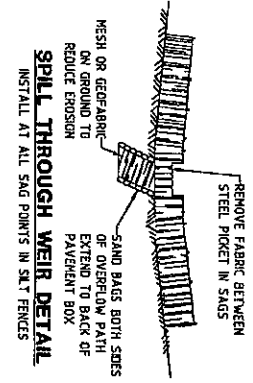
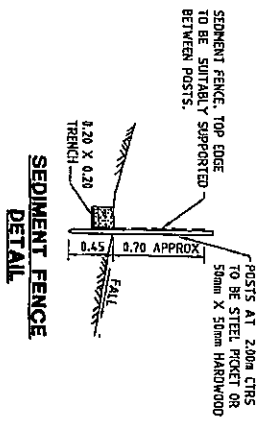
PLANS AND DOCUMENTS

referred to in the UDA

APPROVAL dated 16/9/11



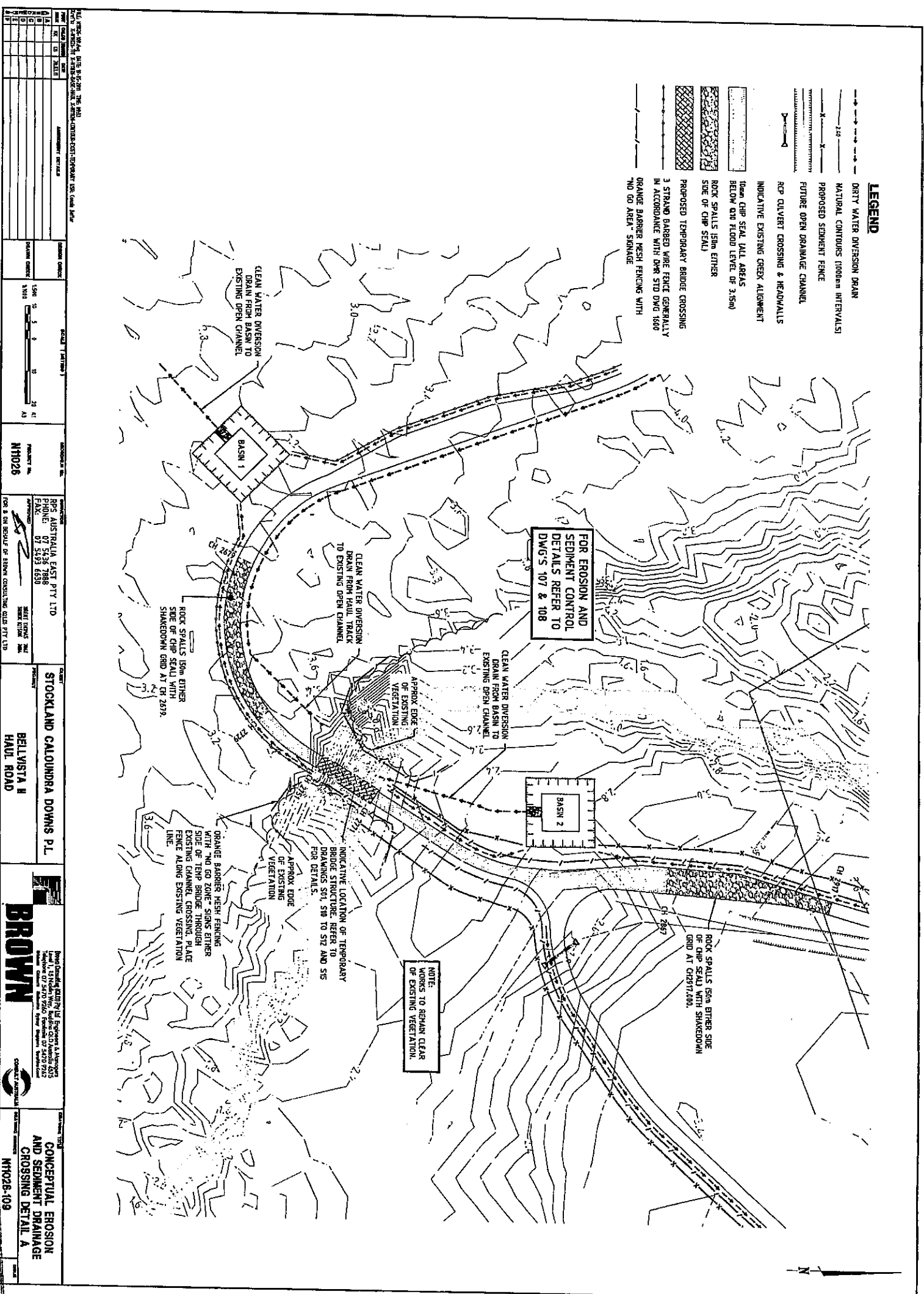
CUTOFF DRAINS - MAINTENANCE PROGRAM
 1. INSPECTIONS TO BE CARRIED OUT AFTER EACH RUNOFF EVENT.
 2. SEDIMENT BUILD UP TO BE REMOVED DURING CONSTRUCTION AS REQUIRED BY THE SUPERINTENDENT.
 NOTE: DRAIN TO BE CIRCULAR PARABOLIC OR TRAPEZOIDAL. V SHAPED IS NOT ACCEPTABLE. GRADIENT BETWEEN 1.7% TO 5.0%.



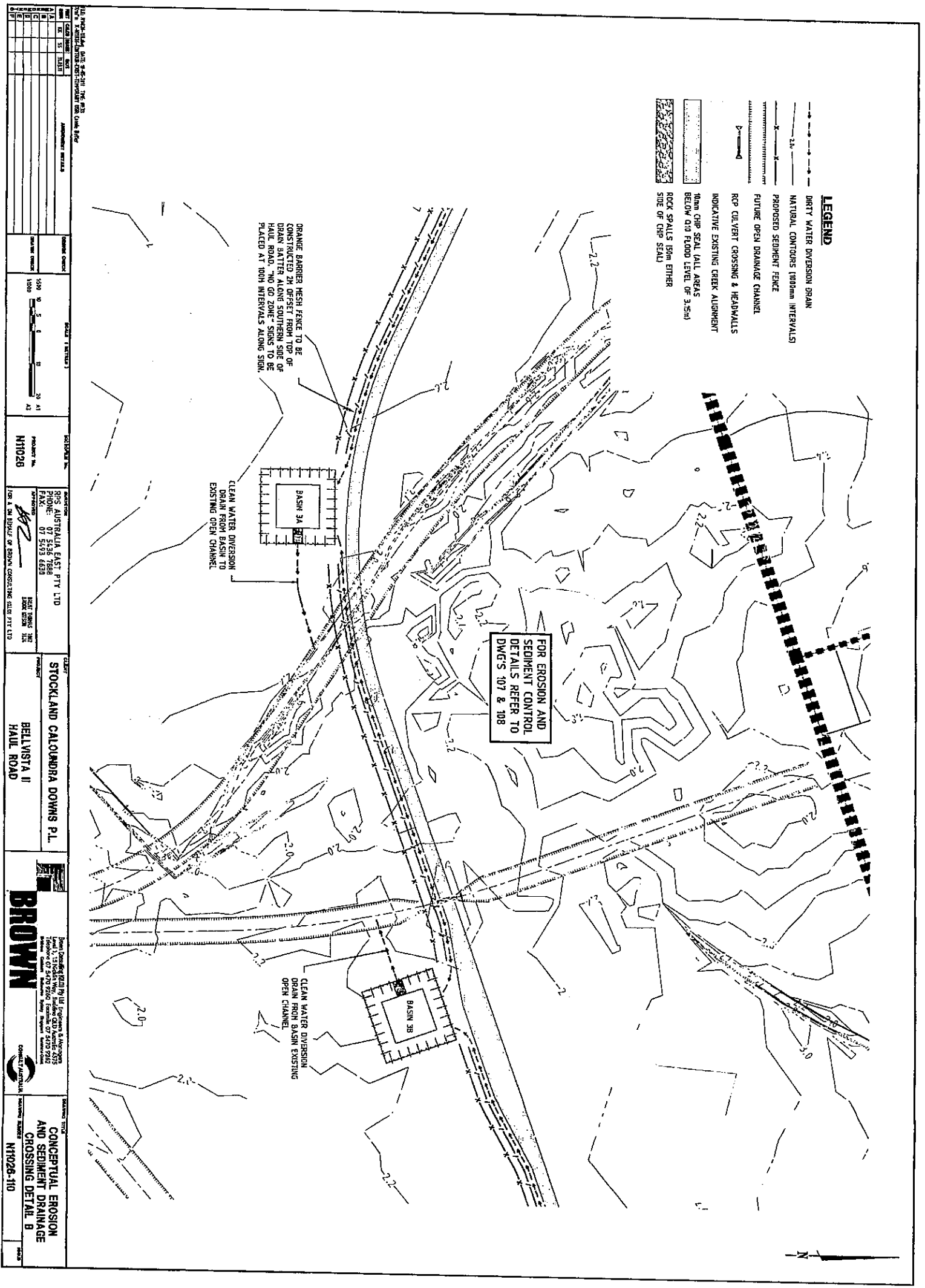
TEMPORARY CONSTRUCTION ENTRY/EXIT SEDIMENT TRAP
 NTS

PROJECT NO.	NTS	PROJECT NAME	STOCKLAND CALOUNDRUM DOWNS P.L.
DATE	1/1/2011	CLIENT	BELVISTA II
DESIGNER	NTS	LOCATION	HAUL ROAD
APPROVED BY	NTS	CONCEPTUAL EROSION AND SEDIMENT CONTROL DETAILS	NTS
DATE	1/1/2011	PROJECT NO.	NT1026-108

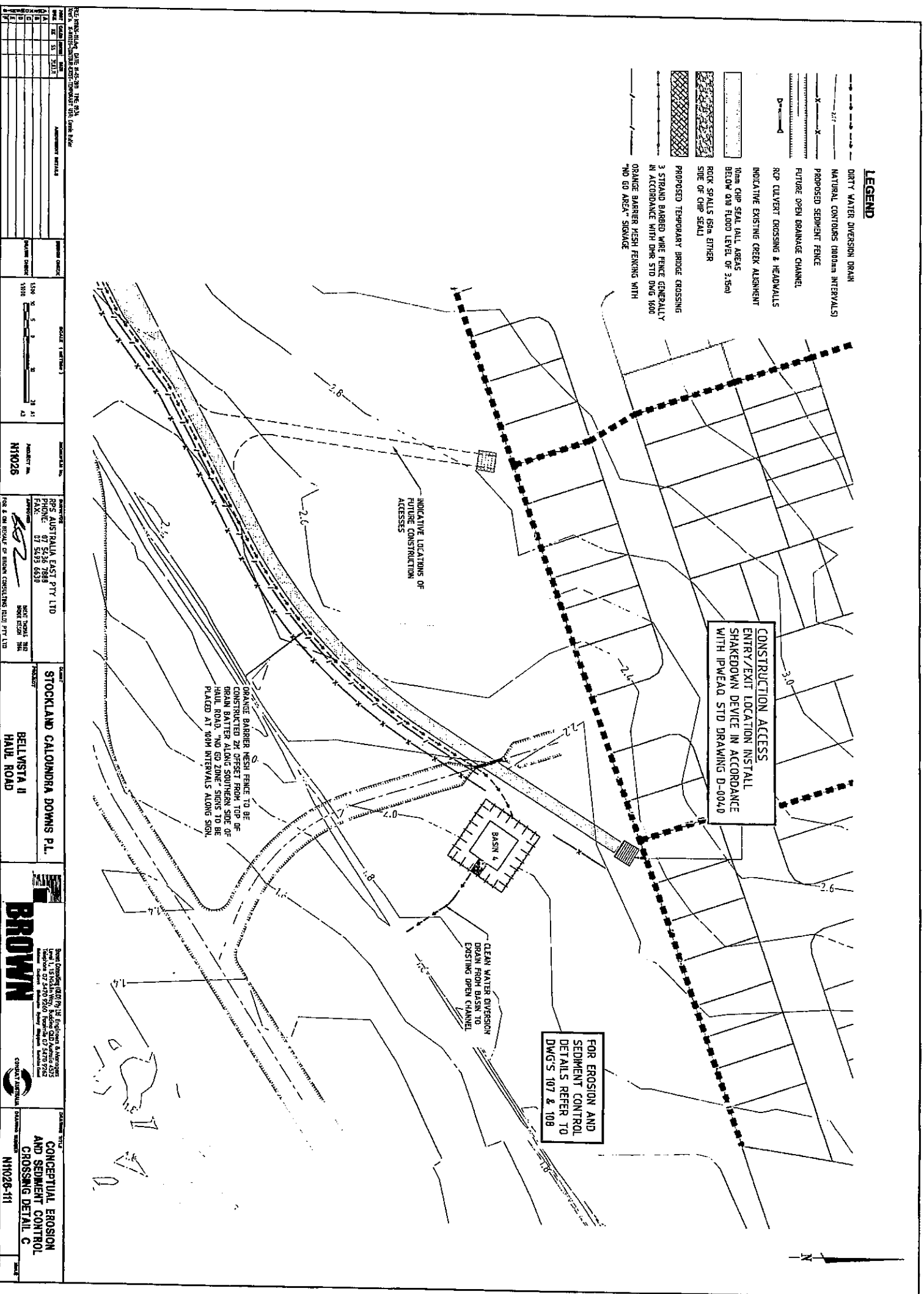
PLANS AND DOCUMENTS
 referred to in the ULDA
 APPROVAL dated 16/9/11



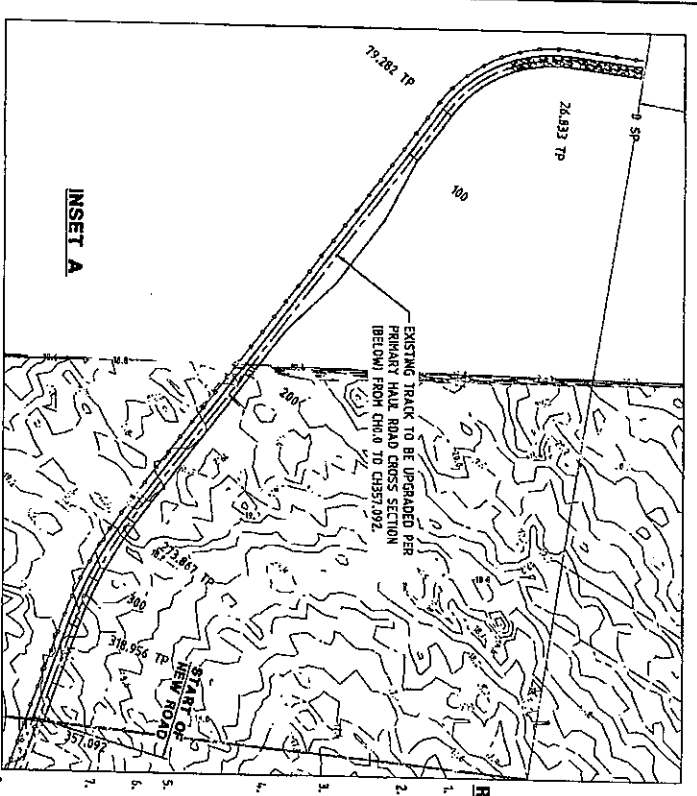
PLANS AND DOCUMENTS
referred to in the ULDA
APPROVAL dated 6/19/11



PLANS AND DOCUMENTS
referred to in the ULDA
APPROVAL dated 16/19/11



PLANS AND DOCUMENTS
referred to in the ULDA
APPROVAL dated 16/7/11



ROADWORKS & DRAINAGE NOTES

1. ALL DIMENSIONS ARE IN METRES UNLESS OTHERWISE SHOWN.
2. CONTRACTOR TO LIAISE WITH ALL THE RELEVANT SERVICE AUTHORITIES TO ASCERTAIN SERVICES PRESENT ON-SITE. ANY ALTERATION WORKS TO SERVICES WILL BE CARRIED OUT BY THAT SERVICE AUTHORITY ONLY.
3. THE CONTRACTOR SHALL NOTIFY THE ENGINEER PRIOR TO COMMENCEMENT OF DEMOLISHING ANY EXISTING STRUCTURES WITHIN THE SITE AREAS.
4. NOT WITHSTANDING THE LIMITS OF CUTTING AND FILLING SHOWN ON THE TYPICAL CROSS SECTION, THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE FINISHED STAGE 2 CONTOURS AFTER AVOIDED DRAINAGES HAVE BEEN APPROVED BY COUNCIL.
5. BATTERS TO HAVE A MINIMUM OF 75MM TOPSOIL AND FILL WITH TURF IF ORDERED.
6. MATCH TO EXISTING AT PAYMENT EXTENSION OR WIDENING TO BE IN ACCORDANCE WITH IPWEA STD DWG SED 8-710.
7. SURVEY TEST RESULTS TO BE FORWARDED TO SUPERVISING ENGINEER FOR REVIEW. IN THE EVENT OF DISCREPANCIES, THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE FINISHED STAGE 2 CONTOURS AFTER AVOIDED DRAINAGES HAVE BEEN APPROVED BY COUNCIL.
8. THE STORMWATER PIPE CLASSES HAVE BEEN DESIGNED FOR SERVICE LOADS ONLY, AND THE CONTRACTOR SHALL ASSESS ANTICIPATED CONSTRUCTION LOADS AND UPGRADE THE PIPE CLASSES, IF NECESSARY, IN ACCORDANCE WITH A.S. 3725-1989, AT THE CONTRACTORS COST.

NOTE: AREAS OF PRIMARY HAUL ROAD BELOW 010 FLOOD LEVEL OF PEELSON DAM AREAS OF SECONDARY HAUL ROAD AS NOTED TO BE 100m LONG SECTED TO BE MAINTAINED.

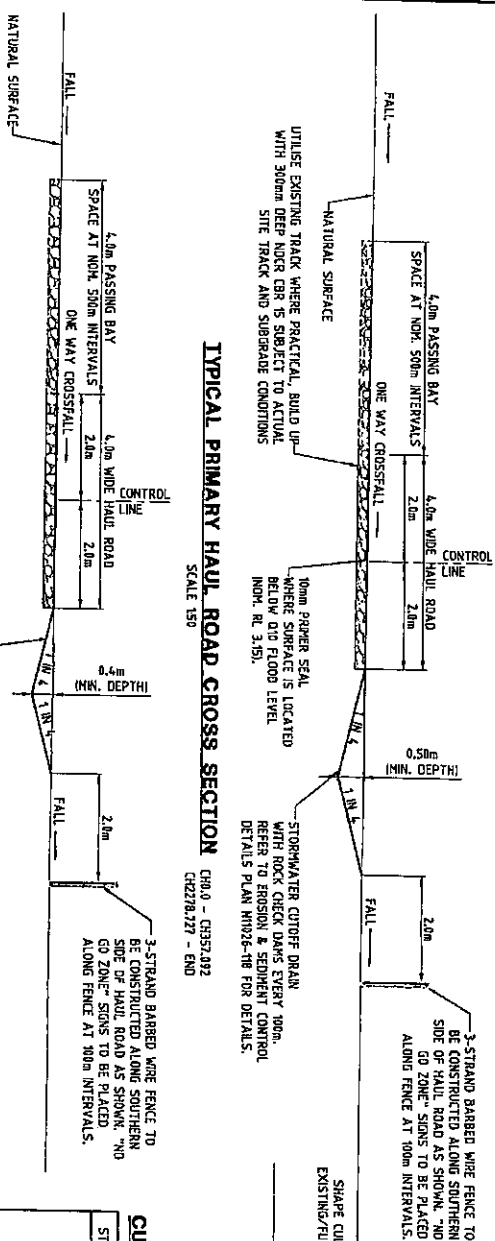
NOTE: ROAD CONSTRUCTION ON PRIMARY HAUL ROAD TO BE COMPLETED BY 22/07/2012. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE FINISHED STAGE 2 CONTOURS AFTER AVOIDED DRAINAGES HAVE BEEN APPROVED BY COUNCIL.

LEGEND

- PROPOSED STAGE BOUNDARY
- DIRTY WATER DIVERSION DRAIN
- NATURAL CONTOURS (200mm INTERVALS)
- FOURDE OPEN DRAINAGE CHANNEL
- REF CULVERT CROSSING & HEADWALLS
- WHEN CHIP SEAL HALL AREAS BELOW 010 FLOOD LEVEL, OF 3.5m
- ROCK SPALLS 150m EITHER SIDE OF CHIP SEAL
- 3 STRAND BARBED WIRE FENCE GENERALLY IN ACCORDANCE WITH DMR STD DWG 1680
- ORANGE BARRED WIRE FENCE WITH 700 GO ZONE SIGNS PLACED AT 100m INTERVALS

TYPICAL PRIMARY HAUL ROAD CROSS SECTION

SCALE 1:50



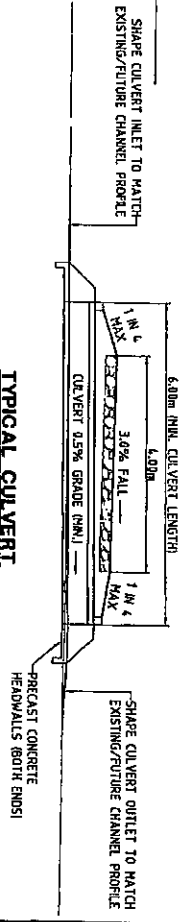
CULVERT DETAIL TABLE

STRUCTURE	SIZE	FLOW
1	750RCP CLASS 3	Q2 FLOW +4.63m
2	750RCP CLASS 3	Q2 FLOW +4.63m
3	750RCP CLASS 3	Q2 FLOW +4.63m
4	750RCP CLASS 3	Q2 FLOW +4.63m
5	750RCP CLASS 3	Q2 FLOW +4.63m
EXIST 1	375RCP CLASS 3	Q2 FLOW +0.79m
EXIST 2	300RCP CLASS 3	Q2 FLOW +0.06m
EXIST 3	400RCP CLASS 3	Q2 FLOW +0.30m

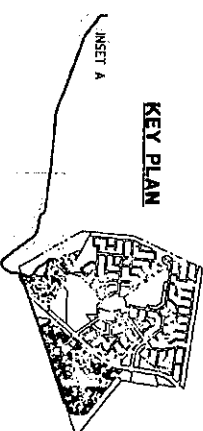
NOTE: EXACT LOCATION AND INVERT LEVELS OF DRAINAGE CULVERTS TO BE DETERMINED ON SITE. ALL CULVERTS TO BE AT 100m 0.5% GRADE.

TYPICAL CULVERT CROSSING DETAIL

SCALE 1:50

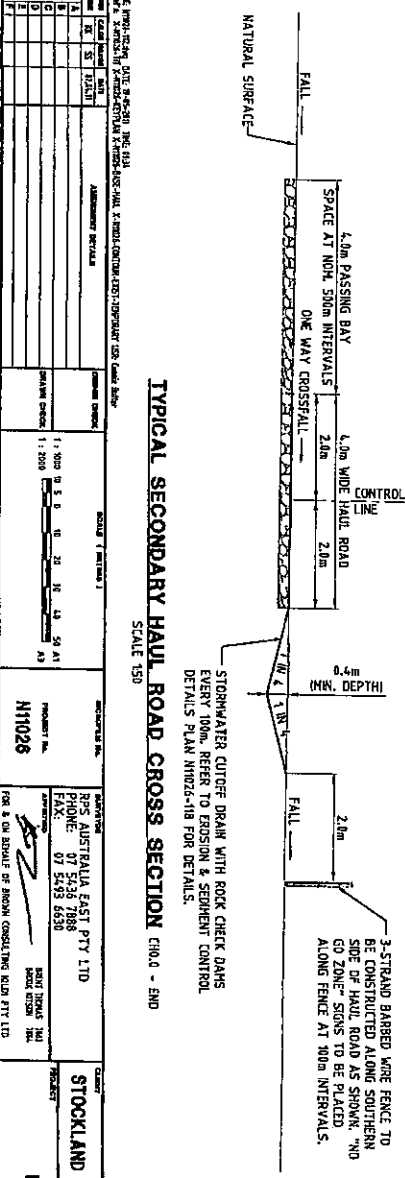


KEY PLAN



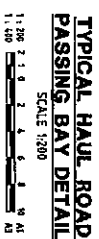
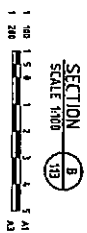
TYPICAL SECONDARY HAUL ROAD CROSS SECTION

SCALE 1:50



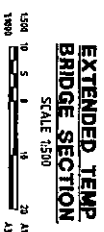
PROJECT TITLE ROADWORKS & DRAINAGE DETAIL PLAN SHEET 1 OF 3		PROJECT NO. N1026	DATE 11/03/2012	SCALE 1:50	DESIGNER BROWN	CHECKED BY BROWN	APPROVED BY BROWN
CLIENT STOCKLAND CALOUNDERA DOWNS P.L.		PROJECT BELVISTA II HAUL ROAD	PROJECT NO. N1026	DATE 11/03/2012	DESIGNER BROWN	CHECKED BY BROWN	APPROVED BY BROWN

PLANS AND DOCUMENTS
referred to in the ULDA
APPROVAL dated 16/1/11



SCALE 1:200

1:200 2:10 2 4 6 8 10 AT
1:400 AS



TIME	GRADE	SCORE	STUDENT
1	55	11.0	A
2	55	11.0	B
3	55	11.0	C
4	55	11.0	D
5	55	11.0	E
6	55	11.0	F

PLANS AND DOCUMENTS
referred to in the ULDA
APPROVAL dated 16/ 7/ 11

STRUCTURAL ENGINEERING DRAWINGS
BELLVISTA II
TEMPORARY HAUL ROAD BRIDGE
LAMEROUGH CREEK

DRAWING INDEX

- COVER SHEET, INDEX & STRUCTURAL NOTES
GENERAL ARRANGEMENT - SHEET 1 - BRIDGE LAYOUT PLAN
GENERAL ARRANGEMENT - SHEET 2 - DECK LAYOUT & SECTION
GENERAL ARRANGEMENT - SHEET 3 - TYPICAL SECTIONS AND DETAILS
ADJUSTMENT - SHEET 1

DESIGN DATA

- STRUCTURAL ELEMENT HAVE BEEN DESIGNED TO THE FOLLOWING CODES
AS 5100 BRIDGE DESIGN
DESIGN
ROADWAY
FLOOR DESIGN - TYPICAL VIB
FLOOD DESIGN LEVEL
FLOOD DESIGN LEVEL
BRIDGE ALSO DESIGNED FOR CATEGORY 620H WHEEL TRACTOR-SCRAPER AND HOVY 0141
DECK WEARING SURFACE: 30mm CRP AND SPRAY SEAL.
DESIGN LIFE FOR ADJUSTMENT, FOOTING, WHIRLWALLS & PSC DECK UNITS
100 YEARS

FOUNDATIONS

- REFER TO SOIL REPORT FOR GEOTECHNICAL INFORMATION ABOUT THIS SITE
FOUND ALL FOOTING ELEMENTS IN NATURAL MATERIAL WITH ALLOWABLE BEARING PRESSURES (FABP) AS FOLLOWS:
ELEMENT
STRIP FOOTINGS
BORED PILES
1800 kPa
ENGAGE A GEOTECHNICAL ENGINEER TO VERIFY SAFE ALLOWABLE BEARING PRESSURES BEFORE PLACEMENT OF BLINDING CONCRETE OR REINFORCEMENT.
ENSURE THE BASES OF ALL EXCAVATIONS ARE FREE OF ANY ORGANIC OR LOOSE MATERIAL AND WATER PRIOR TO PLACEMENT OF CONCRETE. CLEAN OUT BORED PILE BASES WITH A CLEANOUT BUCKET OR BY HAND.

CONCRETE

- ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH ASSS05 AND MS 10
DESIGN AND CONSTRUCT THE FORMWORK: IN ACCORDANCE WITH AS 3610.
CONCRETE SIZES AS DRAWN DO NOT INCLUDE APPLIED FINISHES AND MUST NOT BE REDUCED IN ANY WAY WITHOUT APPROVAL OF THE ENGINEER.
HANG NO HOLES, CRACKS OR EMBEDMENT OF PIPES IN CONCRETE MEMBERS OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS WITHOUT PRIOR APPROVAL OF THE ENGINEER.
SPACE CONDUITS CAST INTO CONCRETE MEMBERS AT MAX. DISTANCE POSITIVE AND UNDER NO CIRCUMSTANCES CLOSER THAN A CLEAR SPACING OF TWICE THE LARGER CONDUIT DIAMETER FROM PARALLEL REINFORCEMENT OR ANY OTHER CONDUIT. NO CONDUITS SHALL BE PLACED WITHIN THE COVER OF THE REINFORCEMENT.
PROPERLY FORM CONSTRUCTION JOINTS AND USE ONLY WHERE SHOWN OR SPECIFICALLY APPROVED BY THE ENGINEER. CAST CONSTRUCTION JOINTS VERTICAL OR HORIZONTAL AND APPROVED AND THOROUGHLY SCHEDULE THE INTERFACE OF HARDENED CONCRETE TO REMAIN ELEVATION.

- MAXIMUM CONCRETE AGGREGATE SIZE TO BE 20mm UNLESS STATED OTHERWISE
CONCRETE CHARACTERISTIC STRENGTHS (F_{CD}) AND COVERS SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE

ELEMENT	EXPOSURE CLASSIFICATION (mm)	COVER GRADE (mm)
BLINDING	B2	20
FOOTINGS	B2	40
INSTRUMENT	B2	45
ADJUSTMENTS	B2	50
PRECAST DECK UNITS	B2	35

- CONCRETE SLUMP SHALL BE 80 mm UNLESS STATED OTHERWISE
TEST CONCRETE USING THE PROJECT ASSIGNED METHOD IN ACCORDANCE WITH AS 1579.
TAKE AT LEAST 2 CYLINDERS FOR SAMPLE SUBMIT TEST RESULTS TO THE ENGINEER
WHEN CONCRETING DURING HOT (MORE THAN 25°C) AND/OR WINDY CONDITIONS, APPLY ALPHALITE ALCOHOL TO HORIZONTAL ELEMENTS AFTER MIN. SET.
INITIATE AN APPROVED METHOD OF CURING FOR ALL CONCRETE ELEMENTS AS SOON AS PRACTICABLE AFTER THE CONCRETE HAS SET AND MAINTAIN IT FOR AT LEAST 7 DAYS.
STRIPPING OF FORMWORK AND PROPPING SHALL BE IN ACCORDANCE WITH AS 3600 CLAUSE 11.2 UNLESS OTHERWISE APPROVED BY THE ENGINEER.

REINFORCING

- ALL WORKMANSHIP AND MATERIAL SHALL BE IN ACCORDANCE WITH MS 11
ABBREVIATIONS FOR STEEL GRADE AND TYPE
"R" - STRUCTURAL GRADE 230 PLAIN ROUND BAR TO AS 601.
"N" - GRADE 500B DEFORMED BAR TO AS/NZS 4471.
"W" - GRADE 500 PLAIN DEFORMED BAR TO AS/NZS 4471.
"W" - GRADE 500 STEEL WIRE TO AS 1530
REINFORCEMENT IS REPRESENTED DIMENSIONALLY AND NOT NECESSARILY SHOWN IN TRUE PROPORTION.
ENSURE THE CORRECT SPACING OF BARS IS MAINTAINED AND ALL REINFORCEMENT IS SUPPORTED IN ITS CORRECT POSITION BY APPROVED PLASTIC BAR CHAIRS. SPACERS OR SUPPORT BARS AT 800 MAX CRS IN BOTH DIRECTIONS.
PVC REINFORCEMENT WITH 125mm MAX DIA. ANNEALED WIRE TIES SUCH THAT IT IS SUPPORTED AND MAINTAINED IN POSITION WITHIN THE TOLERANCES GIVEN IN ASSS00 CLAUSE 11.3.3.
LAPS TO FABRIC REINFORCEMENT SHALL BE AT LEAST 250mm OR 2 CROSS LAPS ON EACH SHEET. 25mm LAPS TO TENSILE WIRE SHALL BE SHOWN MINIMUM AT SPICES AND FULL WIDTH AT INTERSECTIONS. LAPS TO BARS SHALL BE AS FOLLOWS

GRADE 500B BAR LAP LENGTHS	12	16	20	24	28	32	36
BAR DIAMETER	12	16	20	24	28	32	36
LAP LENGTH (TOP BARS)	440	550	660	770	880	990	1100
LAP LENGTH (ALL OTHERS)	440	500	560	620	680	740	800

BORED PILES

- DESIGN AND INSTALLATION OF THE PILES SHALL BE IN ACCORDANCE WITH THE DRAWINGS, SPECIFICATIONS AND ASSS05.
A LIMITED FOUNDATION INVESTIGATION IS AVAILABLE SHOULD ANY FURTHER INFORMATION BE REQUIRED THE CONTRACTOR SHALL ARRANGE THE WORK AT THEIR COST.
THE PILING CONTRACTOR SHALL ENGAGE A QUALIFIED GEOTECHNICAL ENGINEER TO DETERMINE THE PILE TOE LEVELS REQUIRED TO ACHIEVE THE NOMINATED DESIGN LOADS.
DESIGN THE PILES TO CARRY THE LOADS NOMINATED ON THE DRAWINGS IN ACCORDANCE WITH ASSS05 WITH 4% = 0.05.
BORED CAST IN-ITU REINFORCED CONCRETE PILES UNDER COLUMNS AND WALLS SHALL COMPLY WITH THE FOLLOWING:
MINIMUM CONCRETE GRADE F_{CD} = 32MPa WITH A MINIMUM CEMENT CONTENT OF 400kg/CUMETRE
CONCRETE SLUMP SHALL BE IN THE RANGE OF 100mm TO 120mm AT THE POINT OF DISCHARGE
MINIMUM CLEAN CONCRETE COVER TO REINFORCEMENT = 30mm
CONCRETE FOR THE PILES SHALL BE DROPPED OR AT LEAST 300mm ABOVE CUT OFF LEVEL

- THE CONTRACT SUM SHALL BE OPENED TO INCLUDE ALL COSTS ASSOCIATED WITH THE PILING IN THE CONDITIONS FOLLOWS. THE PRINCIPAL WILL NOT ACCEPT ANY CLAIMS ARISING FROM INCREASED COSTS IN PILING TO MEET THE PERFORMANCE REQUIREMENTS OF THE CONTRACT.
SAFE BEARING CAPACITY OF BORED PILES UNDER COLUMNS AND WALLS SHALL BE VERIFIED BY PILE TESTING OF NOT LESS THAN 2 NO. PILES. TEST PILES SHALL BE SELECTED BY THE ENGINEER.
A CERTIFICATE SHALL BE SUPPLIED BY A PRACTISING PRO. STRUCTURAL ENGINEER TO THE EFFECT THAT THE PILING DESIGN AND PILES AS INSTALLED IN THEIR SPECIFIED POSITIONS ARE STRUCTURALLY ADEQUATE TO CARRY THE SPECIFIED DESIGN LOADS.

PERFORMANCE - BASED PILING

- THE DESIGN, MANUFACTURE, INSTALLATION & TESTING OF THE PILES IS TO BE IN ACCORDANCE WITH THE DRAWINGS, SPECIFICATIONS AND ASSS05.
A LIMITED FOUNDATION INVESTIGATION IS AVAILABLE SHOULD ANY FURTHER INFORMATION BE REQUIRED THE CONTRACTOR SHALL ARRANGE THE WORK AT THEIR COST.
THE PILING CONTRACTOR SHALL BE RESPONSIBLE FOR SCHEDULING / QUANTIFYING THE VARIOUS PARAMETERS INCLUDING FOUNDING DEPTHS THAT AFFECT THE WORK IN PROGRESS.
ENGAGE A QUALIFIED GEOTECHNICAL ENGINEER TO DETERMINE THE PILE TOE LEVELS REQUIRED TO ACHIEVE THE NOMINATED DESIGN LOADS.
DESIGN THE PILES TO CARRY THE LOADS NOMINATED ON THE DRAWINGS IN ACCORDANCE WITH ASSS05 WITH 4% = 0.05. WHERE PERMISSIBLE STRESS DESIGN IS PERMITTED, THE FOLLOWING FACTORS OF SAFETY SHALL APPLY:
• STATIC ANALYSIS = 1.0
• DYNAMIC TESTING = 2.0
DESIGN THE PILES TO BE DURABLE FOR 50 YEARS WHEN EXPOSED TO THE GROUND CONDITIONS.
THE CONTRACT SUM SHALL BE OPENED TO INCLUDE ALL COSTS ASSOCIATED WITH THE PILING IN THE CONDITIONS FOLLOWS. THE PRINCIPAL WILL NOT ACCEPT ANY CLAIMS ARISING FROM INCREASED COSTS IN PILING TO MEET THE PERFORMANCE REQUIREMENTS OF THE CONTRACT.
POSITIONS OF SERVICES WHERE PROVIDED ARE GIVEN FOR GUIDANCE ONLY. LOCATION, DEPTH AND TYPE OF SERVICES SHALL BE DETERMINED BY THE CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE FOR SCHEDULING / QUANTIFYING THE VARIOUS PARAMETERS INCLUDING FOUNDING DEPTHS THAT AFFECT THE WORK IN PROGRESS.
PRELIMINARY DESIGN SERVICES SHALL BE PROVIDED TO A DEPTH OF 1m BELOW SERVICE LEVEL WITH 1% CEMENT MODIFIED SAND.
PRELIMINARY DESIGN SERVICES SHALL BE PROVIDED TO A DEPTH OF 1m BELOW SERVICE LEVEL WITH 1% CEMENT MODIFIED SAND.
BUILDING:
INSTALLATION TOLERANCE: PILE POSITION AT CUT OFF LEVEL:
VERTICALITY: 2%
IF THE PILES ARE CUT OFF TOLERANCE CUT OFF THE PILES TO THE LEVELS REQUIRED. DESIGN AND CONSTRUCT ANY NECESSARY MODIFICATIONS TO THE PILING OR PILECAPS. DOCUMENTED TO ENSURE THAT THE LOADS FROM THE SUPERSTRUCTURE ARE ADEQUATELY TRANSFERRED INTO THE PILES. THE COSTS OF ANY SUCH MODIFICATIONS SHALL BE BORNE BY THE CONTRACTOR.
TEST PILES AS FOLLOWS: STATIC PILE LOAD TESTS - ONE PER PILE SIZE AND DYNAMIC LOAD TESTS WITH CAPAN ANALYSIS - 5% OF EACH PILE SIZE. THE ENGINEER SHALL NOMINATE THE PILES TO BE TESTED.
PROVIDE A SURVEY PLAN COMPILED BY A REGISTERED SURVEYOR SHOWING THE AS BUILT LOCATION OF THE PILES RELATIVE TO THEIR DESIGN POSITION PRIOR TO CONSTRUCTION OF THE PILE CAPS. MEASURE LOCATIONS AT CUT OFF LEVEL.
SUPPLY A CERTIFICATE BY A PRACTISING PRO. STRUCTURAL ENGINEER TO THE EFFECT THAT THE PILING DESIGN AND PILES AS INSTALLED IN THEIR SPECIFIED POSITIONS ARE STRUCTURALLY ADEQUATE TO CARRY THE SPECIFIED DESIGN LOADS. PILES WILL NOT BE ACCEPTED UNTIL ENGINEERS CERTIFICATE AND SURVEY PLAN ARE SUPPLIED.

PLANS AND DOCUMENTS
referred to in the ULDA
Approval dated 16/9/11

APPROVED FOR
OPERATIONAL WORKS

Signed: *Michael Cooke*
MICHAEL COOKE RPEd 4948
For & on behalf of
Brown Consulting (Qld) Pty Ltd

LOCALITY PLAN
PROPOSED
BRIDGE
CALOUNDRA
AERODROME

BRIDGE DESIGN CRITERIA:

DESIGN CODE: AS5100:2004

DESIGN LOADING: SLM 600

DESIGN SPEED: 100

EARTHQUAKE ZONE: BECC-1

BRIDGE TYPE: PSC DECK UNITS

CLIENT
Stockland Caloundra Downs Pty Ltd

BROWN CONSULTING (QLD) PTY LTD ENGINEERS & MANAGERS
Level 6 100 Glen Street, Stockland Caloundra Downs
Caloundra Downs QLD 4570
Telephone: 07 5555 5544
Fax: 07 5555 5545
Email: info@brownconsulting.com.au
www.brownconsulting.com.au

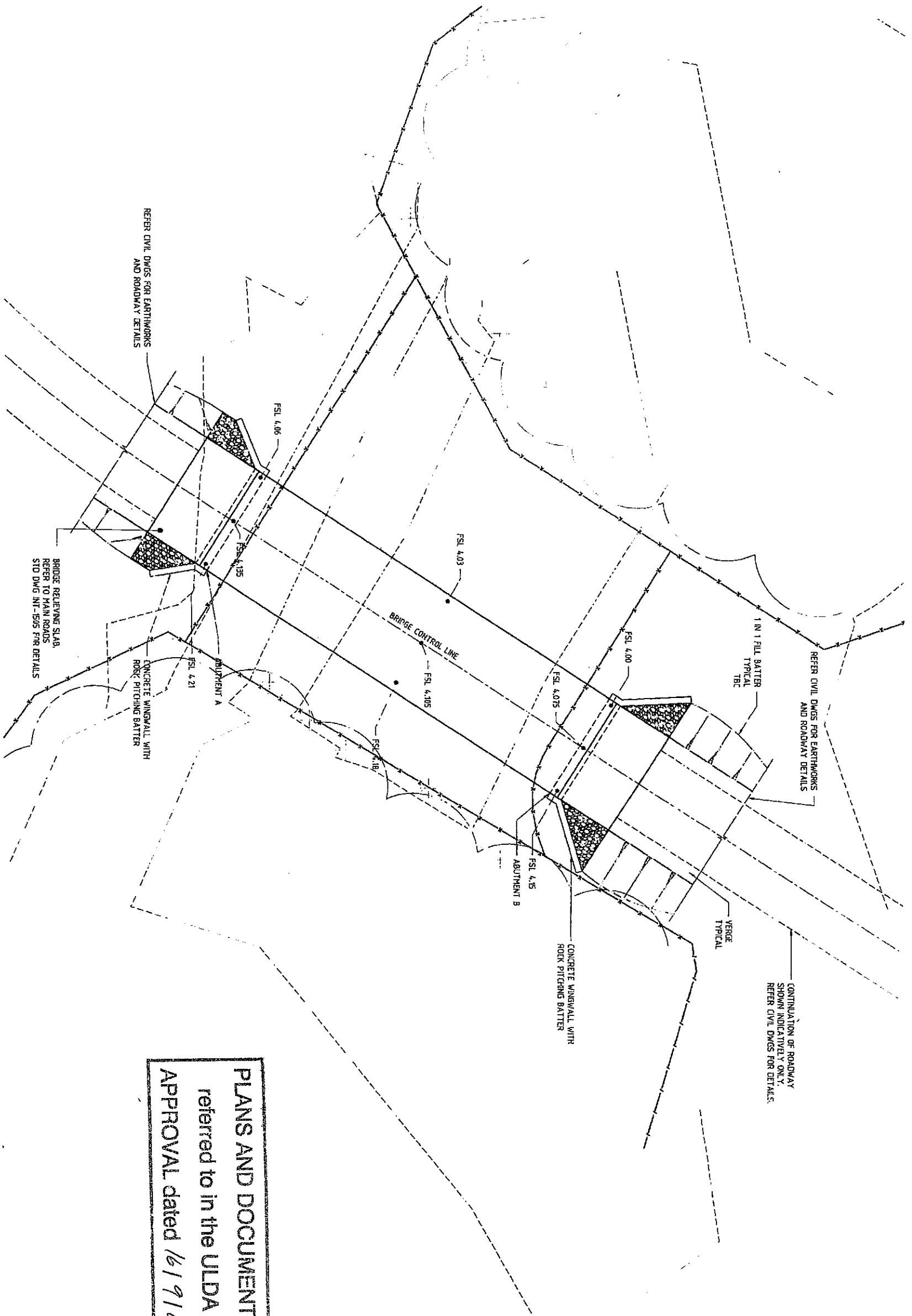
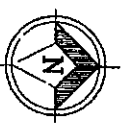
BELLVISTA II
TEMPORARY HAUL
ROAD BRIDGE
LAMEROUGH CREEK

PROJECT NO.
B11056

DRAWING TITLE
COVER SHEET, DRAWING INDEX &
STRUCTURAL NOTES

ISSUE NO.
S01

ISSUE NO.
2



LEGEND

— DENOTES PROPRIETARY BARRIER FENCE WITH NO ZONE SIGN TO MANUFACTURERS DETAIL.

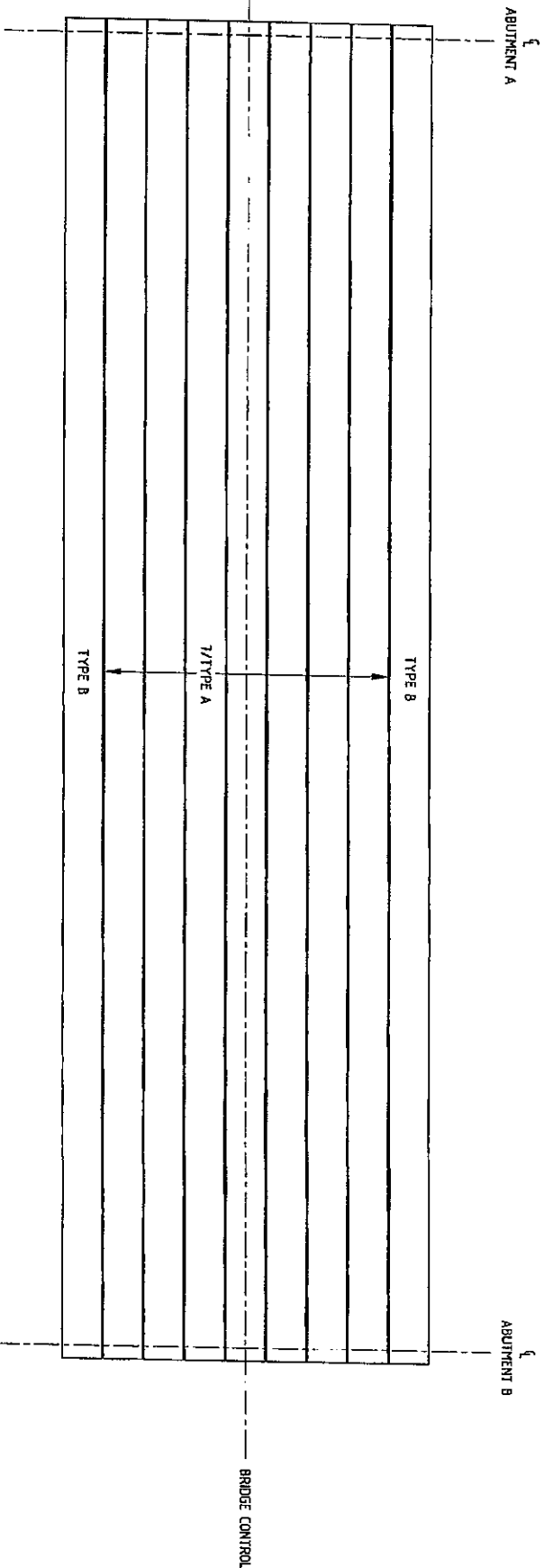
REFER S01 FOR STANDARD STRUCTURAL NOTES

PLANS AND DOCUMENTS
referred to in the ULDA
APPROVAL dated 16/9/11

APPROVED
Signed: *[Signature]*
MICHAEL COOKE RPEQ 4948
For & on behalf of
Brown Consulting (Qld) Pty Ltd

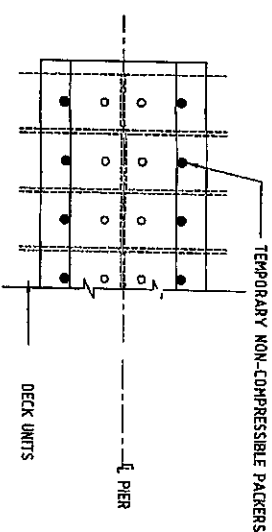
APPROVED FOR
OPERATIONAL WORKS

BRIDGE DESIGN CRITERIA:		DESIGN CODE: ASS100:2004		DESIGN LOADING: SM600		DESIGN SPEED: TBC.		EARTHQUAKE ZONE: BEQC-1		BRIDGE TYPE: PSC DECK UNITS	
GENERAL INFORMATION:		CLIENT:		PROJECT:		BROWNS PROJECT NO.		DRAWING TITLE:		SHEET:	
DRAWN: 10/10/11		Stockland Caloundra Downs Pty Ltd		BELLVISTA II TEMPORARY HAUL ROAD BRIDGE LAMEROUGH CREEK		B11056		GENERAL ARRANGEMENT - SHEET 1 BRIDGE LAYOUT PLAN		S10	
CHECKED: 10/10/11		BROWN CONSULTING (QD) PTY LTD ENGINEERS & ARCHITECTS		PROJECT:		BROWNS PROJECT NO.		DRAWING TITLE:		SHEET:	
DRAWN: 10/10/11		Stockland Caloundra Downs Pty Ltd		BELLVISTA II TEMPORARY HAUL ROAD BRIDGE LAMEROUGH CREEK		B11056		GENERAL ARRANGEMENT - SHEET 1 BRIDGE LAYOUT PLAN		S10	
CHECKED: 10/10/11		BROWN CONSULTING (QD) PTY LTD ENGINEERS & ARCHITECTS		PROJECT:		BROWNS PROJECT NO.		DRAWING TITLE:		SHEET:	
DRAWN: 10/10/11		Stockland Caloundra Downs Pty Ltd		BELLVISTA II TEMPORARY HAUL ROAD BRIDGE LAMEROUGH CREEK		B11056		GENERAL ARRANGEMENT - SHEET 1 BRIDGE LAYOUT PLAN		S10	



DECK UNIT LAYOUT DIAGRAM

SCALE 1:50



DECK UNIT ERECTION AT ABUTMENTS

NTS

CONSTRUCTION PROCEDURE

1. INSTALL NON-COMPRESSIBLE TEMPORARY PACKERS ON TOP OF THE HEADSTOCK, POSITIONED TO SUPPORT THE DECK UNITS. PACKERS TO BE OF SUFFICIENT STRENGTH TO SUPPORT THE WEIGHT OF THE DECK UNITS, AND OF SUCH A HEIGHT UNDER LOAD THAT THE SOFTEN OF THE DECK UNITS WILL CLEAR THE TOP OF THE BEAMS BY 2-3MM AT THE CLOSEST POINT.
2. IMMEDIATELY PRIOR TO LOWERING THE DECK UNITS APPLY AN APPROVED MORTAR TO THE ABUTMENT'S TOP SURFACE. THE THICKNESS OF THE MORTAR IS DETERMINED BY THE HOG OF THE DECK UNITS AND ANY GRADE ON THE BRIDGE. THE THICKNESS SHALL NOT EXCEED 5MM.
3. LOWER THE DECK UNITS CAREFULLY ON TO THE PACKERS ALLOWING THE EXCESS MORTAR TO BE DISPLACED AND CLEANED OFF IMMEDIATELY.
4. PUT HOLDING DOWN BOLTS IN PLACE BEFORE GROUTING THE BOLTS INTO THE HEADSTOCK.
5. PLACE THE FIBRE WASHERS AND SILKONE IN THE TOP OF THE HOLDING DOWN BOLT HOLES BEFORE INSTALLING THE WASHERS AND NUTS.
6. REMOVAL OF PACKERS TO TAKE PLACE A MINIMUM OF 48 HOURS AFTER PLACEMENT OF THE DECK UNITS.

STANDARD PLANS

DESCRIPTION	DWG NO	REVISION
RELIEVING SLAB 3m SPAN	--	A
STANDARD BAR SHAPES	1043	M
HOOK LAP & BEND DETAILS	1044	J
DATE PLATE	1053	G
DECK UNITS	1531	A

NOTES

1. THE FOUNDATION REPORT IS INCLUDED IN THE SCHEME DOCUMENTS. TENDERS ARE ABLE TO VIEW THE CORE SAMPLES, IF AVAILABLE, BY ARRANGEMENT WITH THE DISTRICT DIRECTOR.
2. NO FILLING TO BE PLACED ABOVE SOFTEN OF ABUTMENT HEADSTOCKS UNTIL AT LEAST SEVEN (7) DAYS AFTER ERECTION OF SPANS AND CASTING OF DECK.
3. REINFORCING STEEL TO BE IN ACCORDANCE WITH AS/NZS 4671.
4. DEFORMED BARS - GRADE 500N.
5. ROUND BARS - GRADE R250N.
6. ALL REINFORCING STEEL TO BE ACES CENTERED.

THE CONTRACTOR IS TO SUBMIT A CONSTRUCTION PROCEDURE TO THE SUPERINTENDENT WITH REGARD TO ENSURING THE STABILITY OF THE DECK UNITS DURING CONSTRUCTION.

PLANS AND DOCUMENTS
referred to in the ULDA
APPROVAL dated/6 / 19 / 11

APPROVED
Signed: *Michael Cooke*
MICHAEL COOKE RPEO 4968
For & on behalf of
Brown Consulting (Qld) Pty Ltd

REFER S01 FOR STANDARD STRUCTURAL NOTES

BRIDGE DESIGN CRITERIA:

DESIGN CODE: AS100:2004

DESIGN LOADING: SMT 600

DESIGN SPEED: 100.

EARTHQUAKE ZONE: BEC-1

BRIDGE TYPE: PSC DECK UNITS

ADDITIONAL DETAILS

CLIENT

Stockland Caloundra Downs Pty Ltd

BROWN CONSULTING (QLD) PTY LTD ENGINEERS & ARCHITECTS
Level 5, 109 City Street, South Brisbane QLD Australia 4101
Telephone: 07 3505 5400
Fax: 07 3505 5401
Email: info@brownconsulting.com.au
www.brownconsulting.com.au

BROWN

CONSULTING

PROJECT

BELLVISTA II
TEMPORARY HAUL
ROAD BRIDGE
LAMEROUGH CREEK

REF: H3 PROJECT NO.

B11056

DRAWING TITLE

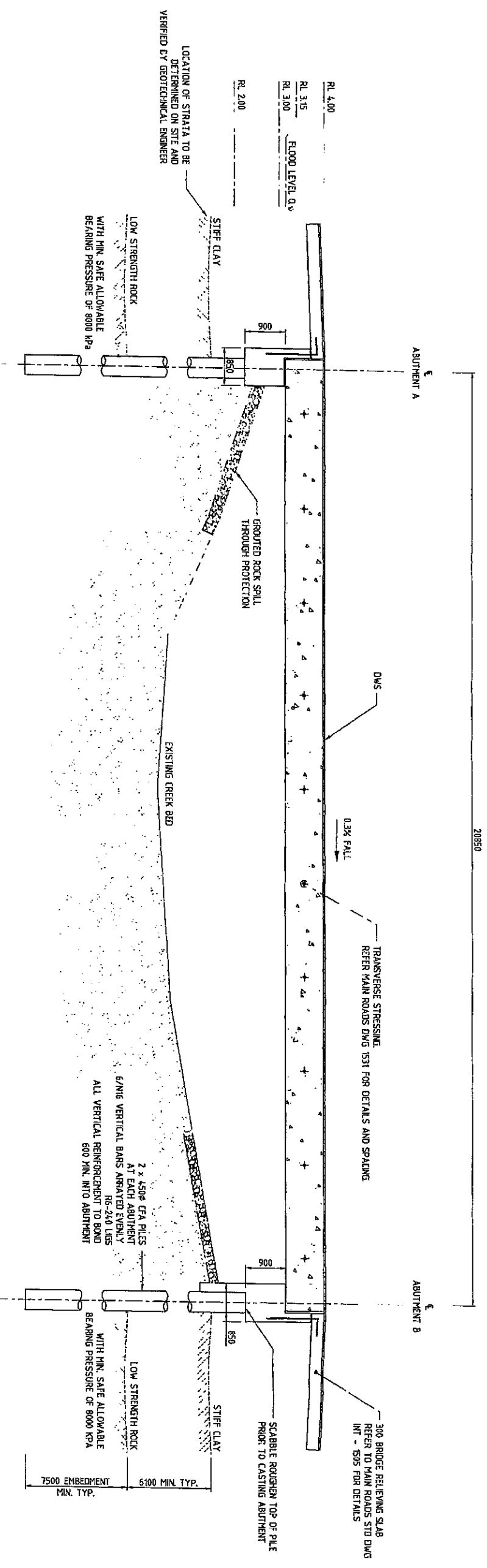
GENERAL ARRANGEMENT - SHEET 2
DECK LAYOUT & SECTION

DRAWING NO.

S11

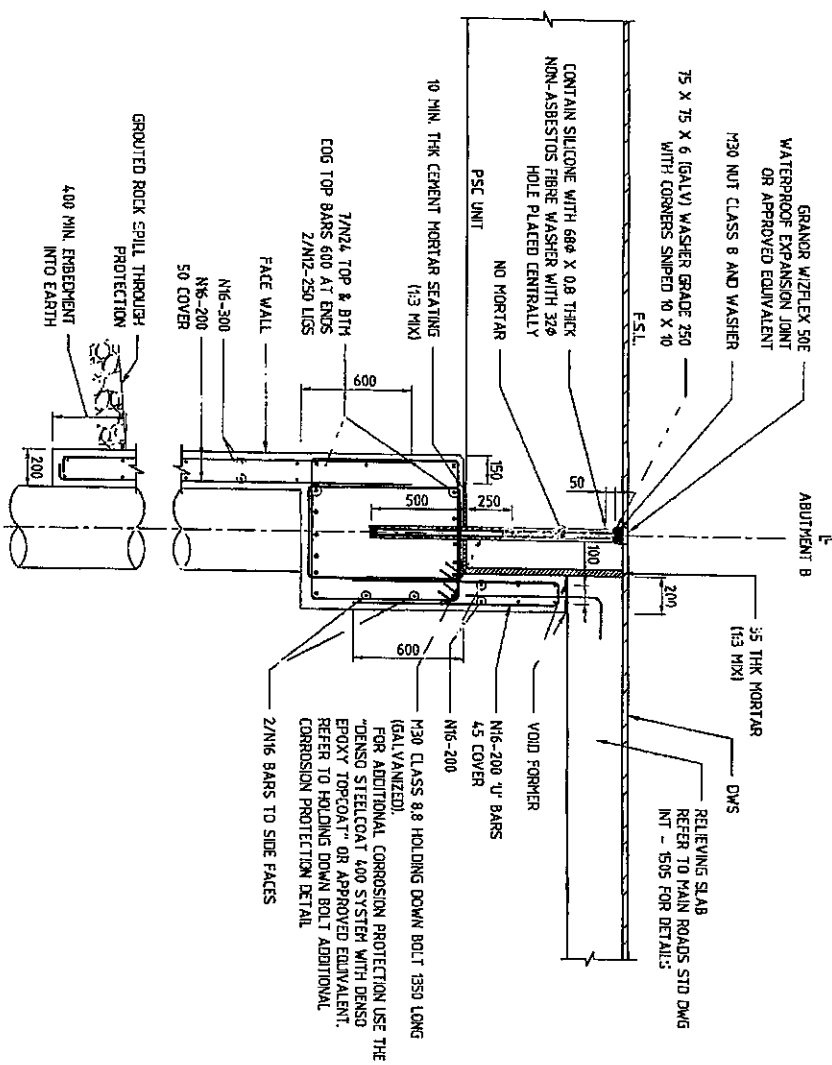
REVISION

2



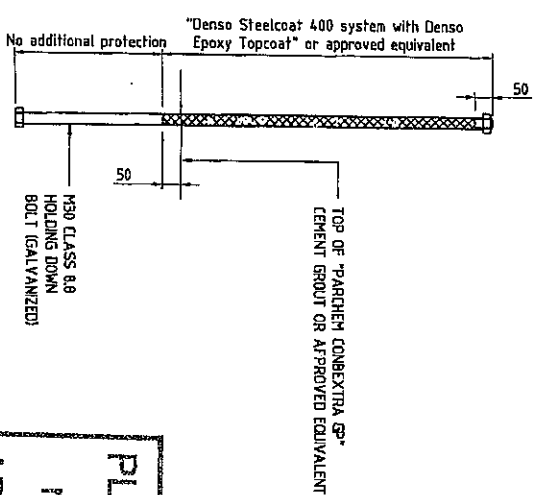
ELEVATION ALONG BRIDGE CONTROL LINE
SCALE 1:50

NOTE:
ALTERNATIVE PILING SYSTEM TO BE
CONFIRMED BY ENGINEER DESIGN FOR
ULTIMATE MAXIMUM COMPRESSIVE LOAD 2400 kN.



ABUTMENT B DETAIL (ABUTMENT A SIMILAR WITHOUT THE FACE WALL)
SCALE 1:20

HOLDING DOWN BOLT ADDITIONAL
CORROSION PROTECTION DETAIL
NTS



PLANS AND DOCUMENTS
referred to in the ULDA
APPROVAL dated 16/9/11

APPROVED
Signed: [Signature]
MICHAEL COOKE RPEd 42948
For & on behalf of
Brown Consulting (Qld) Pty Ltd

APPROVED FOR
OPERATIONAL WORKS

BRIDGE DESIGN CRITERIA:		DESIGN CODE: ASS1002004		DESIGN LOADING: SMI600		DESIGN SPEED: 70C		EARTHQUAKE ZONE: BEDC-1		BRIDGE TYPE: PSC DECK UNITS	
REFER S01 FOR STANDARD STRUCTURAL NOTES		ADJUSTMENT DETAILS		CLIENT		PROJECT		BR 3-735 PROJECT No.		BRIDGE TITLE	
STOCKLAND CALOUNDRIA DOWNS PTY LTD		STOCKLAND CALOUNDRIA DOWNS PTY LTD		BELL VISTA II		TEMPORARY HAUL		B11056		GENERAL ARRANGEMENT - SHEET 3	
BROWN CONSULTING (QLD) PTY LTD		BROWN CONSULTING (QLD) PTY LTD		LAMEROUGH CREEK		GENERAL ARRANGEMENT - SHEET 3		TYPICAL SECTIONS AND DETAILS		SHEET	
2		1		2		1		2		1	

