

PLANS AND DOCUMENTS  
referred to in the PDA  
DEVELOPMENT APPROVAL

Approval no: DEV2025/1602

Date: 27 May 2025



Water Engineering  
Partners

Water Engineering Partners Pty Ltd

## Lot 18B, Northshore Hamilton Flood Assessment

**Prepared for:** Silverstone Developments

**Date:** 13 March 2025

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*The flood modelling presented in this report is based and relies on survey data and other data obtained from third parties. While all reasonable steps have been taken to verify the data, Water Engineering Partners does not guarantee the data obtained or supplied for the investigation.*

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# 1 Introduction

Lot 18B Northshore Hamilton (Lot 6 on SP326594) is located at 260 Macarthur Avenue, Hamilton (the Site). The Site is located within the Northshore Hamilton PDA.

The proposed development of the Site includes ground level retail and a total of 253 apartments over two towers (Building 3 (South) and Building 4 (North)). The development includes:

- A basement providing 186 car park spaces, storage rooms and cold water pump room;
- Ground level comprising:
  - parking for visitors and retail (57 spaces);
  - Bicycle storage (328 spaces);
  - coworking area;
  - apartments (11); and
  - services.
- Level 1 podium with:
  - parking (92 spaces);
  - apartments (12); and
  - services.
- Level 2 podium with:
  - apartments (14); and
  - communal space.
- Building 3 (South) with 9 apartments on each of Levels 3 to 16; and
- Building 4 (North) with 9 apartments on each of Levels 3 to 12.

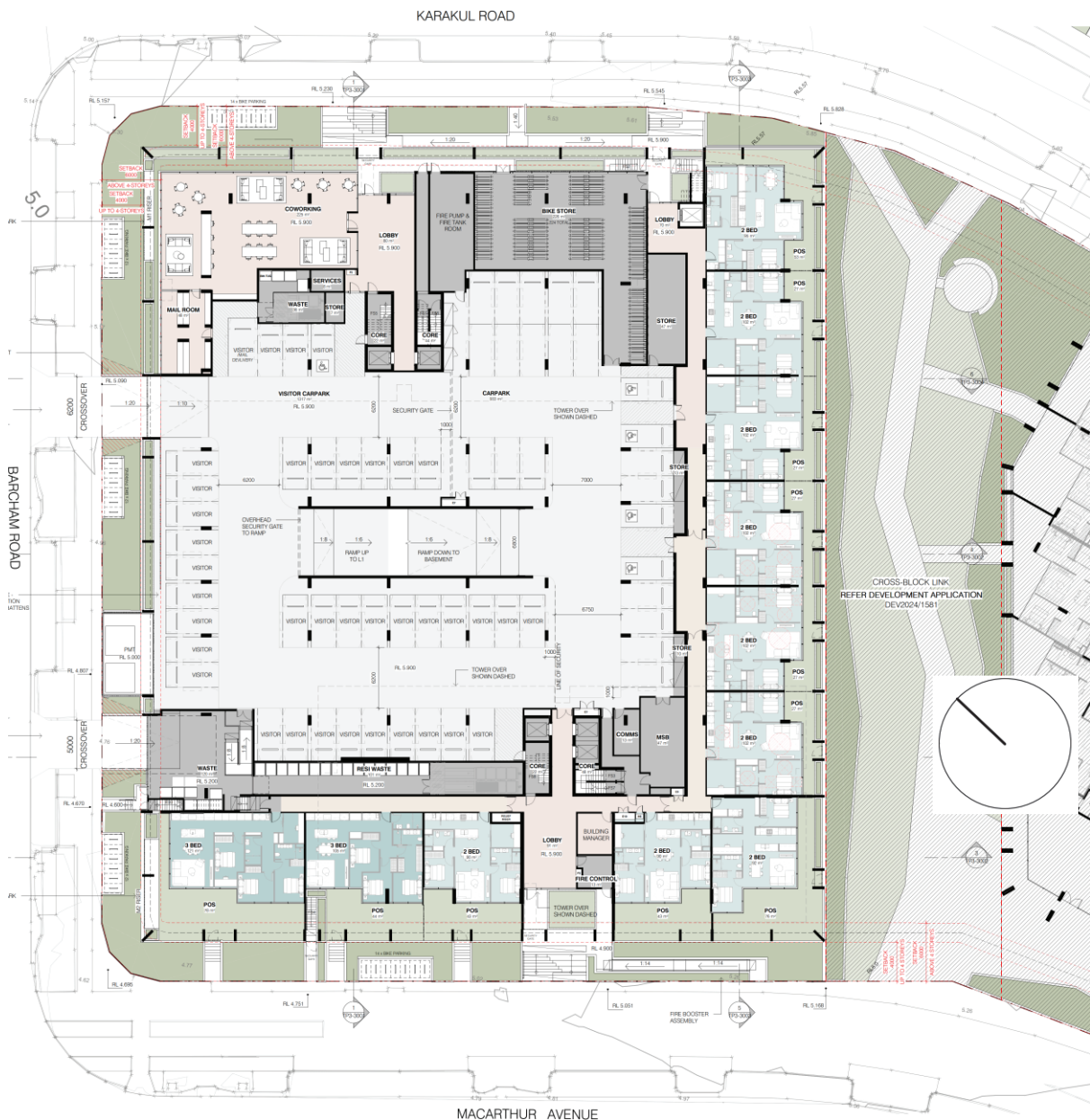
The location of the Site is shown on Figure 1-1.



**Figure 1-1 Site Location and Context Plan (Queensland Globe)**



The key drawings for the building, as provided by Carr Architects, are contained in Appendix A. The ground floor plan for the development (Carr Drawing TP3-1002, Rev TP1) is shown on Figure 1-2.



**Figure 1-2 Ground Floor Plan**

This report considers the various sources of flooding that could affect the Site and the potential impact of the development on flooding.

The site is potentially affected by inundation associated with Brisbane River flooding, local catchment overland flow, local street flow, and storm tide. Section 2 describes the various sources of inundation and the water levels applicable to the site for each source of inundation.

Section 3 details the potential impact of the development on flood levels.

Section 4 details the desirable flood immunity levels for the site with reference to guidance provided by the planning scheme of the local government area in which the PDA lies, namely *City Plan 2014* of Brisbane City Council. This section also details the higher immunity required to achieve immunity with respect to the 1% AEP event including climate change to 2100, which it is understood is the standard being adopted for the current design of Northshore Hamilton.

Section 5 provides an initial consideration of flood risk, noting the expectation that the preparation of a Flood Emergency Management Plan will be a condition of approval for the development.

## 2 Flooding and Storm Tide- Relevant Levels

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### 2.1 General

This section of the report details the potential sources of flooding that could affect the site and presents the flood levels associated with each type of flooding.

Overall, the Site is potentially affected by flooding from four sources:

- Brisbane River flooding;
- Flooding in the local catchment in which the Site lies;
- Flooding due to local flooding in the street system around the site; and
- Storm tides in Moreton Bay.

**To be consistent with current terminology, the severity of a flood is expressed in terms of its Annual Exceedance Probability (AEP). A 1% AEP event has a one percent probability of occurring in one year. This terminology replaces the previous reference to the Average Recurrence Interval (ARI) of an event. A 100-year Average Recurrence Interval event will occur, on average over a long period of time, once every 100 years. A 1% AEP event is equivalent to a 100-year ARI event.**

It is noted that the immunity standard applied under the Brisbane City Council *City Plan 2014* planning scheme varies according to the source of flooding and only fully considers climate change with respect to storm tide. In comparison, a higher design standard, namely the 1% AEP event including climate change to 2100, is being adopted within the PDA for the design of internal roads.

The flood levels nominated in this section refer to both the immunity standard applicable under the Council planning scheme and the higher design standard which is currently being applied within the PDA.

### 2.2 Brisbane River Flooding

The Brisbane River catchment has a total catchment area of about 13,500 km<sup>2</sup> to the Brisbane CBD.

The most recent comprehensive flood modelling of the Brisbane River, known as the Brisbane River Catchment Flood Study (BRCFS) was completed in response to the Queensland Floods Commission of Inquiry held subsequent to the January 2011 Brisbane River flood. The results of the modelling are summarised in the *Technical Summary Report, Comprehensive Hydrologic and Hydraulic Assessments, Brisbane River Catchment Flood Study (Revision 2, February 2017)* (the BRCFS Technical Report).

The flood model developed in support of the BRCFS Technical Report was used by Water Engineering Partners for this investigation under licence from the State Government.

This is consistent with the approach of Brisbane City Council, which has adopted the flood levels derived from the BRCFS study, albeit without climate change.

Section 8.3 of the BRCFS Technical Report detailed four climate change scenarios:

- CC1 – 0.3m sea level rise;
- CC2 – 0.3m sea level rise and 10% increase in rainfall;
- CC3 – 0.8m sea level rise; and
- CC4 – 0.8m sea level rise and 20% increase in rainfall.

The flood levels nominated with respect to climate change in this report reflect the CC4 scenario (equivalent to the RCP8.5 scenario at the time the Technical Report was prepared).

The inundation associated with the Brisbane River 1% AEP event including climate change to 2100 is shown on:

- Figure 2-1 Peak Flood Levels; and
- Figure 2-2: Peak Flood Depths.

With reference to the figures, ground levels at the Site and in the surrounding area are above the river flood level for the 1% AEP event including climate change.

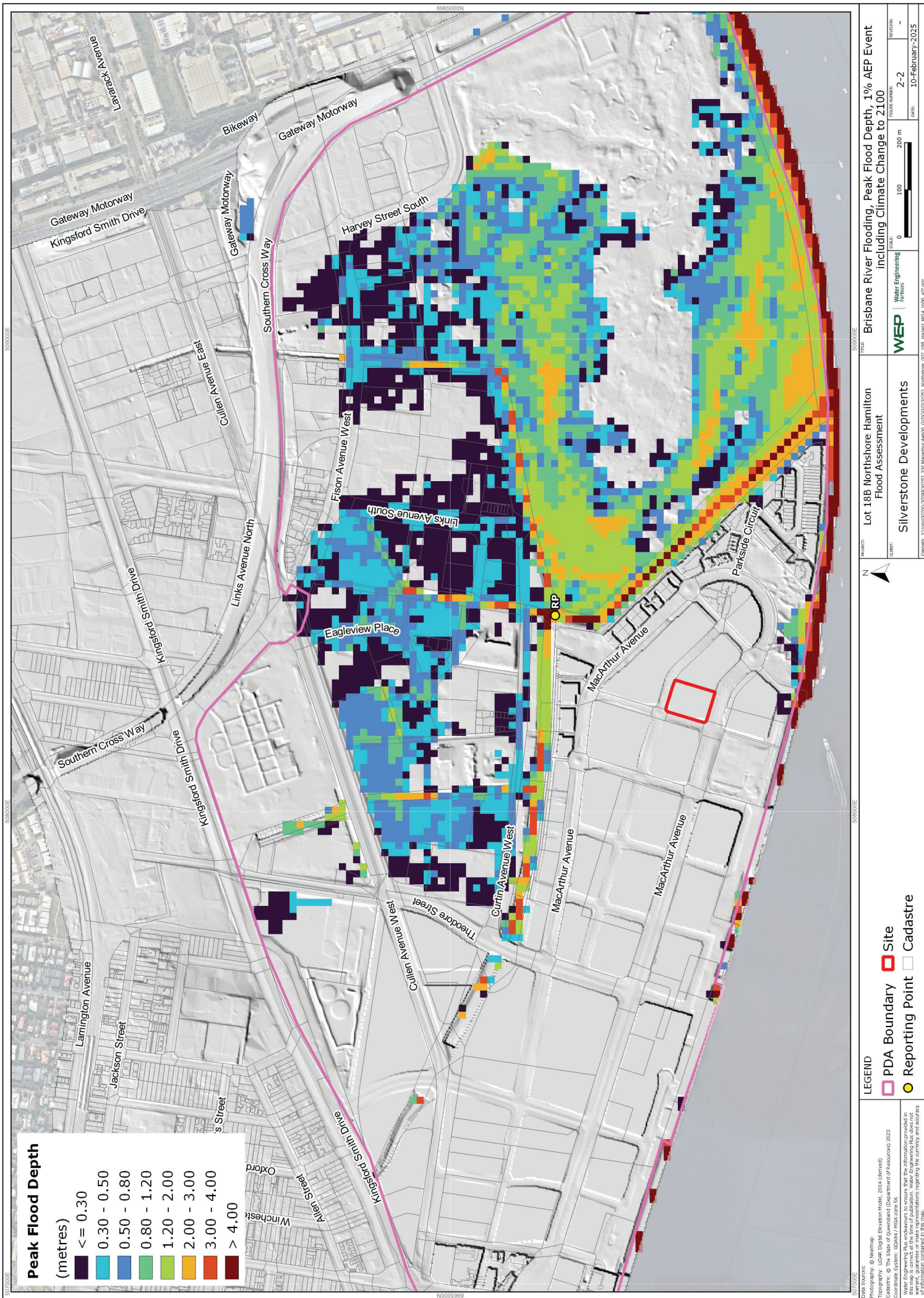
The flood level in the channel to the north of the Site therefore reflects the flood level at the point the channel joins the Brisbane River.

Table 2-1 presents the peak flood levels at the closest point to the Site, namely in the channel to the north of the Site (at the point denoted as “RP” in the figures). The flood level in the channel to the north of the Site reflects the flood level at the point the channel joins the Brisbane River.











## 2.3 Local Catchment Flooding

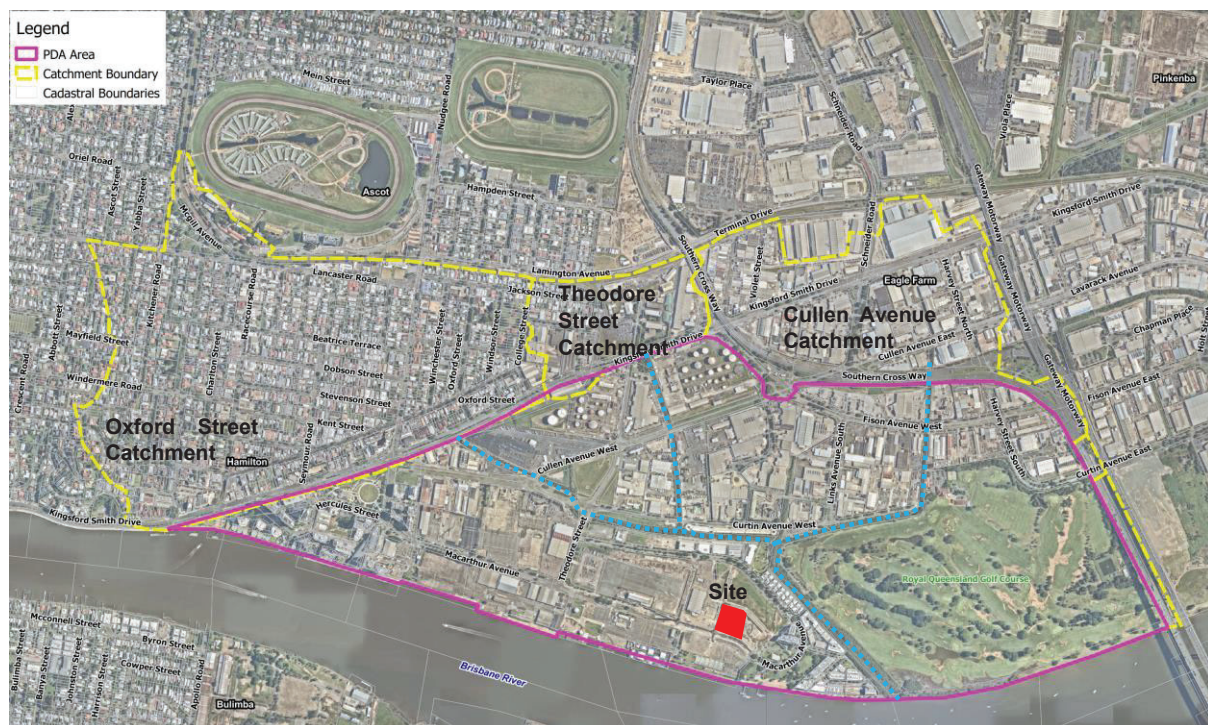
According to the BMT report *External Catchment Drainage Master Plan – Northshore Hamilton* (Revision 1, September 2017) (the Drainage Master Plan), the PDA is located within a local catchment with a total area of approximately 500 hectares (including the PDA).

Figure 2-3 shows the external local catchment areas (in yellow) relative to the PDA based on mapping prepared by consultants BMT. The figure also shows the open channel that drains runoff from the PDA to the Brisbane River.

Runoff from the external catchments (referred to as Oxford Street, Theodore Street and Cullen Avenue East in the BMT report) drain through the PDA to the Brisbane River via existing excavated open channels (shown in blue on Figure 2-3).

The total external catchment area to the PDA is 196 hectares, comprising:

- Oxford Street: 93.4 hectares;
- Theodore Street: 21.8 hectares; and
- Cullen Avenue East: 80.8 hectares.



**Figure 2-3 Extent of Local Catchment**

The model developed in support of the Drainage Master Plan is documented in Appendix B of the *BMT Hamilton Northshore Trunk Drainage Assessment* (Revision 3, September 2019).

BMT is completing flood modelling in support of the road design for the PDA (including the Olympic Village), based on the model developed for the Drainage Master Plan. At the time of preparation of this assessment, the final configuration of the master drainage solution for the PDA is yet to be finally adopted. It is therefore recommended that the flood levels associated with local catchment flooding be confirmed as part of detailed design.

As the developed case modelled by BMT represents the latest modelling of the local catchment (noting it remains to be finalised) and reflects the likely development within the PDA, it was considered appropriate to adopt the developed case model for the purposes of flood level derivation for the local catchment.

BMT provided model outputs for use in the study. Consequently, the flood levels presented in this report are based and rely on the flood model developed by BMT.

The inundation associated with the local catchment 1% AEP event including climate change to 2100 is shown on:

- Figure 2-4: Peak Flood Levels; and
- Figure 2-5: Peak Flood Depths.

With reference to the figures, it is important to note that the modelling includes the calculation of runoff from points within the PDA to the main channel that drains runoff to the Brisbane River. Based on the flood levels presented on Figure 2-4, the inundation of the local road network shown on the figures reflects runoff drained via the road network to the main channel.

Given the shallow nature of the runoff, the flood level in the road network adjacent to the Site is governed by the level of the road rather than the level in the channel.

Consequently, it is considered that in this case, the local catchment flood level of relevance to the determination of development levels is the level in the main channel adjacent to the Site.

Table 2-1 lists the peak flood levels in the main channel to the north of the Site (at the point denoted as “RP” in the figures).











## 2.4 Local Flooding- Street System

As the peak flood levels associated with local street system are governed by and vary according to the longitudinal grading of the road system, the flood levels associated with street runoff are not presented in Section 2.

Further, the freeboard requirements associated with street drainage differ from those associated with local catchment or river flow and are typically based on the recommendations of the *Queensland Urban Drainage Manual* (QUDM, 2017).

As noted in Section 2.3, the local catchment flood modelling completed by BMT included modelling of runoff in the local street system in the vicinity of the Site.

With reference to Figure 2-5, the calculated depths of flooding in the vicinity of the Site are less than 300 mm. Such depths are consistent with standard road design completed in accordance with guidelines such as Section 7.3.16 of QUDM.

Noting that the local catchment modelling considered a considerable area, the level of detail with respect to catchments draining to each part of the road system was necessarily limited. Consequently, recourse was made to design drawings prepared when the roads surrounding the Site were constructed to confirm that the stormwater drainage design for the roads was consistent with standard design practice and that therefore the depth of flooding in the road system is limited.

Most recently, SMEC completed the design of Karakul Road on the southern boundary of the Site, Figure 2-6 shows the internal catchment boundaries defined in the vicinity of the Site, including the subcatchments collecting runoff in Macarthur Avenue to the north. The figure has been annotated to show the approximate boundary of the Site and the location of the driveway to the basement via the ground floor.

Based on a review of the design drawings for Karakul Road and Barcham Road, it is noted that the road design was based on the 1% AEP event without climate change.

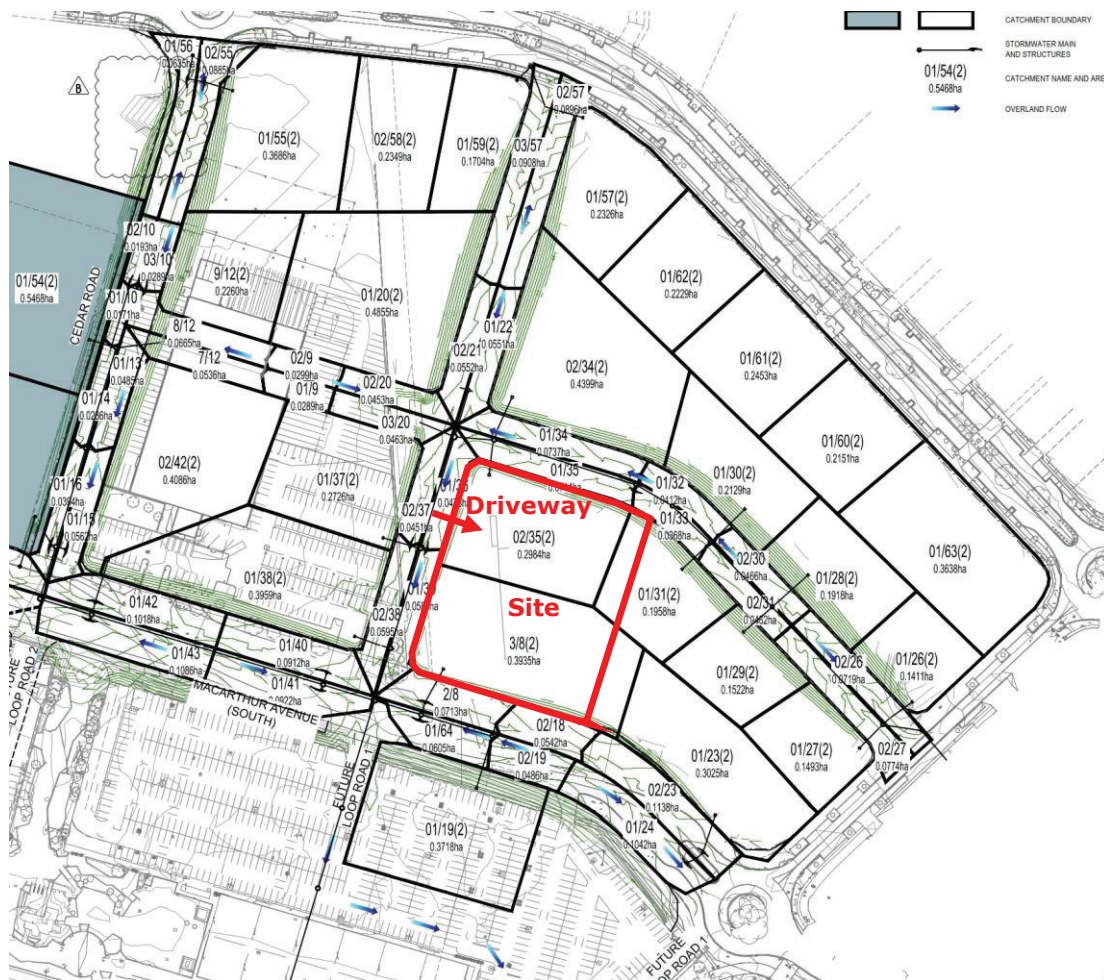
The stormwater drainage calculation sheets for the design of Karakul Road and Barcham Road indicate a maximum flow depth at key points in the vicinity of the site (driveway, substation and Karakul Road) of less than 100 mm for the 1% AEP event (SMEC Drawing 2521E-01-343 Rev A).

Based on a review of the road design, it is considered that the increased flow associated with a climate change scenario (i.e., adding 20% to design rainfall intensities) would result in depths less than 200 mm and generally be consistent with the results obtained from the local catchment modelling undertaken by BMT. Such levels are not expected to be problematic with respect to the Site.

Further, the level of Barcham Road at the driveway entrance to the site is about 800 mm higher than the low point at the intersection of Barcham Road and Macarthur Avenue. As development levels have been set relative to the higher levels in Karakul Road and Macarthur Avenue and the BMT modelling of the area indicates peak local flood depths less than 300 mm at the intersection of Barcham Road and Macarthur Avenue, it is also considered that the drainage of flow in Barcham Road and Karakul Road is not problematic with respect to the Site.

Section 4 includes the consideration of the immunity of the development with respect to street drainage.

Notwithstanding the above, as part of detailed design it is recommended that the capacity of the road and underground drainage system be confirmed to ensure that appropriate freeboard exists to all developed parts of the Site (refer Figure 1-2 and Section 4.2).



**Figure 2-6 Local Subcatchments (SMC Drawing 2521E-01-321 Rev B)**

## 2.5 Storm Tide

A storm surge is the increased water level that results from reduced atmospheric pressure and/or high velocity winds associated with tropical cyclones or intense low-pressure systems in Moreton Bay.

Where storm surge coincides with (high) astronomical tide, the associated increased water level constitutes a storm tide. The resultant storm tide can propagate from Moreton Bay upstream along the Brisbane River to the PDA.

The storm tide levels nominated by Brisbane City Council with respect to current day conditions ([https://www.data.brisbane.qld.gov.au/data/dataset/flood\\_awareness\\_storm\\_tide](https://www.data.brisbane.qld.gov.au/data/dataset/flood_awareness_storm_tide)) are listed in Table 2-1.

Table 8.2.6.3.C of the *Coastal Hazard Overlay Code of City Plan 2014* nominates a level of 3.1 mAHD for the 1% AEP storm tide event including climate change to 2100. It is understood that the climate change allowance is in accordance with the RCP8.5 pathway current at the time that the planning scheme was prepared.

To allow comparison to present day storm tide estimates, this value is also listed in Table 2-1.

The peak flood levels associated with the 1% AEP storm tide event including climate change to 2100 are shown on Table 2-1. A figure presenting water levels was not prepared due to the storm tide level being constant across the PDA.

With reference to the table, existing ground levels across the Site are above the water level associated with the 1% AEP storm tide event including climate change to 2100.



## 2.6 Summary

Table 2-1 presents a summary of the water levels associated with Brisbane River flooding, local catchment flooding and storm tide inundation.

**Table 2-1 Summary of Flood and Inundation Levels**

Event	Flood Level for Source of Flooding (mAHD)		
	Brisbane River Flooding	Local Catchment Flooding	Storm Tide
2% AEP		2.4	
1% AEP	2.1	2.5	2.5
1% AEP with climate change to 2100	3.1	2.8	3.1

*Note: For river and local catchment flooding, nominated flood levels refer to conditions in the main channel that drains the PDA at a point close to the Site.*

### 3 Potential Impact of Development

---

The potential impact of development on flooding and storm tide inundation was assessed as follows:

- **Brisbane River**

Existing ground levels around the perimeter of the Site are higher than the flood level associated with the 1% AEP Brisbane River flood event including climate change to 2100. Consequently, the development will not impact on flood levels in the Brisbane River for events up to this magnitude.

- **Local Catchment.**

Existing ground levels around the perimeter of the Site are higher than the flood level associated with the 1% AEP local catchment flood event including climate change to 2100. Consequently, the development will not impact flood levels in the main channel draining the local catchment for local events up to this magnitude.

- **Local Street Runoff**

It is considered that the design of the road system effectively contains street runoff to the road reserve. As a consequence, and noting that the ground level car park will be over 800 mm higher than the road level at the driveway, it is considered that development of the Site (in terms of Site earthworks and building footprint) will not adversely impact on local street runoff.

- **Storm Tide**

Filling of land does not affect the level reached by storm tide as the storm tide level is governed by conditions in Moreton Bay and it is typically assumed that flow propagates inland to match the storm tide level in the Bay.

In any case, existing ground levels around the perimeter of the Site are above the water level associated with the 1% AEP storm tide.

Given the above, it is considered that the development will not adversely impact flood or inundation levels to the 1% AEP event standard including climate change to 2100.

## 4 Development Levels

### 4.1 Minimum Development Levels

#### 4.1.1 City Plan 2014

For comparative purposes, desirable development levels according to the Brisbane City Council *City Plan 2014* planning scheme were determined as follows:

- Flooding (Brisbane River and Overland Flow)
  - Flood planning categories: Table 8.2.11.3.D, Table 9.4.9.3.C
  - Categories of flood planning levels: Table 8.2.11.3.L, Table 9.4.9.3.B
- Coastal Hazard (Storm Tide)
  - Flood planning level categories: Table 8.2.6.3.D
  - Categories of flood planning levels: Table 8.2.6.3.C

The immunity standard applied for flooding and storm tide under *City Plan 2014* varies according to the source of inundation being considered:

- Brisbane River: 1% AEP event without climate change;
- Local Catchment: 2% AEP event without climate change; and
- Storm Tide: 1% AEP event including climate change to 2100.

Based on the categories and flood level information presented in the tables nominated above and the flood level information presented in *City Plan 2014*, the minimum development levels applicable to the Site are listed in Table 4-1 based on the levels nominated in Table 2-1. The governing level in each case is shaded.

**Table 4-1 Minimum Development Levels – Brisbane City Council Planning Scheme**

Area	Category	Brisbane River <sup>1</sup>		Local Catchment		Coastal/ Storm Tide	
		Standard	Level (mAHD)	Standard	Level (mAHD)	Standard	Level (mAHD)
Residential							
Habitable room	A	RFL+500mm	2.6	2%AEP+500mm	2.9	3.1+0.5	3.6
Non-habitable room/lobby	B	RFL+300mm	2.4	2%AEP+300mm	2.7	3.1+0.3	3.4
Essential electrical services	A	RFL+500mm	2.6	2%AEP+500mm	2.9	3.1+0.5	3.6
Vehicular manoeuvring	D	5% AEP	<2.1	2% AEP	2.4	2% AEP	2.2
Basement entry	C+300	DFL+300mm	2.4	2%AEP+300mm	2.7	3.1+0.3	3.4
Commercial							
Floor Level	C	DFL	2.1	2% AEP	2.4	3.1	3.1
Essential electrical services	A	RFL+500mm	2.6	2%AEP+500mm	2.9	3.1+0.5	3.6
Vehicular manoeuvring	D	5% AEP	<2.1	2% AEP	2.4	2% AEP	2.2
Basement entry	C	DFL	2.1	2% AEP	2.4	3.1	3.1

Notes: 1. Development levels nominated based on 1% AEP (100-year ARI) flood level from new Brisbane River Catchment Flood Study as the RFL (Residential Flood Level), and also conservatively used for the DFL (Defined Flood Level) value.

## 4.1.2 PDA Road Design Standard

Development levels for the current design of the PDA road system are set relative to a higher immunity standard than the standard adopted for the Brisbane City Council planning scheme (i.e., the 1% AEP event including climate change to 2100 regardless of the source of inundation).

Table 4-2 lists the minimum development levels applicable to the Site for the higher standard. For the purposes of the table, the freeboard requirements nominated by Brisbane Council were adopted. The governing level in each case is shaded.

The table also lists requirements for commercial development for completeness and to demonstrate compliance, noting that the manager's office and coworking area on the ground floor and the wellness centre on Level 2 podium are considered to be ancillary to the residential use.

**Table 4-2 Minimum Development Levels – PDA Road Design Standard**

Area	Category	Brisbane River		Local Catchment		Coastal/ Storm Tide	
		Freeboard	Level (mAHD)	Freeboard	Level (mAHD)	Freeboard	Level (mAHD)
Residential							
Habitable room	A	500mm	3.6	500mm	3.3	500mm	3.6
Non-habitable room/lobby	B	300mm	3.4	300mm	3.1	300mm	3.4
Essential electrical services	A	500mm	3.6	500mm	3.3	500mm	3.6
Vehicle Manoeuvring	D	0	3.1	0	3.1	0	3.1
Basement entry	C+300	300mm	3.4	300mm	3.1	300mm	3.4
Commercial							
Floor Level	C	0	3.1	0	3.1	0	3.1
Essential electrical services	A	500mm	3.6	500mm	3.3	500mm	3.6
Vehicular manoeuvring	D	0	3.1	0	3.1	0	3.1
Basement entry	C	0	3.1	0	3.1	0	3.1

## 4.2 Comparison to Proposed Development Levels

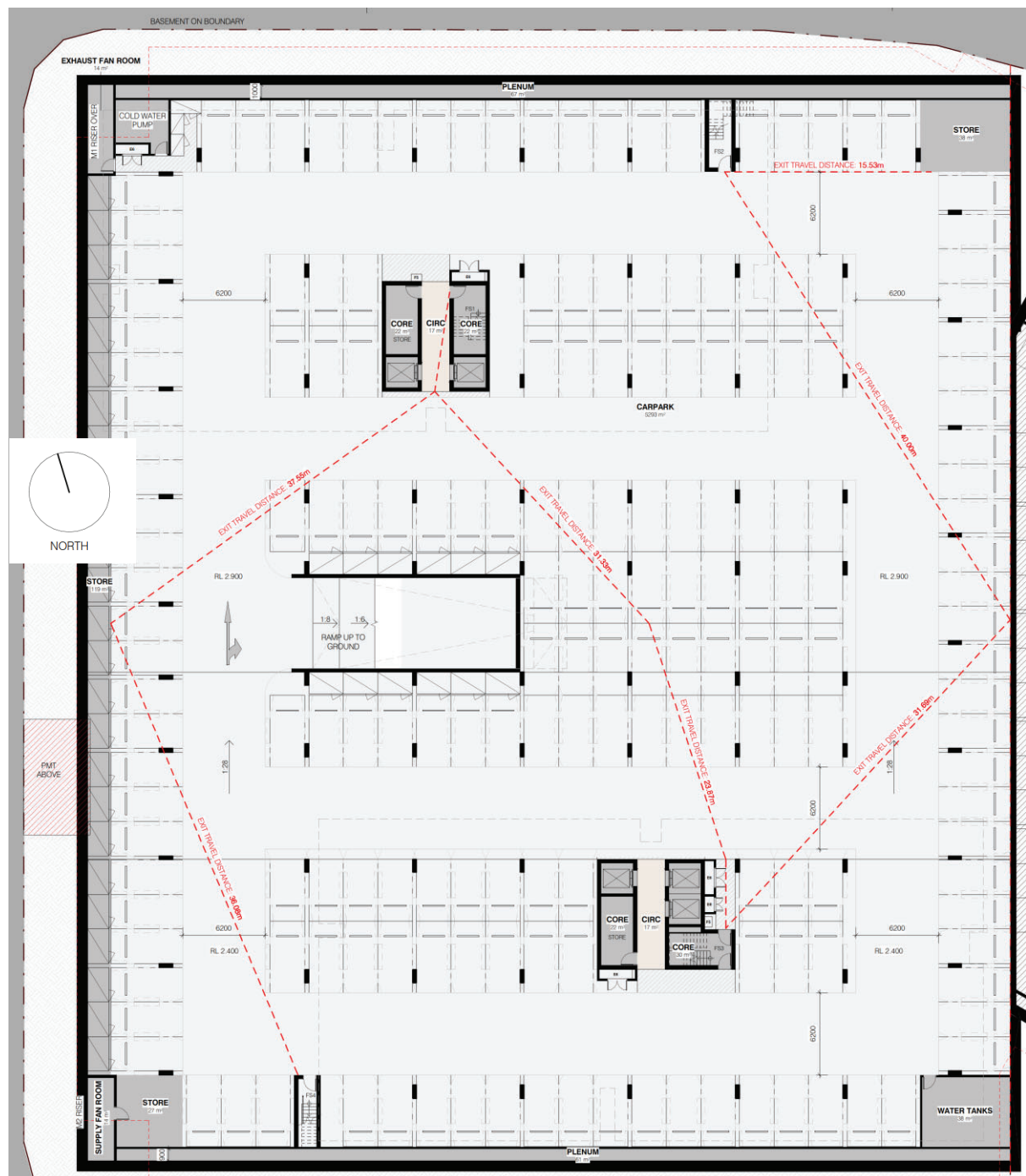
The Carr Architects plan TP3-1002 Rev TP1 (refer Appendix A) nominates a ground floor level for the coworking area, parking and apartments of 5.9 mAHD, with a level of 5.2 mAHD nominated for the waste management area.

Based on this level, the compliance of the development to the nominated minimum development levels is summarised below.

- Basements**

There is no immunity requirement for the floor level of the basement. However, it will be necessary to ensure that the minimum level at each point of entry to the basement meets the requirements nominated in Table 2-1 and in Table 4-1. In this case (refer to discussion of essential electrical services below) it is desirable to adopt a minimum access level of 3.6 mAHD.

Figure 4-1 presents an excerpt from the basement plan for the development (Carr Architects Drawing TP3-1001 Rev TP1).



**Figure 4-1 Basement Level Plan**

Based on Carr Architects Drawing TP3-1002 Rev TP1, the driveway entrance to the basement will be set at 5.9 mAHd, which is well above the requisite minimum level (i.e., 3.6 mAHd).

Similarly, stairwell and lift access points to ground level will have a minimum level of 5.9 mAHd. Again, this level is well above the requisite minimum level.

Finally, it is noted that as part of detailed design it will be necessary to ensure that any vents to the basement satisfy the basement entry immunity requirement. However, based on Carr Architects Drawing TP3-1002 Rev TP1, existing ground levels around the perimeter of the Site are typically in excess of 5.1 mAHD. As this level is above the minimum requirement, it is expected that any venting for the basement will satisfy immunity requirements.

- **Residential- Habitable Rooms**

The lowest habitable rooms are on the Ground floor level (5.9 mAHD). This level is 2.3 metres higher than the minimum requirement with respect to river and local catchment flooding.

Existing levels in Barcham Road vary between about 4.5 and 5.1 mAHD. Existing levels in Karakul Road vary between about 5.1 and 5.6 mAHD. From a comparison of levels in Barcham Road and Karakul Road, the Ground floor level will be generally well above the level in Barcham Road and Karakul Road for the majority of the frontage of the Site. It is recommended that the freeboard at the north-eastern end of the Site at Karakul Road be verified (with localised measures adopted if necessary) to ensure adequate freeboard in that corner of the Site.

- **Residential- Non-Habitable Rooms**

The lowest non-habitable rooms and the coworking area are on the Ground floor level (5.9 mAHD). Similar to the case for habitable rooms, the level is over 2 metres higher than the minimum requirement.

Similarly, although the level of the waste management area (5.2 mAHD) is lower than the general floor level, it is well above the minimum level requirement and acceptable.

- **Essential Electrical Services**

According to Note 2 of Table 8.2.11.3.D of *City Plan 2014*, essential electrical services include ‘any area or room used for fire control panel, telephone PABX, sensitive substation equipment including transformers, low voltage switch gear, high voltage switch gear, battery chargers, protection control and communication equipment, low voltage cables, high voltage cables, and lift or pump controls’.

- *Basement Pump Room*

A pump room is located in the basement (refer Figure 4-1) which is considered to be an essential service. It is therefore necessary for all points of entry to the basement to be at a level of 3.6 mAHD or higher.

Based on the above discussion with regard to the basement, all points of entry will be set at a level 2 metres higher than 3.6 mAHD. Therefore, it will be possible to accommodate essential electrical services in the services area of the basement as required.

- *Main Communications, switchboard and fire pumps*

The Carr Architects Drawing TP3-1002 Rev TP1 identifies the main communications/NBN, main switchboard and fire pump room as being located on ground level (i.e., 5.9 mAHD).

As this level is well above the minimum required immunity level (i.e., 3.6 mAHD), the ground level services will have a level of flood immunity well in excess of that nominally required.

- *Substation*

- a) *Immunity to river and local catchment flooding*

The electrical connection to the Site will be via a substation located adjacent to the driveway on Barcham Road (refer Figure 1-2). The level nominated for the substation is 5 mAHD.



This level is well in excess of the minimum required immunity level with respect to river and local catchment flooding (i.e., 3.6 mAHD).

However, given that flooding of the substation would render the building inoperable, it is recommended that the ability to adopt a higher level for the substation (for example, setting it at the ground floor level of 5.9 mAHD if possible) be considered as part of detailed design.

*b) Immunity to water levels in local street system*

The nominated level for the substation is over 450 mm higher than the invert level of Barcham Road at this location. Based on the drainage calculations presented in the design drawings for Barcham Road (SMEC Drawings 2521E-01-343 Rev A and 2521E-01-344 Rev A for the drainage network presented on Drawing 2521E-02-301 Rev D), the adopted level should provide a freeboard in excess of 300 mm to the water level associated with local street drainage and thereby satisfy the recommendations of Section 7.3.16 of the *Queensland Urban Drainage Manual* (2017) for immunity.

It is recommended that detailed calculations of the capacity of the local drainage system be completed as part of detailed design to confirm the freeboard to the substation.

## 5 Flood Risk Management

### 5.1 Flood Risk- Rare and Extreme Events

Based on the immunity of the road system fronting the site with respect to river and local catchment flooding and storm tide inundation, it is considered that the immunity of the access to and from the Site via the surrounding road network is acceptable with respect to normal design standards.

While there can be a depth of flow associated with street drainage, this is a standard outcome for road design in greenfield sites under QUDM.

However, extreme flood events (i.e., events in excess of the design standard) could result in the inundation of the surrounding road network and the Site. The potential for this to occur is discussed below.

- **Brisbane River**

The peak flood levels at the site associated with Brisbane River flooding are listed in Table 5-1 for a range of rare to extreme events. For each event, the AEP and its corresponding ARI are nominated. It is noted that the 100,000 AEP event can be considered as being equivalent to the largest flood that could conceivably occur in the Brisbane River (the Probable Maximum Flood, or PMF).

Peak flood levels associated with flooding in the Brisbane River were extracted from the results of the *Brisbane River Catchment Flood Study*, (refer Section 2.2).

**Table 5-1 Peak Flood Levels, Rare and Extreme Events in Brisbane River**

Event	Level (mAHD)
1% AEP (100-year ARI)	2.1
0.5% AEP (200-year ARI)	2.5
0.2% AEP (500-year ARI)	2.9
1% AEP with climate change (100-year ARI)	3.1
0.05% AEP (2,000-year ARI)	3.8
0.01% AEP (10,000-year ARI)	6.1
0.001% AEP (100,000-year ARI), PMF	9.4

With reference to Table 5-1, the adopted basement entry and ground floor levels will provide a very high level of immunity (in excess of the 0.05% AEP (2,000-year ARI) and close to the 0.01% AEP (10,000-year ARI) for the basement and ground level. It is expected that Podium Level 1 will be at the level of the PMF in the Brisbane River, with levels 2 and higher above the PMF level.

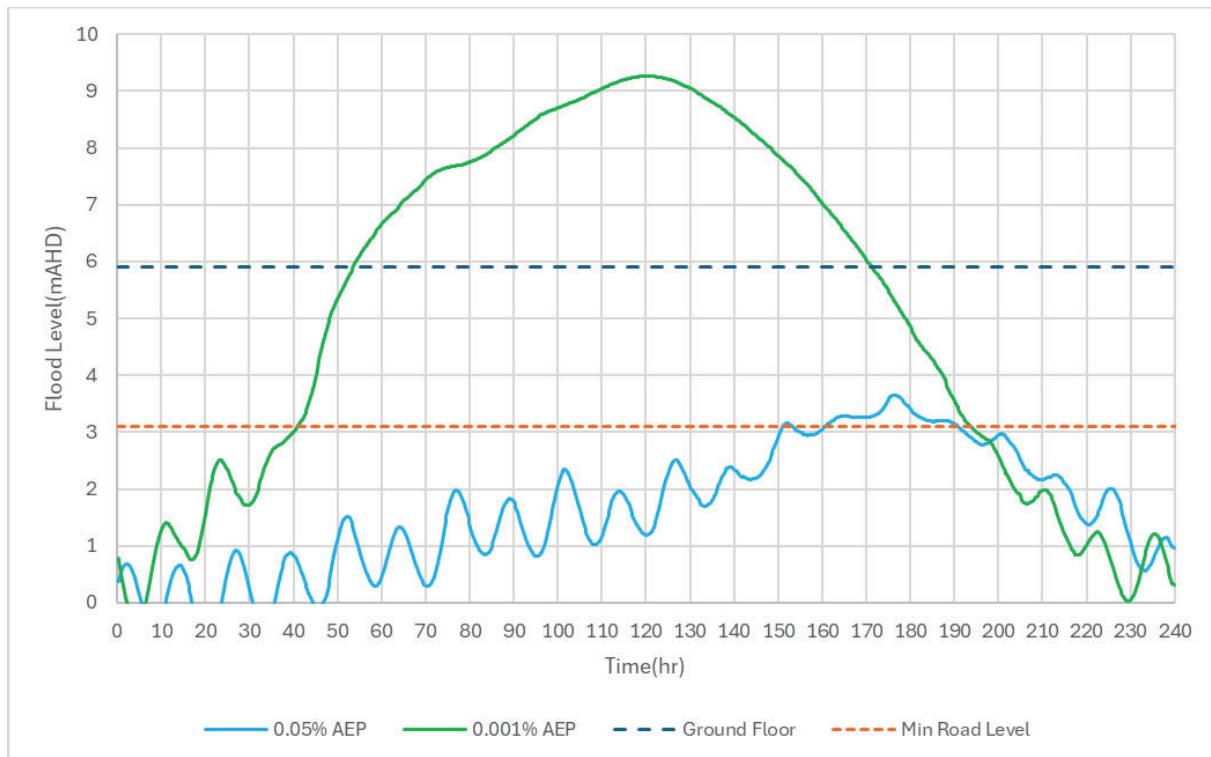
Although the Site has a high level of immunity, extreme flood events could result in the isolation of the Site for a considerable period due to the inundation of the road network that gives access to the Site.

Figure 5-1 presents the variation in flood level over time in the Brisbane River at the Site for the following events:

- 0.05% AEP (2,000-year) event; and
- 0.001% AEP (100,000-year or PMF) event.

In the case of the 0.05% AEP event, as the water level in the river is not sufficiently high to inundate the Site, the level in the river at a point close to the mouth of the channel that drains the local catchment was selected.

The figure also shows the adopted Ground Floor level (5.9 mAH) and the minimum road design level within the PDA (3.1 mAH).



**Figure 5-1 Variation in Flood Level over Time, Brisbane River**

With reference to the figure, the period of isolation will increase with the increasing severity of the event being considered.

For the 0.05% AEP (2,000-year ARI) event, the road network around the Site would be inundated for a period of about 30 hours.

For the PMF event:

- the road network around the Site would be inundated for a period of about 153 hours; and
- the Ground Floor would be inundated for a period of about 118 hours.

It is considered that the above periods of isolation are such that it would be desirable to evacuate the Site prior to its isolation even though it would be possible to shelter above flood level.

According to the Bureau of Meteorology *Flood Warning System for the Brisbane River below Wivenhoe Dam to Brisbane City* ([http://www.bom.gov.au/qld/flood/brochures/brisbane\\_lower/brisbane\\_lower.shtml](http://www.bom.gov.au/qld/flood/brochures/brisbane_lower/brisbane_lower.shtml)), 'Average catchment rainfalls in excess of 200-300mm in 48 hours, may result in stream rises and the possibility of moderate to major flooding and local traffic disabilities throughout the Brisbane River catchment.' Given the period over which rainfall is required in order to cause flooding in the Brisbane River, it is considered that ample time is available to evacuate in advance of flooding.

Further, major flooding in the Brisbane River is accompanied by significant warnings from the State Government. These warnings can be used to trigger evacuation of the Site.

If a shelter in place strategy were to be contemplated as part of further design, it would be necessary to ensure that sufficient provisions could be provided for the period of isolation, together with sufficient infrastructure (water (including any requirements for firefighting, sewer and power (with the substation raised to the level of the ground floor to minimise the potential for inundation)). Further, even then it would be desirable for people with medical conditions to be allowed to evacuate prior to the site being isolated.

- **Local Catchment**

For the local catchment, modelling of rare and extreme events was undertaken by consultants BMT in April 2024. The results of this modelling were supplied to Water Engineering Partners. However, similar to the outcome for the 1% AEP event, the flood levels in the Macarthur Avenue and Karakul Road reflect relatively shallow depths of flooding in the street system from runoff draining to the main channel rather than the flood level in the main channel.

As a consequence of the relatively shallow flow in the road system, the variation in flood level as the severity of flooding increases is minimal. For example, for the PMF event, the maximum calculated flood level in Karakul Street is 5.9 mAHD.

The calculated flood level is dependent on the underlying road surface and therefore the representation of the road surface in the flood model and the discretisation of the catchments within the PDA. There could consequently be a small variation in this level if more detailed modelling was undertaken. However, the modelling is sufficient to indicate that the ground floor (5.9 mAHD) will be at a level similar to the PMF level for local catchment flooding.

Similarly, as the basement is accessed via the ground level, a PMF local catchment event would only result in limited if any water entering the basement level.

Given this, it is considered that the Site (other than the substation on Macarthur Avenue) will either not be inundated or only inundated to a minor extent during extreme events in the local catchment.

Further, given the relatively small size of the local catchment, it is considered that any inundation would be for a limited period.

The relatively small size of the local catchment will also result in the warning time associated with inundation to be short (of the order of 30 minutes). Such a period is not sufficient to provide for evacuation. However, due to the limited inundation of the Site, it will be possible to shelter on Site while local catchment flood events are in progress.

In summary:

- **Brisbane River**
  - Very rare events in the Brisbane River have the potential to isolate the site, with extreme events causing inundation of the Ground Floor of the Site;
  - Podium Level 1 and above of the development will be above the highest possible flood level in the Brisbane River;

- The period of isolation of the Site is significant in the event of Brisbane River flooding is significant; and
- There is ample time available to evacuate prior to flooding of the road network or Site commencing.
- **Local Catchment**
  - Extreme events in the local catchment are unlikely to cause significant inundation of the Site;
  - The basement and ground floor levels have an immunity approximately equivalent to the PMF event.
  - There is insufficient time available to evacuate prior to local catchment flooding commencing; and
  - In general, people can remain on Site during local catchment flood events.

## 5.2 Flood Risk Management

Recognising the potential for extreme flood events to affect the broader PDA, EDQ commissioned the report *Northshore Hamilton – High Level Flood Risk and Flood Emergency Response Assessment* (Version 2, 23 May 2024) (the **High Level Assessment**).

Noting the limited warning time associated with local catchment flooding and the significant warning time available with respect to Brisbane River flooding, the assessment proposes a strategy consisting of initially sheltering in place during severe weather events (which caters for local catchment flooding) and evacuation if a Brisbane River flood event is imminent).

The rationale for this can be summarised as follows:

- Insufficient time will be available to evacuate the Site in advance of an extreme local catchment event;
- Sufficient time will be available to move to higher parts of the Site should an extreme local catchment event occur;
- Extreme events in the local catchment will only result in the inundation of lower parts of buildings for a limited period of time; and
- Ample time is available for evacuation should an extreme event be forecast for the Brisbane River.

As a consequence, the proposed strategy for the PDA as a whole is:

- **Initial Response:** Shelter-in-place following issue of a severe weather warning of major flood alert from the Bureau of Meteorology.
- **Secondary Response:** Evacuation of the Site if flood levels in the Brisbane River reach 2.6 mAHD along the Site's southern boundary.

It is considered that this strategy is consistent with and responds to the nature and severity of local catchment and Brisbane River flooding.

The off-site evacuation route proposed in the High Level Assessment (Figure 4.1 of the High Level Assessment) is presented in Figure 5-2. The figure has been annotated to show the location of the Site.



**Figure 5-2 Identified Off-Site Evacuation Route**

With respect to Figure 5-2, the nominated route can be readily accessed via Barcham Road.

The High Level Assessment proposes the creation of site Flood Emergency Management Plans for each Site.

As noted in Section 4.2 of the High Level Assessment, the plan would include the following elements:

- Nomination of appropriate people to implement the plan (Flood Wardens);
- Appropriate warning systems (including notifications from the Bureau of Meteorology);
- Triggers associated with flood events;
- Measures to be implemented during flood events;
- Communications protocols;
- Emergency Power;
- Training of Flood Wardens; and
- Documentation and revision requirements.

It is noted that the High Level Assessment refers to the use of water level sensors local to each Site to warn of local catchment flooding and possibly a sensor for the PDA as a whole to advise of river flooding.

In this case, the inundation of Karakul Road in local catchment flooding is associated with the drainage of local site runoff to the main channel that drains the local catchment and not flooding in the main channel itself. It is typical for roads to convey such runoff (in excess of the capacity of the underground drainage system). Given that the duration of such flow would be of the order of minutes, it is not considered appropriate to install a sensor for this Site.

However, if EDQ installs gauges in other parts of the PDA where local catchment flooding is relevant, then data from such gauges could potentially be monitored.



Similarly, if a gauge is installed in the Brisbane River by EDQ, then the data from the gauge could be monitored. If a gauge is not present, then it is expected that recourse can be made to other gauges on the Brisbane River. In this regard, as most forecasts issued by the Bureau of Meteorology relate to the City Gauge in the CBD, it is expected that a relationship can be developed between the level at the gauge and the corresponding level at the Site to allow evacuation triggers to be set based on forecast or actual levels at the gauge.

While the risk of flooding of the Site is low, to be consistent with the recommendations of the High Level Assessment it is proposed that the approval of the development be conditioned to require the preparation of a Flood Emergency Management Plan for the Site.

## 6 Conclusion

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Lot 18B Northshore Hamilton is located at 260 Macarthur Avenue, Hamilton.

A review of the potential for the Site to be inundated and the requisite minimum development levels has indicated that the proposed development will have a level of immunity well in excess of that nominally required to satisfy both the requirements of the LGA planning scheme and the higher immunity currently being adopted by EDQ with regard to the design of the road system within the PDA.

Despite this, to account for the isolation and potential inundation of the Site during extreme flood events, it is recommended that any approval for the Site be conditioned to require the preparation of a Flood Emergency Management Plan.

Further, as part of detailed design, it is recommended that:

- Calculations be completed to confirm the immunity of the Site with respect to local street drainage, particularly at the north-eastern corner of the site at Karakul Street and at the substation;
- Given the relatively high flood immunity of the development, the level of the substation be raised (to provide immunity beyond that nominally required) if practicable; and
- Flood levels associated with local catchment flooding be confirmed based on the version of the PDA flood modelling current at the time of detailed design (noting that no significant change in level compared to the information presented in this report is expected).

SITE 18B. NORTHSORE HAMILTON, QUEENSLAND

## MULTI-RESIDENTIAL DEVELOPMENT



## DRAWING LIST

TP3-0000 - PROJECT INFORMATION	
TP3-0001 - DRAWING REGISTER	
TP3-0002 - COMPLIANCE SUMMARY	
TP3-0003 - COMPLIANCE WITH PLANNING REQUIREMENTS - LANDSCAPE AREAS	
TP3-0004 - COMPLIANCE WITH PLANNING REQUIREMENTS - LANDSCAPE AREAS	
TP3-0005 - COMPLIANCE WITH PLANNING REQUIREMENTS - LANDSCAPE AREAS	
TP3-0006 - COMPLIANCE WITH PLANNING REQUIREMENTS - COMMUNAL OPEN SPACES	
TP3-0007 - AREA SUMMARY - TYPICAL LEVEL	
TP3-0100 - SITE PLANS	
TP3-0101 - LOCATION PLAN	
TP3-0102 - SURVEY PLAN	
TP3-0103 - PROPOSED SITE PLAN	
TP3-0300 - GFA PLANS	
TP3-0301 - GFA PLANS	
TP3-0302 - GFA PLANS	
TP3-0400 - UNIVERSAL APARTMENTS	
TP3-0401 - UNIVERSAL APARTMENTS - GA PLANS	
TP3-0402 - UNIVERSAL APARTMENTS - APT PLANS	
TP3-0403 - UNIVERSAL APARTMENTS - APT PLANS	
TP3-1000 - GENERAL ARRANGEMENT PLANS	
TP3-1001 - BASEMENT 01	
TP3-1002 - GROUND LEVEL	
TP3-1003 - LEVEL 1 - PODIUM	
TP3-1004 - LEVEL 2 - PODIUM	
TP3-1013 - LEVELS 3-12 - TYPICAL	
TP3-1014 - LEVELS 13-16 - TYPICAL	
TP3-1015 - ROOF LEVEL	
TP3-2000 - BUILDING ELEVATIONS	
TP3-2001 - ELEVATION - NORTH	
TP3-2002 - ELEVATION - EAST	
TP3-2003 - ELEVATION - SOUTH	
TP3-2004 - ELEVATION - WEST	
TP3-3000 - BUILDING SECTIONS	
TP3-3001 - BUILDING SECTIONS	
TP3-3002 - BUILDING SECTIONS	
TP3-3003 - BUILDING SECTIONS	
TP3-3004 - BUILDING SECTIONS	
TP3-4000 - MATERIAL PALETTE	
TP3-4001 - MATERIAL PALETTE	
TP3-9000 - SHADOW STUDIES	
TP3-9001 - SHADOW DIAGRAMS - JUNE 21 - 9AM + 12PM	
TP3-9002 - SHADOW DIAGRAMS - JUNE 21 - 3PM	
TP3-9003 - SHADOW DIAGRAMS - DECEMBER 22 - 9AM + 12PM	
TP3-9004 - SHADOW DIAGRAMS - DECEMBER 22 - 3PM	

TP1	11/03/2025	DA ISSUE
Row	Date	Child Reason for Issue

Based on Drawings Received:

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TOWN PLANNING ISSUE

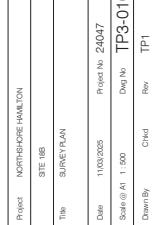
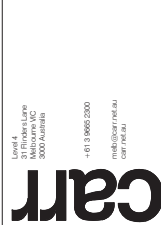
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NOTFORGIVENATION		Project: KNP-EXP-03E HAVETON	
Level 4 Moorburn Lane Moorburn VIC 3000 Australia		SITE 185	
+61 3 9066 2200 mel@carri.net.au carri.net.au		DYNAMIC REGISTER	
		Title	
		Date	
State of (A)		Project No. 23047	
Drawn By	AE	Chkd By	Rev
TP3-000		TP1	





SVM CODE	DESCRIPTION	BENCHMARK	SVM CODE	DESCRIPTION
BM	BENCH MARK		AS	SIGN
BOL	BOLLARD		SO	INSPECTION OPENING
CP	COLUMN / PILLAR		SM	SEWERAGE MANHOLE
EBX	ELECTRICAL BOX		SN	STREET NAME SIGN
ECM	ELEC CABLE MARKER		SPR	SPRINKLER
ELP	ELECTRIC LIGHT POLE		STR	TREE
ELP	ELECTRIC LIGHT POLE		SPV	STOP/SLUCE VALVE
EP	ELECTRICITY POLE		STN	SURVEY STATION
EPH	ELECTRICITY STAY POLE		STMW	STORMWATER MANHOLE
FI	FIRE HYDRANT		TAP	TAP
FI	FIRE HYDRANT		TE	TELECOM PIT
FP	FIRE HYDRANT PILLAR		TM	TELECOM MANHOLE
FP	FIRE HYDRANT PILLAR		TOH	TRANSFORMER OVERHEAD
GMH	GAZ MANHOLE		TP	TELEPHONE POLE
GMK	GAZ MARKER		TL	TRAFFIC LIGHTS
GP	GUIDE POST		TRP	TRAFFIC PIT
GS	GUIDE SIGN		WAV	WATER AIR VALVE
GY	GULLY TRAP		WCW	WATER END CAP
GV	GAZ VALVE		WM	WATER METER
HB	HOSE BOX/REEL		WMH	WATER MANHOLE
HMU	HYDRANT MARKER		WPR	WATER REDUCER
HMU	HYDRANT UNKNOWN		WRV	WATER REFLEX VALVE
PLM	PALM TREE		WSP	WATER SAMPLE POINT
PLM	TELECOM PILLAR		WSW	WATER SLOUR VALVE
PKM	PARKING METER		WTE	WATER TEE JUNCTION
FSM	PERM. SURVEY MARK		WM	WATER VALVE
SEV	SEWERAGE VENT		WM	WATER VALVE MARKER
SHRUB	SHRUB			





## Appendix A      Development Plans

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