Flagstone City Context Area 3 South

Traffic & Transport Assessment



PEET Limited

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CONTENTS

| | | Page |
|-------|--|------|
| 1. | Introduction | 1 |
| 1.1 | Background | 1 |
| 1.2 | Scope of Assessment | 2 |
| 1.3 | Reference Material | 2 |
| 2. | SITE LOCATION & PLANNING CONTEXT | 3 |
| 2.1 | Context Plan Area | 3 |
| 2.2 | Greater Flagstone Priority Development Area (PDA) | 4 |
| 2.2.1 | Overview | 4 |
| 2.2.2 | Greater Flagstone Development Charges and Offset Plan (DCOP) | 4 |
| 2.2.3 | Movement Infrastructure Master Plan (IMP) | 5 |
| 3. | DEVELOPMENT DETAILS | 8 |
| 3.1 | Proposed Development | 8 |
| 3.2 | Road Hierarchy Overview | 8 |
| 3.3 | Cross Section with Bus Stops | 10 |
| 3.4 | Intersections | 11 |
| 3.5 | Access Considerations | 11 |
| 4. | TRAFFIC ASSESSMENT | 12 |
| 4.1 | Strategic Modelling | 12 |
| 4.1.1 | Overview | 12 |
| 4.1.2 | Land Use Plan | 12 |
| 4.2 | Microsimulation Modelling | 14 |
| 4.2.1 | Overview | 14 |
| 4.2.2 | Model Time Periods & Profiling | 15 |
| 4.2.3 | Zone System & Traffic Generation | 15 |
| 4.2.4 | Microsimulation Model Output Overview | 16 |
| 4.3 | Intersection Assessment | 19 |
| 4.3.1 | Overview | 19 |
| 4.3.2 | Intersection Forms and Footprints | 20 |
| 4.3.3 | Intersection Assessment Summary | 20 |
| 5. | ALTERNATIVE TRANSPORT PROVISIONS | 22 |
| 5.1 | Public Transport | 22 |
| 5.2 | Active Transport Provisions | 23 |
| 5.2.1 | Overview | 23 |
| 5.2.2 | Active Transport Strategy Development | 23 |
| 6 | SUMMARY & CONCLUSIONS | 27 |



Tables

- Table 3.1: Typical Road Cross-Section ParametersTable 4.1: Aimsun Model 2066 OD Demand UpdatesTable 4.2: SIDRA Intersection Results Summary
- Table 4.3: Intersection 1 Sensitivity Test SIDRA Results Summary
- Table 5.1: Criteria to Determine Route Hierarchy

Figures

- Figure 1.1: Site Location
- Figure 2.1: Subject Site and Surrounding Context Areas
- Figure 2.2: DCOP Transport (Roads) Trunk Infrastructure
- Figure 2.3: DCOP Transport (Intersections) Trunk Infrastructure
- Figure 2.4: Endorsed Movement IMP
- Figure 2.5: Endorsed Movement IMP
- Figure 3.1: Proposed Road Network Hierarchies
- Figure 3.2: Esplanade Road Cross-Section
- Figure 3.3: Trunk Connector with Bus Stop Cross-Section
- Figure 3.4: Neighbourhood Connector with Bus Stop Cross-Section
- Figure 4.1: 2066 Flagstone City Land Use Plan
- Figure 4.2: Flagstone City Microsimulation (Aimsun) Model Network
- Figure 4.3: Town Centre Zone ID's & Uses
- Figure 4.4: AM Peak Link Density
- Figure 4.5: PM Peak Link Density
- Figure 4.6: Microsimulation Link Delays AM Peak
- Figure 4.7: Microsimulation Link Delays PM Peak
- Figure 4.8: Key Intersections
- Figure 5.1: Indicative Public Transport Coverage
- Figure 5.2: Key Attractors and Active Transport Desire Line Routes
- Figure 5.3: Active Transport Linkages

Appendices

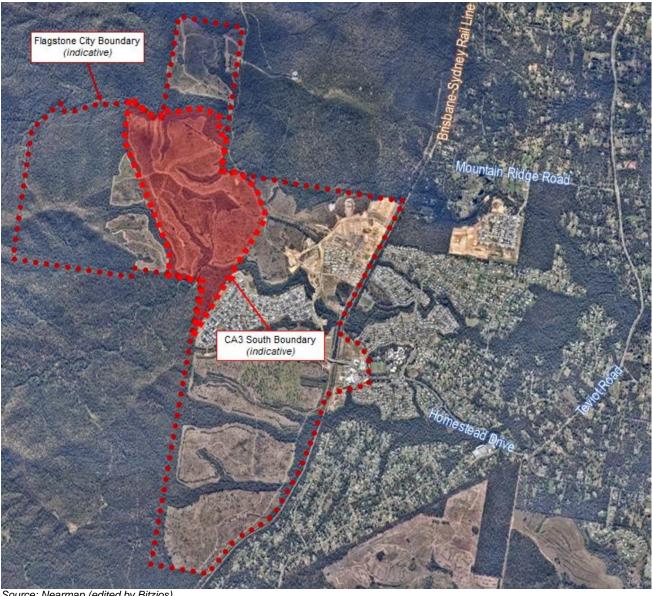
- Appendix A: CA3 South Layout
- Appendix B: Typical Road Cross-Sections
- Appendix C: Key Intersection SIDRA Summaries
- Appendix D: Active Transport Provisions



INTRODUCTION

1.1 **Background**

Bitzios Consulting has been engaged by PEET Flagstone City Pty Ltd (PEET) to undertake a traffic assessment for the proposed Context Area 3 South (CA3 South) within Flagstone City. The location of the subject site with respect to the Flagstone City boundary is illustrated in Figure 1.1.



Source: Nearmap (edited by Bitzios)

Figure 1.1: Site Location

This report documents the proposed transport network and its integration with the surrounding network and future planning in the region. It includes an assessment of key intersections and identifies the necessary alternative transport provisions to suitably service the development, aligning with existing Context Area Plans and the soon to be endorsed Movement Network Infrastructure Master Plan (Movement IMP).



1.2 Scope of Assessment

The purpose of this assessment is to provide an overview and analysis of traffic and transport related elements of the subject site with the following scope of assessment:

- A review of the development's context with relation to transport planning within Flagstone City,
 the Greater Flagstone PDA and overarching transport planning in the South Logan area
- A review of the proposed development against relevant planning to date
- Detailing previous strategic modelling and current microsimulation modelling undertaken used to identify and evaluate appropriate road layout and road cross-sections within the subject site
- Source traffic volumes from the microsimulation modelling to assess key road links and intersections within the subject site
- Review alternate transport provisions throughout the site, including consideration of public and active transport facilities and connections.

1.3 Reference Material

The following planning documents have been considered as a part of this assessment including (but not limited to):

- The Flagstone City Movement Infrastructure Master Plan (IMP) (pending endorsement)
- The Greater Flagstone PDA Development Scheme
- The superseded Infrastructure Charging Offset Plan (ICOP) and mapping
- The Development Charges and Offset Plan (DCOP) and mapping
- The approved Context Area Plans
- Relevant Economic Development Queensland (EDQ) Guidelines and Practice Notes.



2. SITE LOCATION & PLANNING CONTEXT

2.1 Context Plan Area

Flagstone City consists of five (5) overarching Context Plan areas. The subject site forms a large portion of the developable land within CA3, with the CA3 Plan endorsed 31 May 2023. The location of the subject site with respect to the endorsed CA3 plan is illustrated in Figure 2.1.



Figure 2.1: Subject Site and Surrounding Context Areas



2.2 Greater Flagstone Priority Development Area (PDA)

2.2.1 Overview

The Greater Flagstone PDA was declared on the 8th October 2010 and is located west of Jimboomba within the southern part of Council's LGA. Key PDA information is as follows:

- The PDA is made up of three (3) separate land areas totalling 7,188ha
- It is projected to experience significant growth in employment and industry
- It is estimated to accommodate approximately 51,500 dwellings & 138,000 residents.

The larger PDA land area is bordered by the Brisbane-Sydney rail line (which is predominately a freight line at present) located east of the subject site, while the Mount Lindesay Highway is located further to the east. The proposed Town Centre Masterplan forms a key portion of the PDA employment area.

2.2.2 Greater Flagstone Development Charges and Offset Plan (DCOP)

To ensure infrastructure delivery and funding is suitably allocated, EDQ developed the Greater Flagstone PDA Development Charges and Offset Plan (DCOP) effective July 2022. This document provides guidance to trunk infrastructure planning through the PDA. Planned trunk transport infrastructure in the vicinity of the subject site is illustrated in Figure 2.2 and Figure 2.3 for the trunk roads (Map 3) and trunk intersections (Map 4).

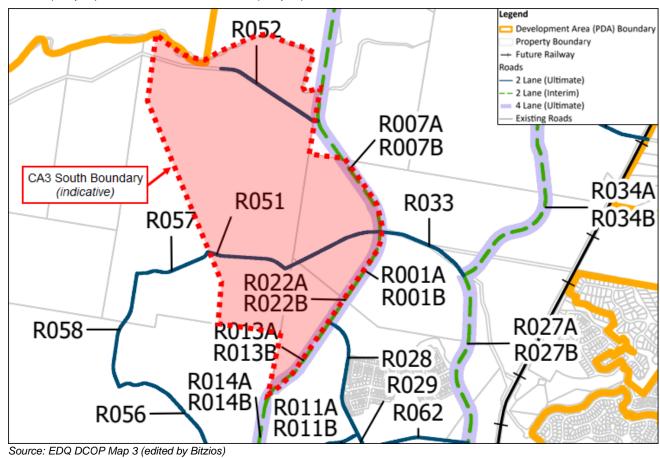


Figure 2.2: DCOP Transport (Roads) Trunk Infrastructure



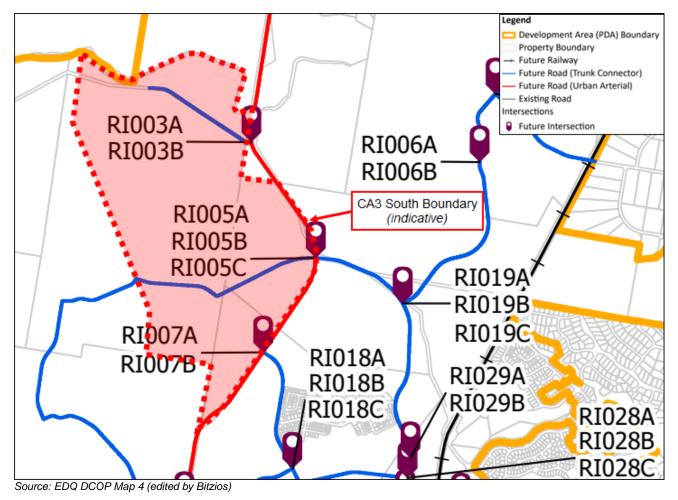


Figure 2.3: DCOP Transport (Intersections) Trunk Infrastructure

2.2.3 Movement Infrastructure Master Plan (IMP)

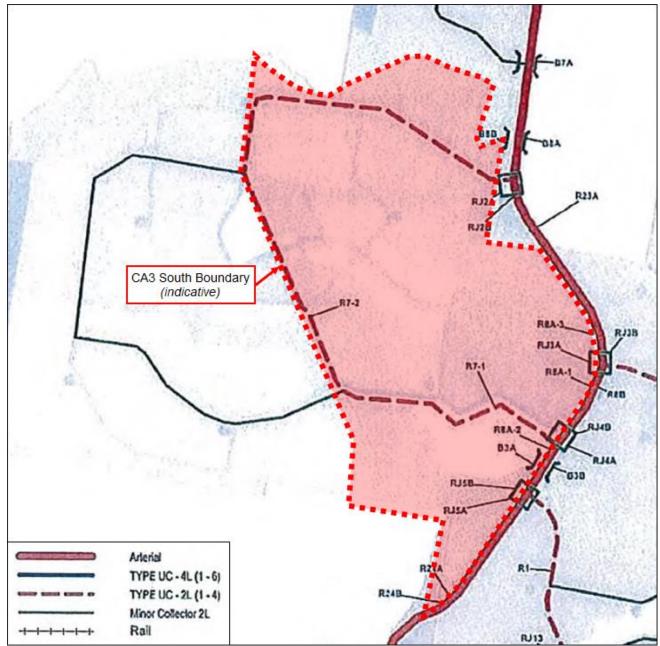
The Flagstone City endorsed Movement IMP (circa. 2015) was considered when developing the transport infrastructure requirements of the subject site. It is noted that an updated Movement IMP is currently going through an application / review process with EDQ which is expected to be endorsed in the near future to better align with more recent planning and development that has occurred to date.

For context, the endorsed Movement IMP identifies:

- The ultimate four-lane configuration of New Beith Road adjacent CA3
- The two-lane trunk collector encircling the majority of the CA3 South area
- Key signalised intersections at the CA3 connections to New Beith Road.

The road network from the endorsed Movement IMP (2015), relative to the CA3 South site area is illustrated in Figure 2.4.





Source: Endorsed Flagstone City Movement Network Infrastructure Master Plan (2015) (edited by Bitzios)

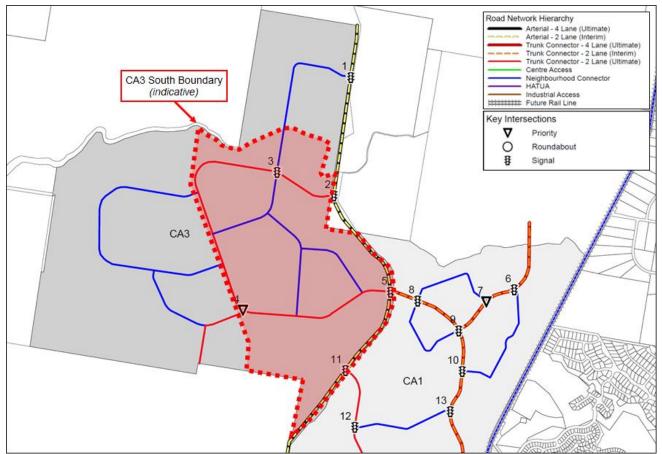
Figure 2.4: Endorsed Movement IMP

Similarly, the current (soon to be endorsed) Movement IMP also identifies:

- The ultimate four-lane configuration of New Beith Road adjacent CA3
- A two-lane trunk connector encircling the majority of the CA3 South area
- Key signalised intersections at the CA3 connections to New Beith Road.

As demonstrated, the current Movement IMP (2023) algins with the endorsed Movement IMP (2015) as well as the relevant DCOP planning over the site. The road network from the current Movement IMP (2023), relative to the CA3 South site area is illustrated in Figure 2.5.





Source: Flagstone City Movement Network Infrastructure Master Plan (2023) (edited by Bitzios)

Figure 2.5: Endorsed Movement IMP

As shown above (and in **Attachment A**), the proposed CA3 South road network generally aligns with the endorsed Movement IMP and current Movement IMP road network. The key difference between the endorsed (2015) and current (2023) Movement IMPs is that the southernmost connection from CA3 to New Beith Road is now proposed to form a 4-way intersection with Flagstonian Drive.

This change directly aligns with the current DCOP planning and improves intersection spacing on the New Beith Road arterial. Testing of the ultimate 4-way intersection form is detailed further in this report.



3. DEVELOPMENT DETAILS

3.1 Proposed Development

The proposed CA3 South development area is comprised of a variety of land uses, including:

- Low-Density Residential (1,635 dwellings)
- Medium-Density Residential (~100 dwellings)
- A District Centre
- Neighbourhood / Local Centres
- A State Primary School
- Child Care Centres
- A Community Centre
- Local, District and Regional Parklands
- An Ambulance Station.

The CA3 South plan prepared by RPS is provided at **Appendix A** and includes the relevant lot / dwelling estimates.

3.2 Road Hierarchy Overview

The proposed road hierarchy within the CA3 South area generally aligns with approvals and planning to date, and EDQ's Guideline No. 6 – Street and Movement Network.

Typical cross-section parameters for each of the proposed hierarchies are detailed in Table 3.1.

Table 3.1: Typical Road Cross-Section Parameters

| Road Hierarchy | Reserve | Pavement | Lanes | Parking | Bus | Footpath | Dedicated Cycle | Trunk |
|----------------------------|---------|----------|-------|-------------------|-----|----------------------|-----------------------------|-------|
| Trunk Connector (4-Lane) | 32.0m | 19.0m | 4 | No | Yes | 1.5m (both sides) | 3.0m cycle track (2-way) | Yes |
| Trunk Connector (2-Lane) | 23.7m | 11.8m | 2 | Yes | Yes | 1.5m (both sides) | 3.0m cycle track (2-way) | Yes |
| Neighbourhood Connector | 20.2m | 11.2m | 2 | On-Street | Yes | 1.5m (both sides) | No | No |
| Neighbourhood Access | 16.5m | 7.5m | 2 | On-Street | No | 1.5m (one side) | No | No |
| Neighbourhood Lane | 6.5m | 5.5m | 2 | No | No | No | No | No |
| Esplanade Road | 14.5m | 8.3m | 2 | Yes (one-side) | No | 1.5m (one side) | No | No |

The proposed road network hierarchies are illustrated in Figure 3.1 for key roads i.e. connector and trunk roads, with illustrations of the proposed cross-sections provided at **Appendix B**.



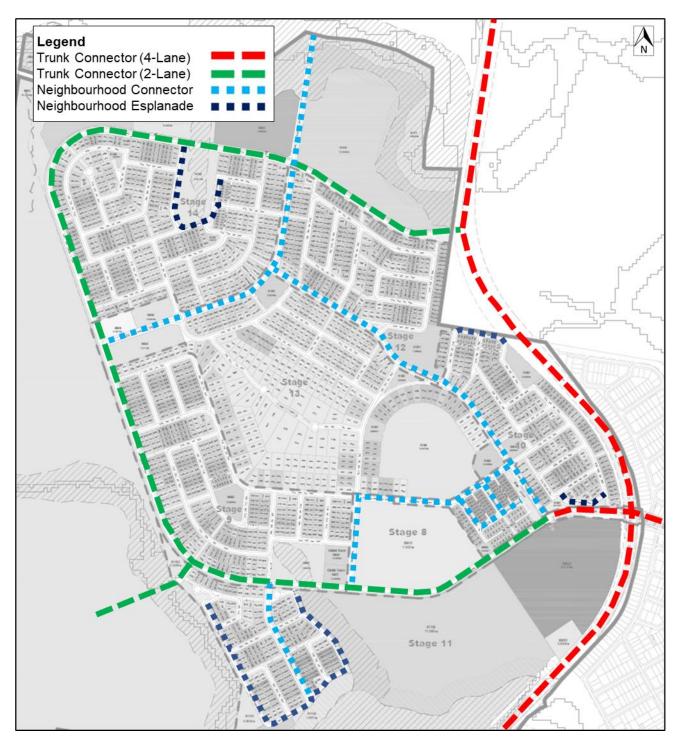


Figure 3.1: Proposed Road Network Hierarchies

As detailed above, the road cross-sections are consistent with the relevant Movement IMP, with the exception of the Esplanade Road which is shown in Figure 3.2.



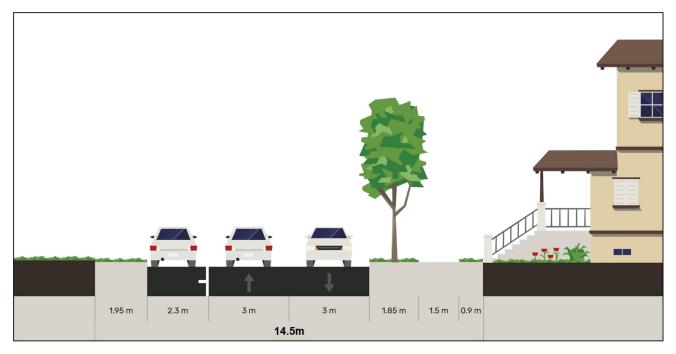


Figure 3.2: Esplanade Road Cross-Section

As shown, the proposed esplanade road is generally in accordance with the EDQ Guideline No. 6, Neighbourhood Access Street (6m). The only difference from the standard cross-section is the proposed verge width on the side of the road fronting adjacent parkland. As this verge is proposed only adjacent parkland and is not required to cater for pathways, this reduced verge width is not considered to result in any adverse impacts to amenity or active transport connectivity.

3.3 Cross Section with Bus Stops

Provision of bus facilities is detailed in Section 5.1, with bus stops proposed on Neighbourhood Connector and 2-lane Trunk Connector roads within the site area.

Typical Trunk Connector and Neighbourhood Connector cross-sections at bus stop locations are shown in Figure 3.3 and Figure 3.4.

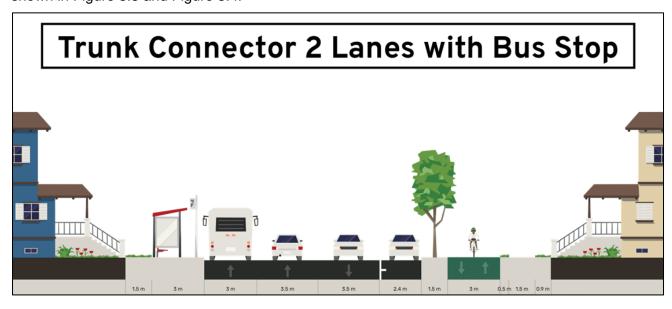


Figure 3.3: Trunk Connector with Bus Stop Cross-Section



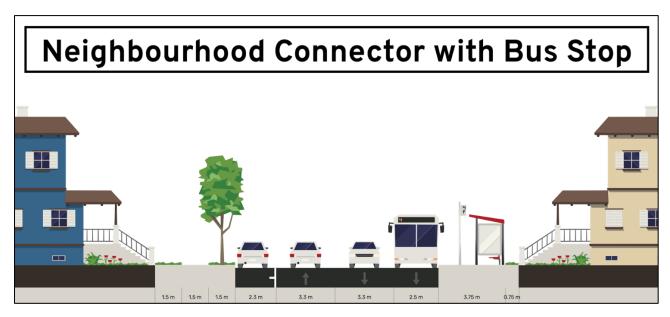


Figure 3.4: Neighbourhood Connector with Bus Stop Cross-Section

3.4 Intersections

Key principles for the proposed intersection forms and their locations were as follows:

- Intersections with four-lane roads shall be signalised or otherwise restricted to left-in / left-out
- Roundabouts shall be minimised however are considered appropriate at four-way intersections, where servicing a primary component of residential land uses
- Intersection spacings shall generally comply with those specified in EDQ's Guideline No. 6 for the relevant hierarchy.

Forecast performance of key intersections is assessed in Section 4.3.

3.5 Access Considerations

Determination of access locations has not yet been undertaken throughout the subject site and should be the subject of further applications for specific developments. That said, the proposed road network and land use arrangements have considered future access locations at a high level such as, loading / servicing requirements, car parking access, direct lot access etc.

As such, a base set of principles shall be adhered to when determining access locations through future applications and shall be provided in accordance with the following general requirements:

- Property accesses shall be provided on a property's lower order frontage road where practical
- Direct property access should be minimised on Trunk Connector roads where traffic volumes >10,000 veh/d or posted speeds >60km/h
- Property accesses shall be clear of prohibited access locations, in particular:
 - Be located clear of nearby intersections as per AS2890.1
 - Be located outside of the functional area of an intersection to avoid queuing impacts
 - Be provided perpendicular to the road unless designed as a one-way access for ease of manoeuvring
- Vehicle crossover design and separation of vehicle crossovers shall be as per the relevant requirements of IPWEAQ Standard Drawings RS-049, RS-050 and RS-051 and Logan City Council requirements.

As mentioned, detailed access design shall be undertaken with each future development application and apply the above general principles to ensure there is no adverse impact to the surrounding road network and that safety is not compromised.



4. TRAFFIC ASSESSMENT

4.1 Strategic Modelling

4.1.1 Overview

Veitch Lister Consulting Pty Ltd (VLC) were previously engaged to produce strategic models of the Flagstone PDA including PEET's development area. Strategic models were developed within VLC's Zenith model of Southeast Queensland with a cordon of the study area taken from the Greater Flagstone Strategic Traffic Model (GFSTM) to provide data relevant to PEET land to inform microsimulation (Aimsun) modelling.

A review of this model cordon was undertaken in comparison to the more recently developed South Logan Strategic Traffic Model (SLSTM) with similar strategic modelling outputs noted within the Flagstone PDA between the GFSTM and SLSTM models. As such, despite updated planning within the South Logan region, key Flagstone City strategic model outcomes remain relevant for the purpose of Aimsun modelling, detailed further below.

The details of the strategic modelling methodology and outcomes is documented in VLC's Cordon Modelling report for PEET (May 2019).

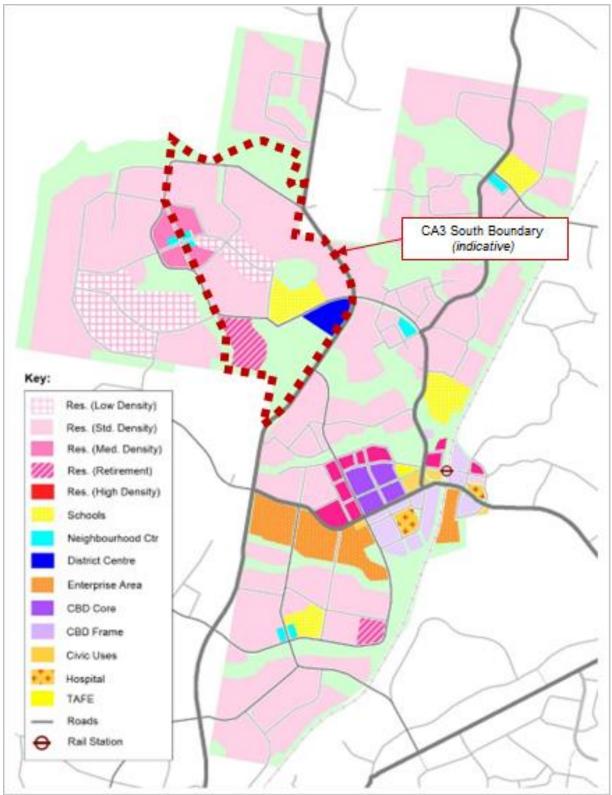
4.1.2 Land Use Plan

The land use patterns assumed in VLC's Flagstone City strategic model for the Ultimate (2066) development scenario are shown overleaf in Figure 4.1.

The development of this land use plan within the VLC model was done in such a way using single use zones such that land parcels are capable of being relocated throughout the development area (and within the model's origin-destination (OD) demand matrix) to account for changes in planning and land use proposals.

Regardless, Figure 4.1 is representative of the VLC modelling completed on behalf of PEET.





Source: VLC Cordon Modelling of PEET's Flagstone City – Final Report

Figure 4.1: 2066 Flagstone City Land Use Plan



4.2 Microsimulation Modelling

4.2.1 Overview

A 2066 microsimulation model of the Flagstone City area was prepared by Bitzios using Aimsun Next 22.0.1 traffic modelling software. This model was prepared to evaluate route choice and provide detailed insight into vehicle trip movements within Flagstone City. This facilitates analysis of road link and intersection performance at a higher level of detail than possible with strategic modelling undertaken. This model was developed as a part of the update of the Movement Network IMP currently under review, and further refined for subsequent applications within Flagstone City.

The Flagstone City road network as modelled in Aimsun is illustrated in Figure 4.2.

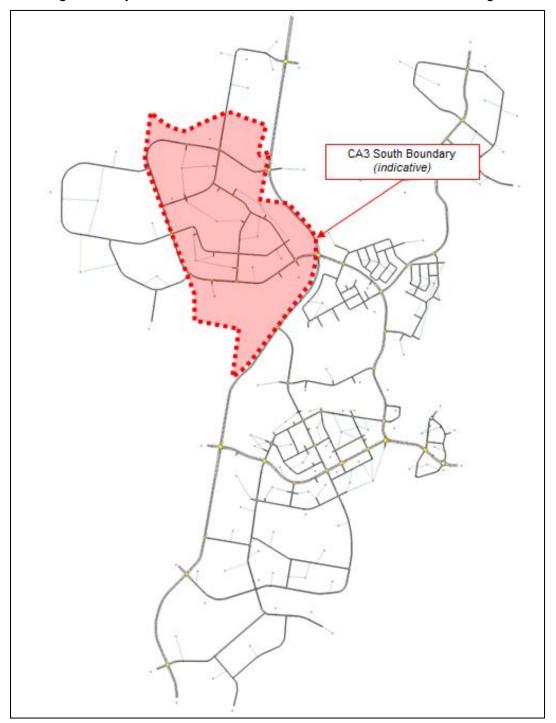


Figure 4.2: Flagstone City Microsimulation (Aimsun) Model Network



4.2.2 Model Time Periods & Profiling

The strategic model cordon provided 2-hour AM and PM peak traffic volumes. In the ultimate (2066) model year, these volumes are multiplied by a factor of 0.53 to determine peak hour traffic volumes, as per the average of VLC's SEQ strategic model.

Given the study area of the model is predominantly greenfield, network peak hours are unknown. As such, AM and PM peak hours are arbitrarily assumed as follows:

AM Peak: 07:30 – 08:30
 PM Peak: 16:30 – 17:30

The model also includes a 30-minute 'warm-up' and 'cool-down' period for each peak.

4.2.3 Zone System & Traffic Generation

The zone system for the microsimulation model was generally adopted as per the VLC strategic modelling with OD matrices of the study area taken from the cordon of the strategic model. This approach was taken to ensure as close to zone equivalence as possible with the VLC modelling. In the ultimate scenario (2066), 115 centroids are present within the cordon model, including 14 connections to external road links and neighbouring development areas, as per the cordon 'cut' from the strategic modelling.

With further development progression, significant changes in areas of the Flagstone City Structure Plan have occurred since the VLC cordon matrices were developed. To account for modified land uses and the updated scale of development expected in some stages, OD matrices for input into the microsimulation model have been updated accordingly as detailed in the Movement IMP traffic modelling report (*P2300.002R Flagstone City_IMP Traffic Modelling Report*, 01/11/2023).

With the development of the proposed CA3, model zones within CA3 have been further refined to reflect the additional detail reflected in the plan. Subsequent scaling of OD demands for zones are outlined in Table 4.1, based on dwelling numbers for residential zones and developable area for non-residential zones.

Table 4.1: Aimsun Model 2066 OD Demand Updates

| 7 | | VLC Cord | lon Model | CA3 So | Zene | |
|------------|-------------------------------|--------------------|-----------|-------------------|-----------|-----------------|
| Zone ID | Land Use | Model Area (ha) | Dwellings | Zone Area (ha) | Dwellings | Zone Scaling |
| 3942 | Local Centre | 1.66 | - | 2.496 | - | 150.4% |
| 3946 | Low Density Residential | 12.75 | 765 | - | 153 | 20% |
| 3947 | School | 7 | - | 7 | - | 100% |
| 3948 | Low Density Residential | 4.8 | 29 | - | 225 | 775.9% |
| 3949 | Low Density Residential | 5.67 | 33 | - | 36 | 109.1% |
| 3953 | Low Density Residential | 3.07 | 77 | - | 67 | 87% |
| 3954 | Medium Density Residential | 5.35 | 134 | - | 100 | 74.6% |
| 3955 | Low Density Residential | 23.17 | 280 | - | 397 | 141.8% |
| 3956 | Low Density Residential | 21.64 | 259 | - | 312 | 120.5% |
| 3957 | Low Density Residential | 40 | 573 | - | 447 | 77.7% |
| 3958 | District Centre | 7.6 | - | 8.015 | - | 105.5% |

The relevant zones are shown graphically in Figure 4.3.



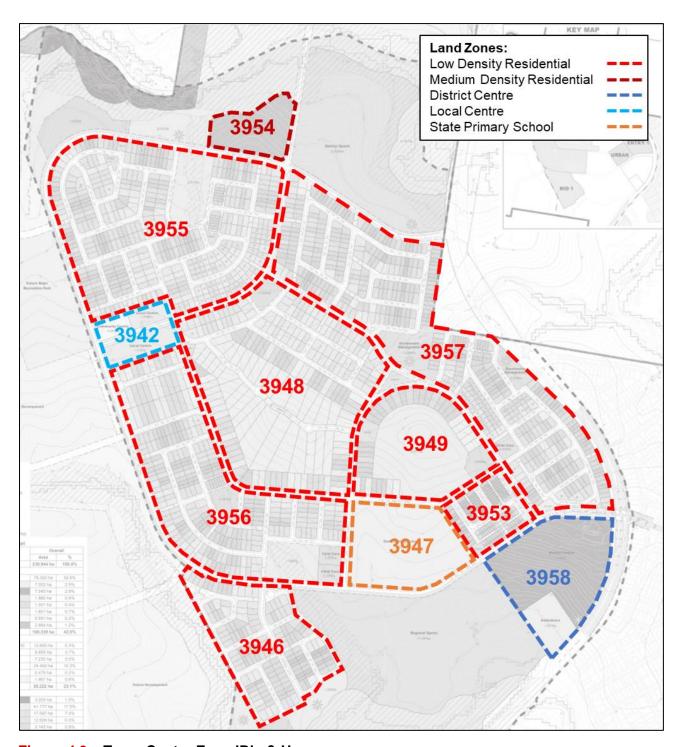


Figure 4.3: Town Centre Zone ID's & Uses

4.2.4 Microsimulation Model Output Overview

Link densities and link delays during the peak hours within the modelled CA3 South area are illustrated in Figure 4.4 to Figure 4.7.



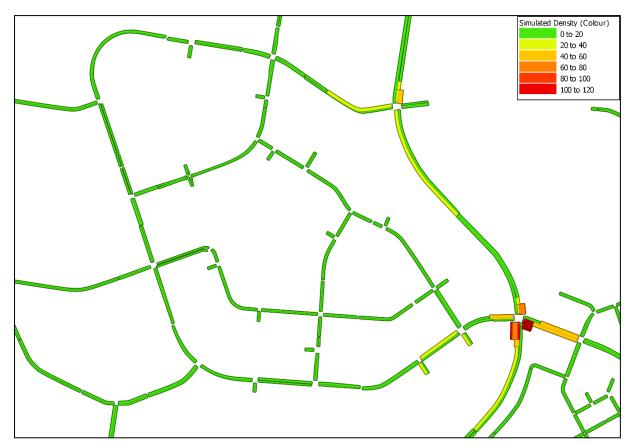


Figure 4.4: AM Peak Link Density

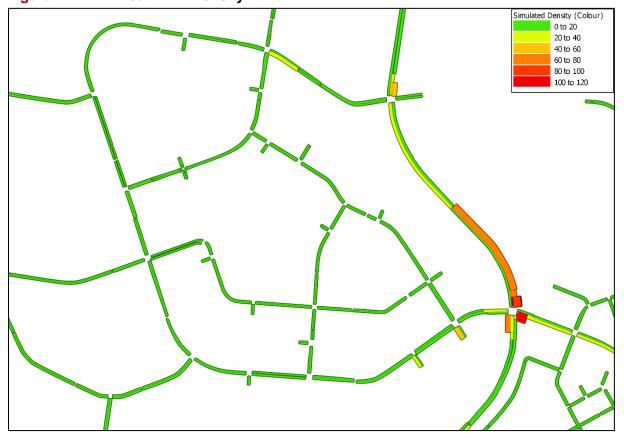


Figure 4.5: PM Peak Link Density



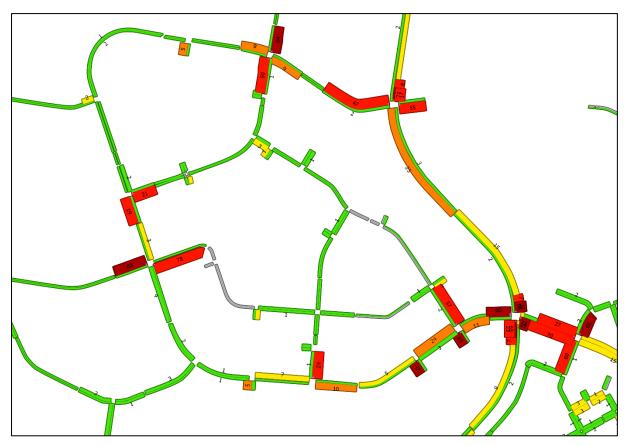


Figure 4.6: Microsimulation Link Delays - AM Peak

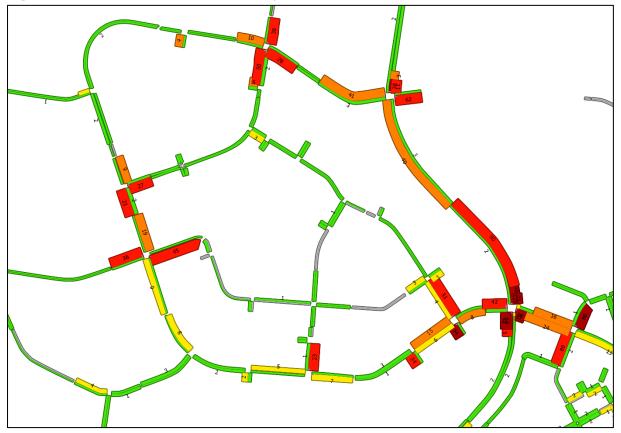


Figure 4.7: Microsimulation Link Delays - PM Peak



As shown, link delays largely occur through key signalised intersections as delays are inherent with a signalised intersection given all vehicles are required to stop at some point. That said, based on the delay results, links are typically expected to be within acceptable performance limits. These signalised intersection delays are typical in an urban environment and considering the forecast growth to the ultimate 2066 design year. Regardless, further assessment of these intersections is detailed further below

4.3 Intersection Assessment

4.3.1 Overview

The key intersections assessed comprise of future trunk intersections identified in the updated Movement IMP as well as additional signalised intersections within the CA3 South area itself. The intersections assessed and their relevant ID are shown in Figure 4.8.

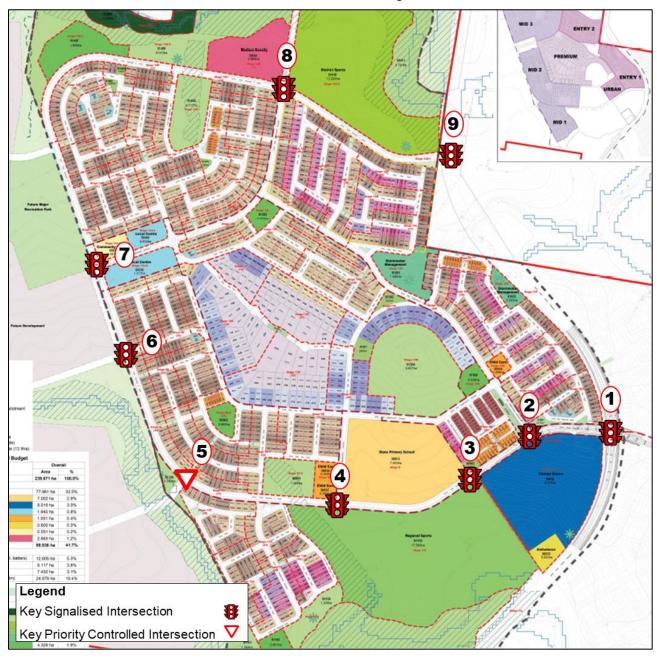


Figure 4.8: Key Intersections



4.3.2 Intersection Forms and Footprints

Key intersection layouts were transferred into SIDRA Intersection (v9.1) based on the Aimsun model layouts and assessed using peak flows extracted from the Aimsun models. This was to gain an indication of critical peak performance at the intersection level.

The resultant intersection forms and layouts are indicative only and subject to change based on further detailed assessment associated with future stages of development. Importantly, ultimate intersection forms are based on 2066 planning (42 years into the future) which may be subject to change over time.

NOTE: turn pocket lengths indicated on SIDRA layouts are indicative only and subject to design to accommodate queuing, deceleration and operational factors.

4.3.3 Intersection Assessment Summary

A summary of the SIDRA intersection results for the key intersections identified is outlined in Table 4.2. Detailed SIDRA outputs are provided in **Appendix C**.

Table 4.2: SIDRA Intersection Results Summary

| Intersection ID | DOS | (v/c) | Avg. D | elay (s) | LOS | | |
|-----------------|------|-------|--------|----------|-----|----|--|
| intersection iD | AM | PM | AM | PM | AM | PM | |
| 1 | 0.89 | 1.06 | 44 | 110 | D | F | |
| 2 | 0.82 | 0.87 | 29 | 35 | С | D | |
| 3 | 0.64 | 0.73 | 10 | 11 | А | В | |
| 4 | 0.54 | 0.44 | 8 | 6 | А | А | |
| 5 | 0.48 | 0.21 | 6 | 5 | А | А | |
| 6 | 0.33 | 0.43 | 9 | 16 | А | В | |
| 7 | 0.14 | 0.20 | 7 | 9 | А | А | |
| 8 | 0.71 | 0.62 | 23 | 22 | С | С | |
| 9 | 0.89 | 0.90 | 52 | 52 | D | D | |

As shown, with forecast ultimate (2066) traffic volumes, intersections are expected to generally operate within acceptable performance limits in the AM and PM peak hours. The New Beith Road / Flagstonian Drive intersection is forecast to exceed typical performance thresholds in the PM peak only. However, due to the aspirational nature and long-term uncertainty associated with forecasting vehicle and pedestrian volumes for such a long design horizon (i.e. 42-years into the future), it is unclear if these forecasts will ever be met.

Further to this, a key component of the New Beith Road / Flagstonian Drive intersection performance is the impact of fully protected pedestrian movements on the efficiency of the signals noting the long pedestrian crossing distances. Above results adopt SIDRA default pedestrian crossing volumes of 50 pedestrians per hour on each crossing. As such, a sensitivity test was undertaken to test the comparative performance of the intersection if pedestrian volumes were reduced, thereby reducing green time for the pedestrian phase and improving signal efficiency. The east-west pedestrian crossing on the southern approach of the New Beith Road / Flagstonian Drive intersection was determined as critical to intersection performance. Table 4.3 below demonstrates that the intersection would operate within acceptable performance limits if these east-west pedestrian volumes were reduced to 20 per hour.



Table 4.3: Intersection 1 Sensitivity Test SIDRA Results Summary

| Interpostion ID | DOS | (v/c) | Avg. D | elay (s) | LOS | | |
|-----------------|------|-------|--------|----------|-----|----|--|
| Intersection ID | AM | PM | AM | PM | AM | PM | |
| 1 | 0.87 | 0.99 | 39 | 84 | D | F | |

Based on the above, and the aspirational and somewhat uncertain nature of the 42-year design horizon, the intersection configuration and performance is considered acceptable.

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5. ALTERNATIVE TRANSPORT PROVISIONS

5.1 Public Transport

Regional and local bus routes are proposed throughout Context Area 3 generally in accordance with the current Movement IMP and the Context Area plan. Exact bus stop locations through the masterplan area are subject to consultation with TMR / TransLink and may be determined as a part of further applications for specific stages of the CA3 South. Regardless, indicative locations of bus stop pairs within the CA3 site area are shown in Figure 5.1 demonstrating a high level of expected public transport coverage within the town centre.

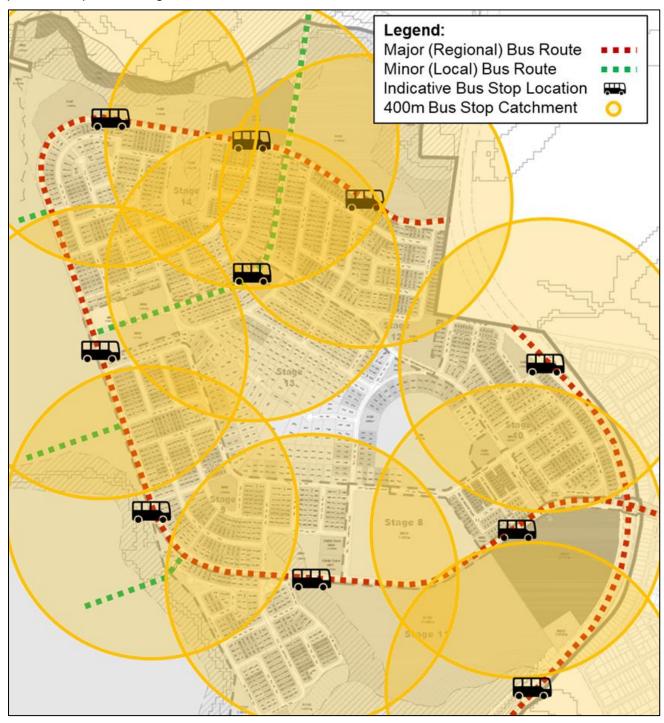


Figure 5.1: Indicative Public Transport Coverage



Where possible, bus stops are located close to signalised intersections to allow road crossings to take place via the signalised crossing(s). Where midblock stops are proposed, these are generally colocated with green space / 'pedestrian linkages' to the nearby residential catchment. This affords residents / passengers with more direct connection points from the stop infrastructure / bus service.

As shown, the majority of the proposed development area is located within reasonable walking distance of a future public transport route / stop, with only a small portion of the lowest density, slope-affected Lots outside of the 400m walking radius. Consistent with public transport requirements of the Flagstone Development Scheme, the indicative bus stop locations identified will result in more than 95% of residential dwellings to be located within walking distance of a bus stop.

Note that bus stop types shall be subject to future applications and detailed design and be in accordance with TransLink's *Public Transport Infrastructure Manual (PTIM)*.

5.2 Active Transport Provisions

5.2.1 Overview

Pedestrian and cycle infrastructure will be provided throughout the masterplan area both within the road reserve and outside the road reserve through park linkages and private / shared pedestrian zones.

Major active transport links with separated cycle tracks and pedestrian paths within the road reserve are proposed within:

- New Beith Road (North-south adjacent the CA3 South area)
- 2-lane trunk roads (encircling the majority of the CA3 South area)
- The Regional Sport Park (through the parklands).

Shared paths shall also be provided on the frontage of the state primary school site. Parks, open space and pedestrian linkages are also proposed throughout the site area, with a large number of linkages between residential lots to provide direct pedestrian access between local neighbourhoods and the wider active (and public) transport network.

5.2.2 Active Transport Strategy Development

An active transport strategy was developed for the site area to deliver a hierarchy of active transport provisions. The strategy is based on:

- Key active transport attractors
- Relevant user types
- Key active transport desire line routes.

Key active transport attractors consist of the major land uses including retail/commercial centres, schools, and various types of parklands. These land uses are expected to attract a substantial active transport demand from the residential lots throughout the site area

The key attractors and key pedestrian desire line routes through the site area are depicted in Figure 5.2 and provided in **Appendix D**.



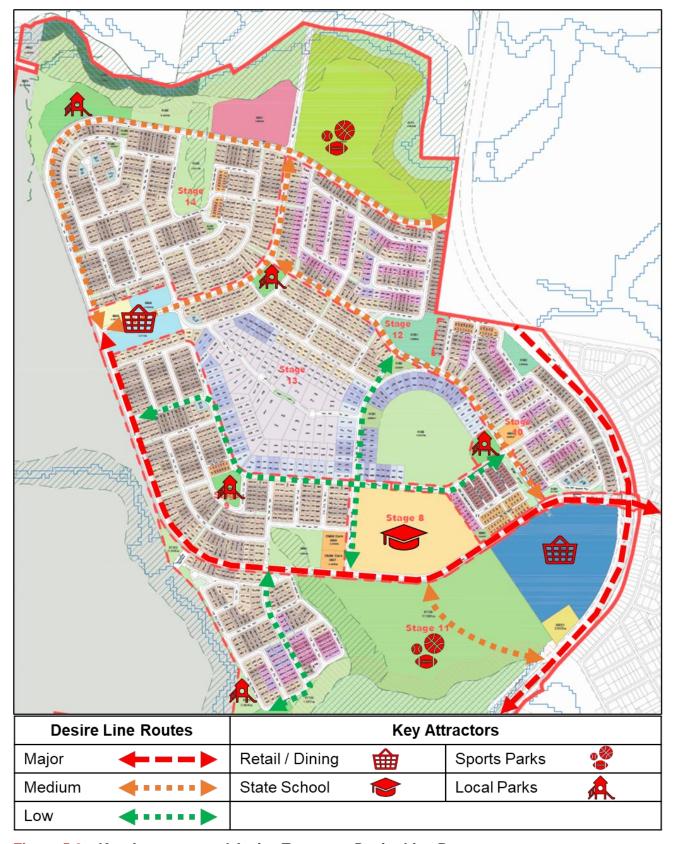


Figure 5.2: Key Attractors and Active Transport Desire Line Routes

Based on the above attractors and desire line routes, an active transport route hierarchy was developed to specify a set of criteria and relevant infrastructure based on route classification.



The overarching hierarchy classification considers three levels, being:

- Major
- Medium
- Low.

The hierarchy does not suggest any lower level of importance or quality of infrastructure, rather defines the type of trip attractors/generators and user types that are most likely to use the prevailing facility, and therefore assist in the determination of an appropriate facility/infrastructure type.

For example:

a Major classification applies to routes / infrastructure that service multiple trip attractors such as connections between schools, parklands and major retail centres. This hierarchy may warrant a wider treatment due to a higher volume of active transport users.

a Low classification applies to routes / infrastructure that primarily service the immediate catchment, are not expected to service a large number of user types and warrant pathway treatments within the road reserve e.g. 1.5m footpath(s).

The route hierarchy criteria are summarised in Table 5.1.

Table 5.1: Criteria to Determine Route Hierarchy

| Classification | Criteria | Infrastructure |
|----------------|---|--|
| Major | Provides connections: Within or between key centres To schools, shopping centres, transit hubs/stops Along major, higher volume transport routes | Includes: Separated cycleways Shared paths outside road reserve Shared paths ≥3m inside road reserve both sides of road |
| Medium | Provides connections: Within and between residential areas To major active transport routes To open space / parks To public transport stops | Includes: Shared path inside road reserve one side Shared paths ≤3m inside road reserve both sides of road Pathways outside road reserve (open space) |
| Low | Provides connections: Within immediate catchment (400m) On lower order, lower volume transport routes | Includes: Pathways inside road reserve one side (e.g. 1.5m footpath) |

The key active transport linkages throughout the site area were therefore developed adopting the strategy of key attractors, key desire line routes and the criteria outlined in the route hierarchy classification. The key active transport linkages and route hierarchy is shown in Figure 5.3 and provided in **Appendix D**.

As shown, the proposed key active transport linkages are considered to provide a high-level of service and accessibility for the CA3 South site and provide connectivity to surrounding services and properties. The proposed connections also generally align with the active transport components of the current Movement IMP.

Further, the DCOP identifies an off-road shared path through the Regional Sport Park and along the southern extent of the CA3 area. While not specifically identified on the plan included at **Appendix A**, the proposal does not preclude part provision of this path, and this off-road shared path is recommended to form part of any future approvals relating to the sports park.



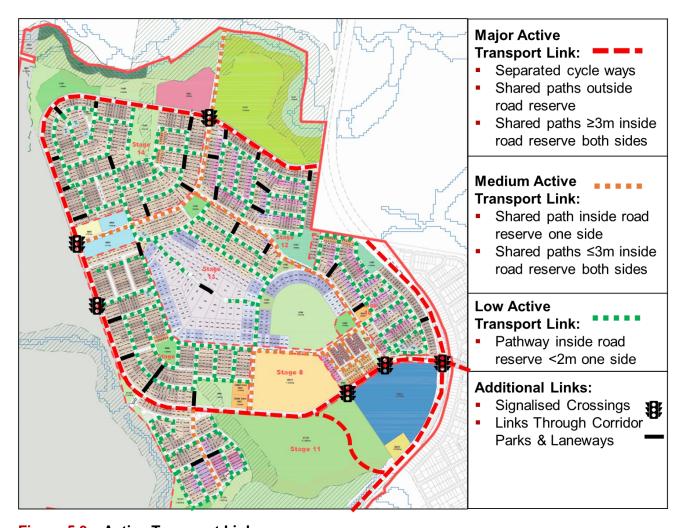


Figure 5.3: Active Transport Linkages



6. SUMMARY & CONCLUSIONS

The traffic and transport findings for the Flagstone Town Centre Masterplan are summarised below:

- The proposed development is located within the southern portion of Context Area 3 (CA3 South) of Flagstone City and incorporates a range of land uses generally consistent with relevant planning
- CA3 South includes in the order of 1,700 low-medium density dwellings, local and district centres, a primary school and child care, community centre, ambulance station and parks and open spaces
- Proposed road hierarchies are generally in accordance with the Movement IMP and/or the EDQ Street and Movement Network PDA Guideline, with exception only to the "Neighbourhood Esplanade" which reflects EDQ's Neighbourhood Access (16.5m) with a marginal reduction in verge width on one side. Road hierarchies are therefore considered to meet the relevant requirements
- Strategic traffic modelling was undertaken by VLC for which cordon matrices were extracted for the development of microsimulation (Aimsun) traffic models of Flagstone City, this process has been used throughout all previous application material and as such, presents a consistent approach to determining road network / intersection requirements
- Microsimulation modelling indicates that CA3 links are typically expected to be within acceptable
 performance limits with some delays noted on approaches to signalised intersections on trunk
 roads. This is typical of signalised intersections in an urban environment, noting the 2066 design
 year adopted for the ultimate assessment
- Overall findings of intersection modelling of key intersections demonstrate that, based on current estimates of future traffic volumes, key intersections as detailed can generally cater for peak traffic demands within CA3 south
- Approximately 95% of the proposed site is located within 400m of a future bus route / stop / service
 achieving the public transport requirements of the Development Scheme. Stops are generally
 located close to signalised intersections (crossings) or midblock adjacent green pedestrian links
- The CA3 South site is considered to provide a highly accessible active transport network, with a range of route options both within and outside the road reserve. The proposed connections also generally align with the active transport components of the Movement IMP and do not preclude the off-road shared path provisions identified in the DCOP.

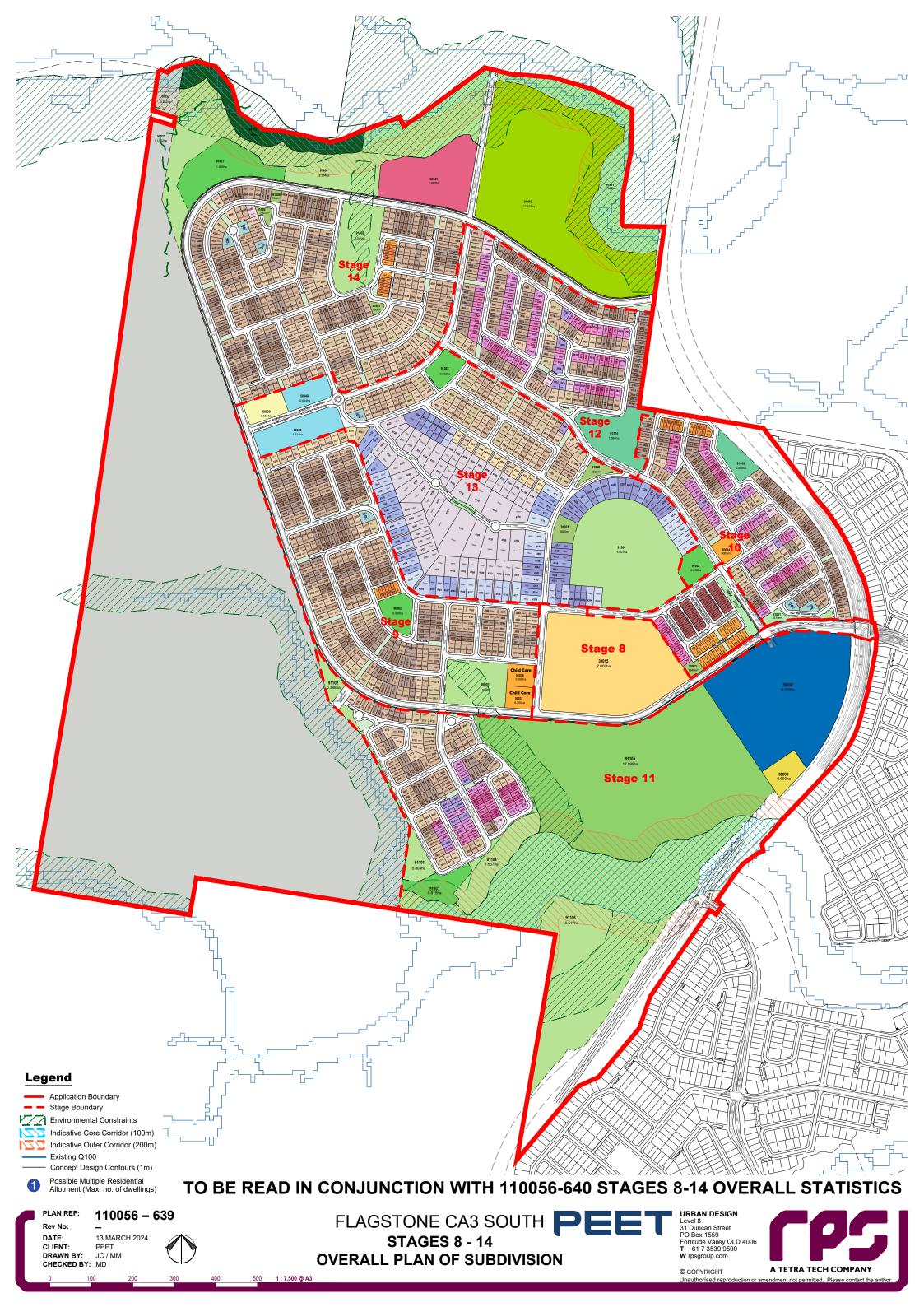
Based on the above assessment, it is concluded that the proposed road network, intersection arrangements and alternate transport provisions are expected to accommodate transport demands associated with the proposed CA3 South development.





Appendix A: CA3 South Layout





CA3 SOUTH - Stage 8 - 14 Yield Breakdown

| CA3 SOUTH - Stage 8 - 14 Yield Breakdown | | | | | | | | | | | | |
|--|---------|-------------|-------------|-------------|-------------|------------|-------------|-------|--------|--|--|--|
| Lot Type | Stage 8 | Stage 9 | Stage 10 | Stage 11 | Stage 12 | Stage 13 | Stage 14 | Ov | erall | | | |
| | Yield | Yield | Yield | Yield | Yield | Yield | Yield | Yield | % | | | |
| 25m Deep Terrace Product | | | | | | | | | | | | |
| Terrace 9.5m Allotment | _ | 32 | _ | _ | _ | _ | _ | 32 | 2% | | | |
| Subtotal | _ | 32 | _ | _ | _ | _ | _ | 32 | 2% | | | |
| 25m Deep Product | | | | | | | | | | | | |
| Villa 10m Allotment | _ | 4 | 8 | 15 | 7 | 1 | _ | 34 | 2% | | | |
| Premium Villa 12.5m Allotment | _ | 7 | 21 | 15 | 41 | _ | _ | 84 | 5% | | | |
| Courtyard 14m Allotment | _ | _ | 28 | 16 | 33 | _ | _ | 77 | 5% | | | |
| Premium Courtyard 16m Allotment | _ | _ | 3 | 8 | 5 | 1 | _ | 17 | 1% | | | |
| Premium Traditional 20m Allotment | _ | _ | 4 | 1 | 4 | _ | 1 | 10 | 1% | | | |
| Subtotal | _ | 11 | 64 | 55 | 90 | 1 | 1 | 222 | 14% | | | |
| 28m Deep Terrace Product | | , | | | | | | | • | | | |
| Terrace 7.5m Allotment | _ | 5 | 9 | _ | _ | _ | 10 | 24 | 1% | | | |
| Terrace 9.5m Allotment | _ | 26 | 4 | _ | _ | _ | 4 | 34 | 2% | | | |
| Subtotal | _ | 31 | 13 | _ | _ | _ | 14 | 58 | 4% | | | |
| 30m Deep Product | | ' | ' | | · | | | | · | | | |
| Villa 10m Allotment | _ | 53 | 23 | 5 | 26 | _ | 47 | 154 | 9% | | | |
| Premium Villa 12.5m Allotment | _ | 89 | 52 | 29 | 66 | _ | 94 | 330 | 20% | | | |
| Courtyard 14m Allotment | _ | 123 | 42 | 40 | 43 | 37 | 170 | 455 | 28% | | | |
| Premium Courtyard 16m Allotment | _ | 24 | 6 | 15 | 8 | 44 | 54 | 151 | 9% | | | |
| Traditional 18m Allotment | _ | 4 | _ | _ | _ | 11 | _ | 15 | 1% | | | |
| Premium Traditional 20m Allotment | | 11 | 5 | 9 | 5 | 22 | 14 | 66 | 4% | | | |
| Possible Multiple Residential Allotment | | 1 | 2 | | _ | 1 | 3 | 7 | 0% | | | |
| Subtotal | | 305 | 130 | 98 | 148 | 115 | 382 | 1178 | 72% | | | |
| 50m+ Deep Product | | | | | | | | | | | | |
| Courtyard 14m Allotment | _ | T | | | <u> </u> | 30 | | 30 | 2% | | | |
| Premium Courtyard 16m Allotment | | _ | _ | | _ | 25 | _ | 25 | 2% | | | |
| Traditional 18m Allotment | _ | _ | _ | | _ | 20 | _ | 20 | 1% | | | |
| Premium Traditional 20m Allotment | _ | _ | _ | | _ | 14 | _ | 14 | 1% | | | |
| Ridgetop Allotment | | | _ | | | 56 | _ | 56 | 3% | | | |
| Subtotal | - | | _ | | _ | | | 145 | 9% | | | |
| Subtotal | _ | _ | _ | _ | _ | 145 | _ | 140 | 370 | | | |
| Total Residential Allotments | _ | 379 | 207 | 153 | 238 | 261 | 397 | 1635 | 100% | | | |
| Residential Net Density | | 14.7 dw/ha | 15.3 dw/ha | 13.6 dw/ha | 14.8 dw/ha | 8.9 dw/ha | 11.9 dw/ha | | dw/ha | | | |
| Super Lots | _ | 14.7 uw/iia | 15.5 UW/IIa | 13.6 UW/IIa | 14.0 UW/11a | 0.9 uw/iia | 11.9 GW/IIa | 13.0 | uw/iia | | | |
| Local Centre | _ | T _ | T _ | _ | _ | _ | _ | | | | | |
| District Centre | _ | | _ | 1 | _ | | _ | | 1 | | | |
| Ambulance | | | | 1 | | | | | 1 | | | |
| Child Care | _ | 2 | <u> </u> | _ | _ | | _ | | 3 | | | |
| Community Centre | _ | _ | _ | | _ | | _ | | _ | | | |
| State Primary School | 1 | <u> </u> | | | | | | | 1 | | | |
| Medium Density Allotment | | _ | _ | _ | _ | _ | _ | | | | | |
| | _ | _ | _ | _ | _ | _ | 1 | 1 | | | | |
| Balance Allotment | _ | - | _ | _ | _ | _ | 2 | | 2 | | | |
| Subtotal | 1 | 2 | 1 | 2 | _ | _ | 3 | | 9 | | | |
| Total Allatina mate | | 204 | 200 | 455 | 900 | 204 | 400 | | *44 | | | |
| Total Allotments | 1 | 381 | 208 | 155 | 238 | 261 | 400 | 10 | 644 | | | |
| Maximum Potential Posidential Durallings | | | | | | | | | | | | |
| Maximum Potential Residential Dwellings | _ | 380 | 209 | 153 | 238 | 262 | 402 | 10 | 644 | | | |
| (Includes Multiple Residential Allotments) | | | | | | | | | | | | |

CA3 SOUTH - Stage 8 - 14 Land Budget

| | Stage 8 | Stage 9 | Stage 10 | Stage 11 | Stage 12 | Stage 13 | Stage 14 | Ove | rall |
|---|-----------|-----------|-----------|-----------|-----------|-----------|------------|------------|--------|
| | Area | Area | % |
| Land Use | 10.082 ha | 28.083 ha | 16.892 ha | 66.313 ha | 16.131 ha | 37.552 ha | 144.019 ha | 319.072 ha | 100.0% |
| Saleable Area | | | | | | | | | |
| Residential Allotments | _ | 14.631 ha | 7.860 ha | 6.287 ha | 9.271 ha | 23.013 ha | 16.894 ha | 77.956 ha | 24.4% |
| Medium Density | _ | _ | _ | _ | _ | _ | 2.863 ha | 2.863 ha | 0.9% |
| Local Centre | _ | _ | _ | _ | _ | 1.945 ha | _ | 1.945 ha | 0.6% |
| District Centre | _ | _ | _ | 8.015 ha | _ | _ | _ | 8.015 ha | 2.5% |
| Ambulance | _ | _ | _ | 0.600 ha | _ | _ | _ | 0.600 ha | 0.2% |
| Child Care | _ | 0.700 ha | 0.301 ha | _ | _ | _ | _ | 1.001 ha | 0.3% |
| Community Centre | _ | _ | _ | _ | _ | 0.551 ha | _ | 0.551 ha | 0.2% |
| State Primary School | 7.002 ha | _ | _ | _ | _ | _ | _ | 7.002 ha | 2.2% |
| Total Area of Allotments | 7.002 ha | 15.331 ha | 8.161 ha | 14.902 ha | 9.271 ha | 25.509 ha | 19.757 ha | 99.933 ha | 31.3% |
| Road | | | | | | | | | |
| North South Arterial Dedication (incl. batters) | 0.266 ha | _ | 3.079 ha | 9.562 ha | _ | _ | 0.132 ha | 13.039 ha | 4.1% |
| Trunk Connector 2 Lanes (23.7m) | 2.102 ha | 2.498 ha | _ | 0.144 ha | 0.028 ha | 0.327 ha | 4.052 ha | 9.151 ha | 2.9% |
| Neighbourhood Connector (20.2m) | 0.712 ha | 1.771 ha | 0.598 ha | 0.689 ha | 1.746 ha | 1.016 ha | 0.687 ha | 7.219 ha | 2.3% |
| Neighbourhood Access Street (16.5m) | _ | 5.257 ha | 3.512 ha | 3.116 ha | 3.243 ha | 3.813 ha | 5.971 ha | 24.912 ha | 7.8% |
| Laneway (6.5m) | _ | 0.320 ha | 0.075 ha | _ | _ | _ | 0.082 ha | 0.477 ha | 0.1% |
| Pedestrian Linkages | | 0.365 ha | 0.132 ha | 0.163 ha | 0.256 ha | 0.214 ha | 0.535 ha | 1.665 ha | 0.5% |
| Total Area of New Road | 3.080 ha | 10.211 ha | 7.396 ha | 13.674 ha | 5.273 ha | 5.370 ha | 11.459 ha | 56.463 ha | 17.7% |
| Open Space | | | | | | | | | |
| Conservation Buffer | | _ | _ | _ | _ | _ | 1.988 ha | 1.988 ha | 0.6% |
| Corridor Park / Conservation | | 1.564 ha | _ | 19.327 ha | _ | 5.627 ha | 13.997 ha | 40.515 ha | 12.7% |
| Stormwater Management | _ | _ | 0.555 ha | _ | 1.588 ha | _ | _ | 2.143 ha | 0.7% |
| Regional Sports | | _ | _ | 17.595 ha | _ | _ | _ | 17.595 ha | 5.5% |
| District Sports | | _ | _ | _ | _ | _ | 12.624 ha | 12.624 ha | 4.0% |
| Neighbourhood Recreation Park | _ | 0.580 ha | 0.539 ha | 0.815 ha | _ | _ | 1.905 ha | 3.839 ha | 1.2% |
| Local Recreation Park | | 0.124 ha | _ | _ | _ | 0.502 ha | 0.255 ha | 0.881 ha | 0.3% |
| Local Linear Recreation Park | - | 0.273 ha | 0.241 ha | _ | _ | 0.542 ha | _ | 1.056 ha | 0.3% |
| Total Open Space | - | 2.541 ha | 1.335 ha | 37.737 ha | 1.588 ha | 6.671 ha | 30.769 ha | 80.641 ha | 25.3% |
| Balance Allotments | | | | | | | | | |
| Balance Allotment | _ | _ | _ | _ | _ | _ | 82.034 ha | 82.034 ha | 25.7% |
| Total Balance Allotments | _ | _ | _ | _ | _ | _ | 82.034 ha | 82.034 ha | 25.7% |

PLAN REF: 110056 - 640

Rev No: DATE: 13 MARCH 2024 CLIENT: PEET
DRAWN BY: JC / MM
CHECKED BY: MD Not to Scale @ A3



FLAGSTONE CA3 SOUTH PEET
STAGES 8 - 14 **OVERALL PLAN OF SUBDIVISION STATISTICS**

URBAN DESIGN
Level 8
31 Duncan Street
PO Box 1559
Fortitude Valley QLD 4006
T +61 7 3539 9500
W rpsgroup.com

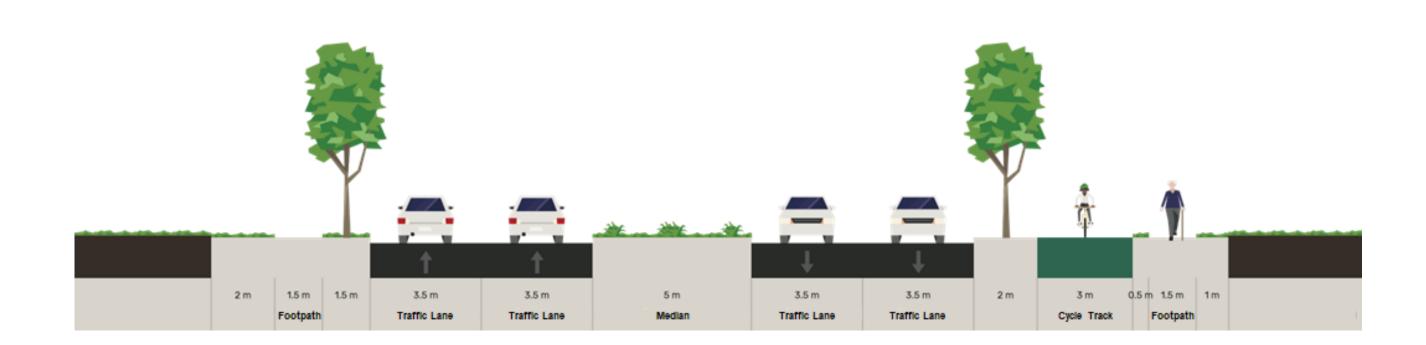
A TETRA TECH COMPANY



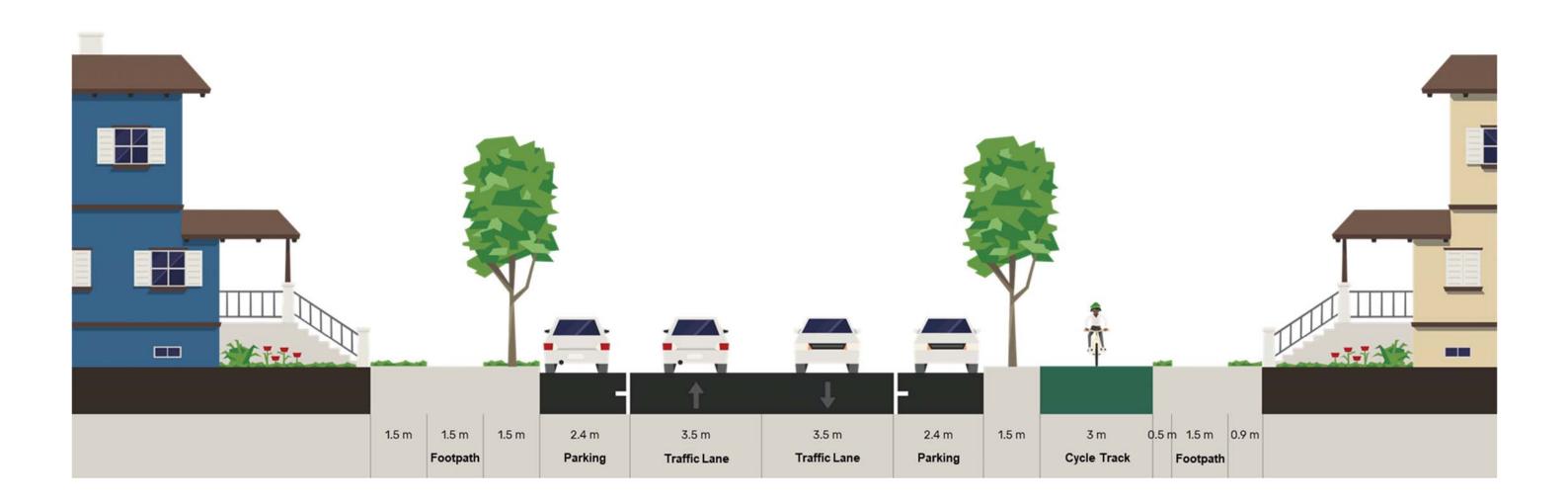
Appendix B: Typical Road Cross-Sections



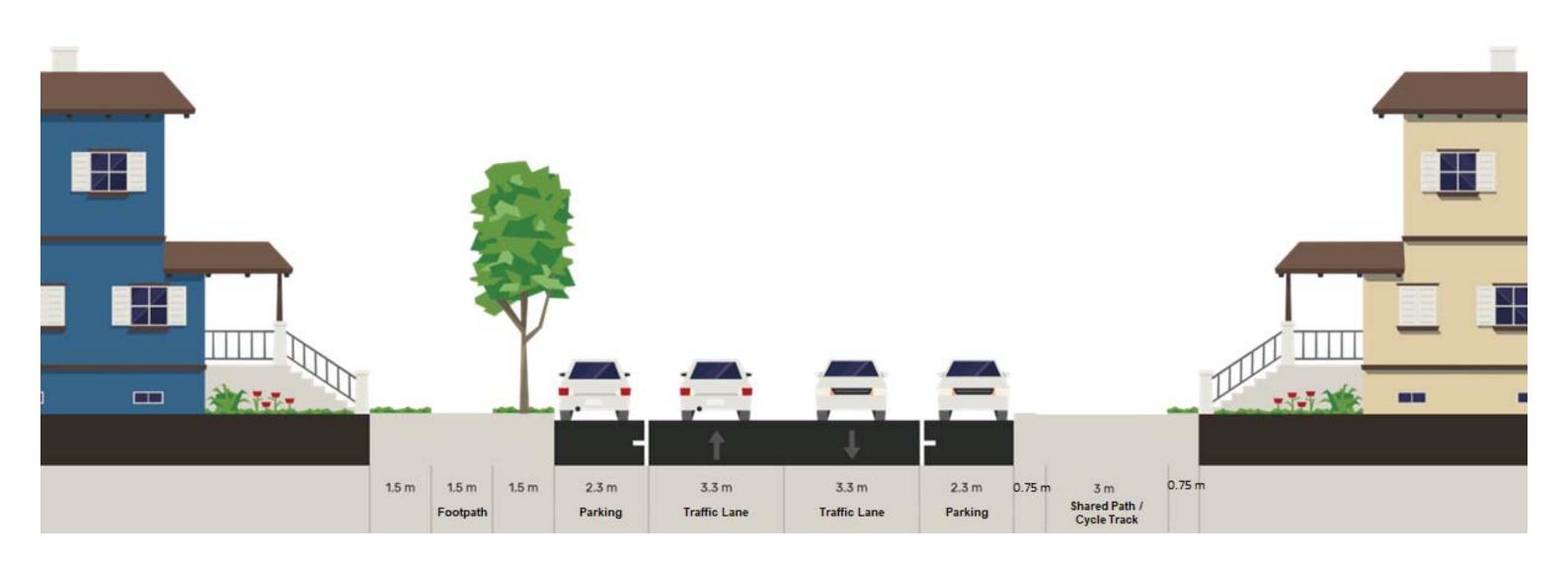
Arterial & Trunk Connector (4 Lanes) Ultimate



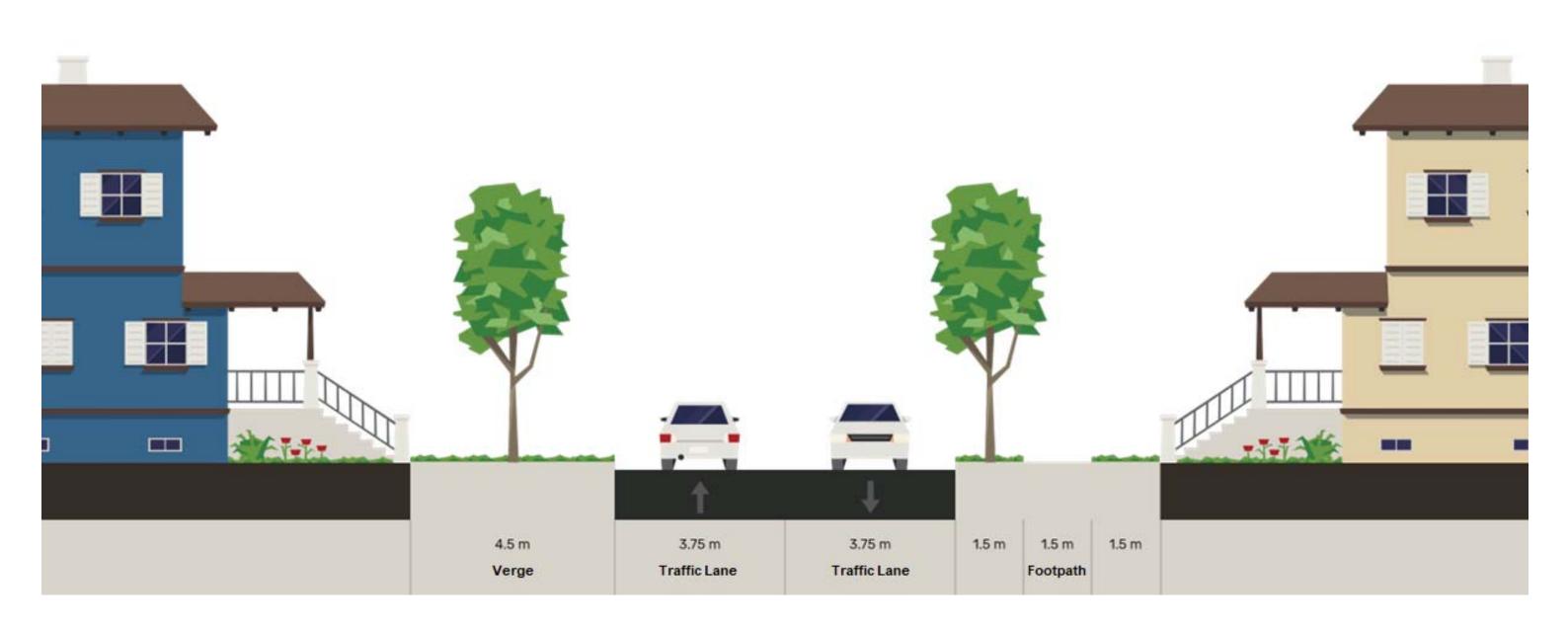
Trunk Connector 2 Lanes (With Parking)



Neighbourhood Connector

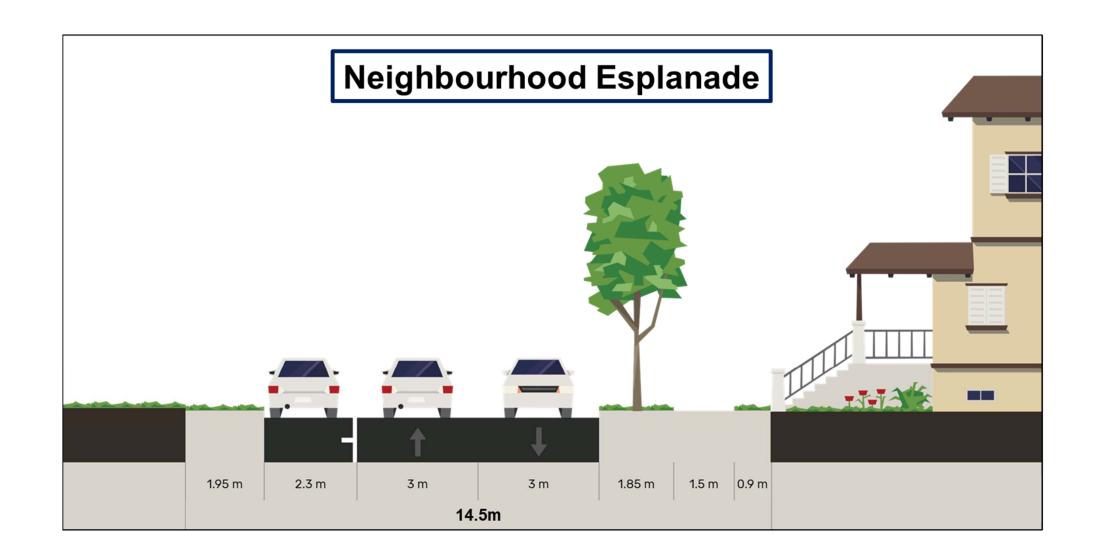


Neighbourhood Access 16.5m



Neighbourhood Lane







Appendix C: Key Intersection SIDRA Summaries



NETWORK LAYOUT

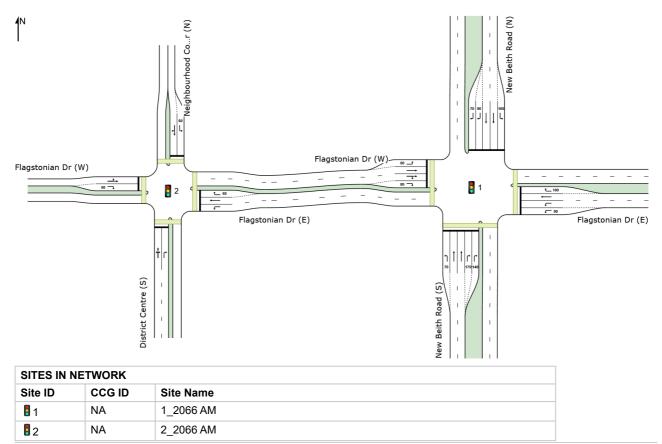
■■ Network: N101 [2066 District Centre AM (Network Folder:

General)]

New Network

Network Category: (None)

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



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Project: P:\P2300 Flagstone City Stages 2-5\Technical\Models\SIDRA\2024_02_16 CA3 Masterplan SIDRAs\P2300.001M 2066 CA3 South
Masterplan SIDRA Models.sip9

Site: 1 [1_2066 AM (Site Folder: 2066 AM)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: N101 [2066 District Centre AM (Network Folder: General)]

New Beith Road / Flagstonian Drive

Site Category: (None)

| Vehi | cle M | ovement | Perfo | rma | nce | | | | | | | | | | |
|--------|--------|-----------|--------|-------------|------------------|-------------|----------------|-------|----------|------------|----------|------|--------------|---------------|-------|
| Mov | Turn | Mov | Dem | | | rival | Deg. | Aver. | Level of | Aver. Back | Of Queue | | Eff. | Aver. | Aver. |
| ID | | Class | | ows HV/1 | ا-ا ا Total] | ows HV 1 | Satn | Delay | Service | [Veh. | Dist] | Que | Stop Rate | No. of Cycles | Speed |
| | | | veh/h | | veh/h | % | v/c | sec | | veh | m m | | rtato | O y oloo | km/h |
| South | n: New | Beith Ro | ad (S) | | | | | | | | | | | | |
| 1 | L2 | All MCs | 160 | 2.5 | 160 | 2.5 | 0.166 | 22.4 | LOS C | 1.8 | 13.0 | 0.55 | 0.71 | 0.55 | 45.8 |
| 2 | T1 | All MCs | 1346 | 2.6 | 1346 | 2.6 | 0.847 | 30.2 | LOS C | 20.4 | 146.0 | 0.88 | 0.85 | 0.95 | 53.1 |
| 3 | R2 | All MCs | 675 | 1.3 | 675 | 1.3 | * 0.877 | 62.4 | LOS E | 12.4 | 87.6 | 1.00 | 0.98 | 1.24 | 30.5 |
| Appro | oach | | 2181 | 2.2 | 2181 | 2.2 | 0.877 | 39.6 | LOS D | 20.4 | 146.0 | 0.90 | 0.88 | 1.01 | 43.9 |
| East: | Flagst | tonian Dr | (E) | | | | | | | | | | | | |
| 4 | L2 | All MCs | 240 | 2.5 | 240 | 2.5 | 0.315 | 46.5 | LOS D | 3.4 | 24.2 | 0.89 | 0.78 | 0.89 | 35.0 |
| 5 | T1 | All MCs | 260 | 1.9 | 260 | 1.9 | * 0.646 | 43.5 | LOS D | 7.9 | 56.5 | 0.97 | 0.81 | 0.97 | 16.2 |
| 6 | R2 | All MCs | 221 | 2.7 | 221 | 2.7 | 0.580 | 49.5 | LOS D | 6.7 | 47.7 | 0.95 | 0.82 | 0.95 | 36.6 |
| Appro | oach | | 721 | 2.4 | 721 | 2.4 | 0.646 | 46.3 | LOS D | 7.9 | 56.5 | 0.94 | 0.81 | 0.94 | 31.0 |
| North | : New | Beith Roa | ad (N) | | | | | | | | | | | | |
| 7 | L2 | All MCs | 370 | 3.2 | 370 | 3.2 | 0.477 | 19.8 | LOS B | 5.7 | 40.8 | 0.77 | 0.80 | 0.77 | 49.5 |
| 8 | T1 | All MCs | 789 | 6.6 | 789 | 6.6 | 0.766 | 41.8 | LOS D | 12.4 | 91.9 | 0.98 | 0.88 | 1.03 | 46.4 |
| 9 | R2 | All MCs | 178 | 2.2 | 178 | 2.2 | * 0.893 | 73.5 | LOS E | 3.4 | 24.3 | 1.00 | 0.98 | 1.49 | 25.0 |
| Appro | oach | | 1337 | 5.1 | 1337 | 5.1 | 0.893 | 39.9 | LOS D | 12.4 | 91.9 | 0.92 | 0.87 | 1.02 | 43.8 |
| West | Flags | tonian Dr | (W) | | | | | | | | | | | | |
| 10 | L2 | All MCs | 274 | 3.3 | 274 | 3.3 | 0.722 | 51.2 | LOS D | 8.7 | 62.9 | 0.99 | 0.86 | 1.04 | 32.2 |
| 11 | T1 | All MCs | 346 | 1.4 | 346 | 1.4 | * 0.896 | 64.8 | LOS E | 6.5 | 46.4 | 1.00 | 1.03 | 1.39 | 17.6 |
| 12 | R2 | All MCs | 155 | 0.0 | 155 | 0.0 | 0.835 | 65.6 | LOS E | 5.6 | 39.1 | 1.00 | 0.96 | 1.27 | 27.1 |
| Appro | oach | | 775 | 1.8 | 775 | 1.8 | 0.896 | 60.2 | LOS E | 8.7 | 62.9 | 1.00 | 0.96 | 1.24 | 25.5 |
| All Ve | hicles | | 5014 | 2.9 | 5014 | 2.9 | 0.896 | 43.8 | LOS D | 20.4 | 146.0 | 0.92 | 0.88 | 1.04 | 39.5 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

| Ped | destrian Mo | vement | Perforr | nance | | | | | | | |
|-----------|---------------|--------------|----------------|---------------------|----------------|-------------|--------------|--------------|----------------|-----------------|----------------|
| Mo\ ID | / Crossing | Dem. Flow | Aver. Delay | Level of Service | AVERAGE QUE | | Prop. Que | Eff. Stop | Travel Time | Travel Dist. | Aver. Speed |
| | | ped/h | sec | | [Ped ped | Dist] m | | Rate | sec | m | m/sec |
| Sou | th: New Beith | | | | | | | | | | |
| P1 | Full | 50 | 48.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | 63.7 | 20.0 | 0.31 |

| East: Flagstonian | Dr (E) | | | | | | | | | |
|-------------------|--------|------|-------|-----|-----|------|------|-------|-------|------|
| P2 Full | 50 | 48.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | 63.7 | 20.0 | 0.31 |
| West: Flagstonian | Dr (W) | | | | | | | | | |
| P4 Full | 50 | 49.3 | LOS E | 0.1 | 0.1 | 0.95 | 0.95 | 203.1 | 200.0 | 0.98 |
| All Pedestrians | 150 | 48.6 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | 110.2 | 80.0 | 0.73 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Project: P:\P2300 Flagstone City Stages 2-5\Technical\Models\SIDRA\2024_02_16 CA3 Masterplan SIDRAs\P2300.001M 2066 CA3 South Masterplan SIDRA Models.sip9

Site: 2 [2_2066 AM (Site Folder: 2066 AM)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: N101 [2066 District Centre AM (Network Folder: General)]

New Site

Site Category: (None)

| Vehi | cle M | ovement | Perfo | rma | nce | | | | | | | | | | |
|--------|----------|------------|--------|-------------|-----------------|-------------|----------------|-------|----------|------------|----------|------|--------------|---------------|-------|
| Mov | Turn | Mov | Dem | | | rival | Deg. | Aver. | Level of | Aver. Back | Of Queue | | Eff. | Aver. | Aver. |
| ID | | Class | | ows HV 1 | اء ا Total] | ows HV 1 | Satn | Delay | Service | [Veh. | Dist] | Que | Stop Rate | No. of Cycles | Speed |
| | | | veh/h | | veh/h | % | v/c | sec | | veh | m | | . 15.15 | 0,0.00 | km/h |
| South | n: Distr | ict Centre | e (S) | | | | | | | | | | | | |
| 1 | L2 | All MCs | 2 | 0.0 | 2 | 0.0 | 0.173 | 39.5 | LOS D | 1.0 | 7.0 | 0.90 | 0.71 | 0.90 | 18.0 |
| 2 | T1 | All MCs | 7 | 0.0 | 7 | 0.0 | 0.173 | 30.1 | LOS C | 1.0 | 7.0 | 0.90 | 0.71 | 0.90 | 27.8 |
| 3 | R2 | All MCs | 89 | 3.5 | 89 | 3.5 | 0.173 | 33.5 | LOS C | 1.0 | 7.3 | 0.90 | 0.72 | 0.90 | 9.8 |
| Appro | oach | | 99 | 3.2 | 99 | 3.2 | 0.173 | 33.4 | LOS C | 1.0 | 7.3 | 0.90 | 0.72 | 0.90 | 12.3 |
| East: | Flagst | onian Dr | (E) | | | | | | | | | | | | |
| 4 | L2 | All MCs | 297 | 2.1 | 297 | 2.1 | * 0.676 | 23.8 | LOS C | 4.1 | 29.5 | 0.96 | 0.86 | 1.00 | 23.4 |
| 5 | T1 | All MCs | 301 | 2.1 | 301 | 2.1 | 0.533 | 24.4 | LOS C | 5.8 | 41.1 | 0.89 | 0.75 | 0.89 | 28.0 |
| 6 | R2 | All MCs | 29 | 3.6 | 29 | 3.6 | * 0.407 | 48.3 | LOS D | 0.7 | 5.3 | 1.00 | 0.71 | 1.00 | 25.7 |
| Appro | oach | | 627 | 2.2 | 627 | 2.2 | 0.676 | 25.2 | LOS C | 5.8 | 41.1 | 0.93 | 0.80 | 0.95 | 25.7 |
| North | : Neig | hbourhoo | d Conn | ecto | r (N) | | | | | | | | | | |
| 7 | L2 | All MCs | 173 | 1.8 | 173 | 1.8 | 0.353 | 29.0 | LOS C | 3.2 | 22.7 | 0.85 | 0.78 | 0.85 | 28.1 |
| 8 | T1 | All MCs | 27 | 0.0 | 27 | 0.0 | * 0.090 | 29.0 | LOS C | 0.6 | 4.3 | 0.86 | 0.67 | 0.86 | 29.4 |
| 9 | R2 | All MCs | 5 | 0.0 | 5 | 0.0 | 0.090 | 32.2 | LOS C | 0.6 | 4.3 | 0.86 | 0.67 | 0.86 | 31.0 |
| Appro | oach | | 205 | 1.5 | 205 | 1.5 | 0.353 | 29.1 | LOS C | 3.2 | 22.7 | 0.85 | 0.76 | 0.85 | 28.3 |
| West | : Flags | tonian Dr | (W) | | | | | | | | | | | | |
| 10 | L2 | All MCs | 2 | 0.0 | 2 | 0.0 | 0.816 | 37.9 | LOS D | 12.7 | 90.3 | 0.97 | 0.95 | 1.10 | 31.8 |
| 11 | T1 | All MCs | 555 | 1.9 | 555 | 1.9 | * 0.816 | 32.3 | LOS C | 12.7 | 90.3 | 0.97 | 0.95 | 1.10 | 16.4 |
| 12 | R2 | All MCs | 46 | 4.5 | 46 | 4.5 | 0.215 | 43.5 | LOS D | 1.0 | 7.2 | 0.94 | 0.73 | 0.94 | 16.7 |
| Appro | oach | | 603 | 2.1 | 603 | 2.1 | 0.816 | 33.2 | LOS C | 12.7 | 90.3 | 0.97 | 0.93 | 1.09 | 15.0 |
| All Ve | hicles | | 1535 | 2.1 | 1535 | 2.1 | 0.816 | 29.4 | LOS C | 12.7 | 90.3 | 0.93 | 0.84 | 0.99 | 21.5 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

| Ped | destrian Mo | vement | Perforr | nance | | | | | | | |
|-----------|-----------------|--------------|----------------|---------------------|---------------|----------------|--------------|--------------|----------------|-----------------|----------------|
| Mov ID | / Crossing | Dem. Flow | Aver. Delay | Level of Service | AVERAGE QU | BACK OF EUE | Prop. Que | Eff. Stop | Travel Time | Travel Dist. | Aver. Speed |
| | | ped/h | sec | | [Ped ped | Dist] m | | Rate | sec | m | m/sec |
| Sou | th: District Co | | | | 700 | | | | | | ,, |
| P1 | Full | 50 | 31.8 | LOS D | 0.1 | 0.1 | 0.92 | 0.92 | 185.7 | 200.0 | 1.08 |

| East: Flagstonian | Dr (E) | | | | | | | | | |
|-------------------|----------|--------|-------|-----|-----|------|------|-------|-------|------|
| P2 Full | 50 | 31.8 | LOS D | 0.1 | 0.1 | 0.92 | 0.92 | 185.7 | 200.0 | 1.08 |
| North: Neighbourh | nood Cor | nector | (N) | | | | | | | |
| P3 Full | 50 | 31.8 | LOS D | 0.1 | 0.1 | 0.92 | 0.92 | 185.7 | 200.0 | 1.08 |
| West: Flagstonian | Dr (W) | | | | | | | | | |
| P4 Full | 50 | 31.8 | LOS D | 0.1 | 0.1 | 0.92 | 0.92 | 185.7 | 200.0 | 1.08 |
| All Pedestrians | 200 | 31.8 | LOS D | 0.1 | 0.1 | 0.92 | 0.92 | 185.7 | 200.0 | 1.08 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA INTERSECTION 9.1 | Copyright © 2000-2024 Akcelik and Associates Pty Ltd | sidrasolutions.com
Organisation: BITZIOS CONSULTING | Licence: PLUS / FLOATING | Processed: Friday, 15 March 2024 2:16:05 PM
Project: P:\P2300 Flagstone City Stages 2-5\Technical\Models\SIDRA\2024_02_16 CA3 Masterplan SIDRAs\P2300.001M 2066 CA3 South
Masterplan SIDRA Models.sip9

Site: 1 [1_2066 AM_Sensitivity (Site Folder: 2066 AM)]
Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: N101 [2066 District Centre AM_Sensitivity (Network Folder: General)]

New Beith Road / Flagstonian Drive

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 100 seconds (Site Practical Cycle Time)

| Vehi | cle M | ovement | Perfo | rma | nce | | | | | | | | | | |
|--------|---------|-----------|--------|-------------|-----------------|-------------|----------------|-------|----------|------------|----------|------|--------------|---------------|-------|
| Mov | Turn | Mov | Dem | | | rival | Deg. | Aver. | Level of | Aver. Back | Of Queue | | Eff. | Aver. | Aver. |
| ID | | Class | | ows HV 1 | اء ا Total] | ows HV 1 | Satn | Delay | Service | [Veh. | Dist] | Que | Stop Rate | No. of Cycles | Speed |
| | | | veh/h | | veh/h | % | v/c | sec | | veh | m | | | | km/h |
| South | n: New | Beith Ro | ad (S) | | | | | | | | | | | | |
| 1 | L2 | All MCs | 160 | 2.5 | 160 | 2.5 | 0.159 | 17.4 | LOS B | 1.5 | 10.4 | 0.52 | 0.70 | 0.52 | 47.2 |
| 2 | T1 | All MCs | 1346 | 2.6 | 1346 | 2.6 | 0.815 | 23.6 | LOS C | 17.2 | 123.0 | 0.84 | 0.79 | 0.89 | 55.7 |
| 3 | R2 | All MCs | 675 | 1.3 | 675 | 1.3 | * 0.874 | 57.8 | LOS E | 11.3 | 80.2 | 1.00 | 0.98 | 1.26 | 31.8 |
| Appro | oach | | 2181 | 2.2 | 2181 | 2.2 | 0.874 | 33.7 | LOS C | 17.2 | 123.0 | 0.87 | 0.85 | 0.97 | 46.4 |
| East: | Flagst | tonian Dr | (E) | | | | | | | | | | | | |
| 4 | L2 | All MCs | 240 | 2.5 | 240 | 2.5 | 0.227 | 35.5 | LOS D | 2.7 | 19.6 | 0.80 | 0.77 | 0.80 | 39.3 |
| 5 | T1 | All MCs | 260 | 1.9 | 260 | 1.9 | * 0.844 | 51.3 | LOS D | 8.5 | 60.4 | 1.00 | 0.99 | 1.23 | 14.4 |
| 6 | R2 | All MCs | 221 | 2.7 | 221 | 2.7 | 0.758 | 53.8 | LOS D | 6.8 | 48.7 | 1.00 | 0.89 | 1.12 | 35.2 |
| Appro | oach | | 721 | 2.4 | 721 | 2.4 | 0.844 | 46.8 | LOS D | 8.5 | 60.4 | 0.93 | 0.89 | 1.06 | 30.9 |
| North | : New | Beith Roa | ad (N) | | | | | | | | | | | | |
| 7 | L2 | All MCs | 370 | 3.2 | 370 | 3.2 | 0.536 | 20.7 | LOS C | 5.6 | 39.9 | 0.83 | 0.81 | 0.83 | 48.9 |
| 8 | T1 | All MCs | 789 | 6.6 | 789 | 6.6 | 0.746 | 37.0 | LOS D | 11.2 | 82.5 | 0.97 | 0.87 | 1.02 | 48.2 |
| 9 | R2 | All MCs | 178 | 2.2 | 178 | 2.2 | * 0.696 | 59.4 | LOS E | 2.8 | 20.3 | 1.00 | 0.85 | 1.15 | 28.2 |
| Appro | oach | | 1337 | 5.1 | 1337 | 5.1 | 0.746 | 35.5 | LOS D | 11.2 | 82.5 | 0.94 | 0.85 | 0.98 | 45.6 |
| West | : Flags | tonian Dr | (W) | | | | | | | | | | | | |
| 10 | L2 | All MCs | 274 | 3.3 | 274 | 3.3 | 0.629 | 42.8 | LOS D | 7.5 | 53.7 | 0.95 | 0.83 | 0.95 | 34.7 |
| 11 | T1 | All MCs | 346 | 1.4 | 346 | 1.4 | * 0.814 | 53.1 | LOS D | 5.6 | 39.7 | 1.00 | 0.95 | 1.24 | 20.2 |
| 12 | R2 | All MCs | 155 | 0.0 | 155 | 0.0 | 0.759 | 56.8 | LOS E | 4.9 | 34.3 | 1.00 | 0.90 | 1.17 | 29.2 |
| Appro | oach | | 775 | 1.8 | 775 | 1.8 | 0.814 | 50.2 | LOS D | 7.5 | 53.7 | 0.98 | 0.90 | 1.12 | 28.1 |
| All Ve | hicles | | 5014 | 2.9 | 5014 | 2.9 | 0.874 | 38.6 | LOS D | 17.2 | 123.0 | 0.91 | 0.86 | 1.01 | 41.5 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

| Pedestrian Mo | vement | Perforr | nance | | | | | | | | | | | | |
|--------------------|---------------------------|----------------|---------------------|----------------|----------------|--------------|--------------|----------------|-----------------|----------------|--|--|--|--|--|
| Mov ID Crossing | Dem. Flow | Aver. Delay | Level of Service | AVERAGE QUE | BACK OF EUE | Prop. Que | Eff. Stop | Travel Time | Travel Dist. | Aver. Speed | | | | | |
| | ped/h | sec | | [Ped ped | Dist] m | | Rate | sec | m | m/sec | | | | | |
| South: New Beith | South: New Beith Road (S) | | | | | | | | | | | | | | |
| P1 Full | 20 | 43.3 | LOS E | 0.1 | 0.1 | 0.93 | 0.93 | 58.7 | 20.0 | 0.34 | | | | | |

| East: Flagstonian | Dr (E) | | | | | | | | | |
|-------------------|--------|------|-------|-----|-----|------|------|-------|-------|------|
| P2 Full | 50 | 43.3 | LOS E | 0.1 | 0.1 | 0.93 | 0.93 | 58.7 | 20.0 | 0.34 |
| West: Flagstonian | Dr (W) | | | | | | | | | |
| P4 Full | 50 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | 198.1 | 200.0 | 1.01 |
| All Pedestrians | 120 | 43.7 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | 116.8 | 95.0 | 0.81 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA INTERSECTION 9.1 | Copyright © 2000-2024 Akcelik and Associates Pty Ltd | sidrasolutions.com
Organisation: BITZIOS CONSULTING | Licence: PLUS / FLOATING | Processed: Friday, 15 March 2024 2:16:08 PM
Project: P:\P2300 Flagstone City Stages 2-5\Technical\Models\SIDRA\2024_02_16 CA3 Masterplan SIDRAs\P2300.001M 2066 CA3 South Masterplan SIDRA Models.sip9

Site: 2 [2_2066 AM_Sensitivity (Site Folder: 2066 AM)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: N101 [2066 District Centre AM_Sensitivity (Network Folder: General)]

New Site

Site Category: (None)

| Class | Vehic | cle M | ovemen | t Perfo | rma | nce | | | | | | | | | | |
|--|--------|---------|------------|---------|------|-------|-----|----------------|-------|---------|------------|----------|------|---------|------|-------|
| South: District Centre (S) | | Turn | | | | | | | | | Aver. Back | Of Queue | | | | Aver. |
| South: District Centre (S) South: District Centre (S) 1 | טו | | Class | | | | | Satn | Delay | Service | ſ Veh. | Dist 1 | Que | | | Speed |
| 1 L2 All MCs 2 0.0 2 0.0 0.173 39.5 LOS D 1.0 7.0 0.90 0.71 0.90 18.0 2 T1 All MCs 7 0.0 7 0.0 0.173 30.1 LOS C 1.0 7.0 0.90 0.71 0.90 27.6 3 R2 All MCs 89 3.5 89 3.5 0.173 33.5 LOS C 1.0 7.3 0.90 0.72 0.90 9.6 Approach 99 3.2 99 3.2 0.173 33.4 LOS C 1.0 7.3 0.90 0.72 0.90 12.3 East: Flagstonian Dr (E) 4 L2 All MCs 297 2.1 297 2.1 *0.676 23.8 LOS C 4.1 29.5 0.96 0.86 1.00 23.4 5 T1 All MCs 301 2.1 301 2.1 0.533 24.4 LOS C 5.8 41.1 0.89 0.75 0.89 28.0 6 R2 All MCs 29 3.6 29 3.6 *0.407 48.3 LOS D 0.7 5.3 1.00 0.71 1.00 25.3 Approach 627 2.2 627 2.2 0.676 25.2 LOS C 5.8 41.1 0.93 0.80 0.95 25.3 North: Neighbourhood Connector (N) 7 L2 All MCs 173 1.8 173 1.8 0.353 29.0 LOS C 3.2 22.7 0.85 0.78 0.85 28.3 8 T1 All MCs 27 0.0 27 0.0 *0.090 29.0 LOS C 0.6 4.3 0.86 0.67 0.86 29.4 9 R2 All MCs 2 0.0 2 7 0.0 *0.090 32.2 LOS C 0.6 4.3 0.86 0.67 0.86 29.4 9 R2 All MCs 5 0.0 5 0.0 0.090 32.2 LOS C 0.6 4.3 0.86 0.67 0.86 31.0 Approach 205 1.5 205 1.5 0.353 29.1 LOS C 3.2 22.7 0.85 0.76 0.85 28.3 West: Flagstonian Dr (W) 10 L2 All MCs 2 0.0 2 0.0 0.816 37.9 LOS D 12.7 90.3 0.97 0.95 1.10 16.4 Approach 603 2.1 603 2.1 0.816 32.3 LOS C 12.7 90.3 0.97 0.95 1.10 16.4 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.95 1.10 16.4 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.6 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.6 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.6 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.6 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.6 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.6 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.6 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.6 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.6 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.6 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.6 Approach 603 | | | | | | | | v/c | sec | | | | | . 15.15 | | km/h |
| 2 T1 All MCs 7 0.0 7 0.0 0.173 30.1 LOS C 1.0 7.0 0.90 0.71 0.90 27.6 3 R2 All MCs 89 3.5 89 3.5 0.173 33.5 LOS C 1.0 7.3 0.90 0.72 0.90 9.6 Approach 99 3.2 99 3.2 0.173 33.4 LOS C 1.0 7.3 0.90 0.72 0.90 12.3 East: Flagstonian Dr (E) 4 L2 All MCs 297 2.1 297 2.1 *0.676 23.8 LOS C 4.1 29.5 0.96 0.86 1.00 23.4 5 T1 All MCs 301 2.1 301 2.1 0.533 24.4 LOS C 5.8 41.1 0.89 0.75 0.89 28.6 6 R2 All MCs 29 3.6 29 3.6 *0.407 48.3 LOS D 0.7 5.3 1.00 0.71 1.00 25.7 Approach 627 2.2 627 2.2 0.676 25.2 LOS C 5.8 41.1 0.93 0.80 0.95 25.7 North: Neighbourhood Connector (N) 7 L2 All MCs 173 1.8 173 1.8 0.353 29.0 LOS C 3.2 22.7 0.85 0.78 0.85 28.3 8 T1 All MCs 27 0.0 27 0.0 *0.090 29.0 LOS C 0.6 4.3 0.86 0.67 0.86 29.4 9 R2 All MCs 5 0.0 5 0.0 0.090 32.2 LOS C 0.6 4.3 0.86 0.67 0.86 31.0 Approach 205 1.5 205 1.5 0.353 29.1 LOS C 3.2 22.7 0.85 0.76 0.85 28.3 West: Flagstonian Dr (W) 10 L2 All MCs 2 0.0 2 0.0 0.816 37.9 LOS D 12.7 90.3 0.97 0.95 1.10 16.4 11 T1 All MCs 555 1.9 555 1.9 *0.816 32.3 LOS C 12.7 90.3 0.97 0.95 1.10 16.4 12 R2 All MCs 46 4.5 46 4.5 0.215 43.5 LOS D 1.0 7.2 0.94 0.73 0.94 16.5 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Appro | South | : Distr | ict Centre | e (S) | | | | | | | | | | | | |
| 3 R2 All MCs 89 3.5 89 3.5 0.173 33.5 LOS C 1.0 7.3 0.90 0.72 0.90 9.8 Approach 99 3.2 99 3.2 0.173 33.4 LOS C 1.0 7.3 0.90 0.72 0.90 12.3 East: Flagstonian Dr (E) 4 L2 All MCs 297 2.1 297 2.1 *0.676 23.8 LOS C 4.1 29.5 0.96 0.86 1.00 23.4 5 T1 All MCs 301 2.1 301 2.1 0.533 24.4 LOS C 5.8 41.1 0.89 0.75 0.89 28.6 6 R2 All MCs 29 3.6 29 3.6 *0.407 48.3 LOS D 0.7 5.3 1.00 0.71 1.00 25.7 Approach 627 2.2 627 2.2 0.676 25.2 LOS C 5.8 41.1 0.93 0.80 0.95 25.7 North: Neighbourhood Connector (N) 7 L2 All MCs 173 1.8 173 1.8 0.353 29.0 LOS C 3.2 22.7 0.85 0.78 0.85 28.8 T1 All MCs 27 0.0 27 0.0 *0.090 29.0 LOS C 0.6 4.3 0.86 0.67 0.86 29.4 9 R2 All MCs 5 0.0 5 0.0 0.090 32.2 LOS C 0.6 4.3 0.86 0.67 0.86 31.0 Approach 205 1.5 205 1.5 0.353 29.1 LOS C 3.2 22.7 0.85 0.76 0.85 28.3 West: Flagstonian Dr (W) 10 L2 All MCs 2 0.0 2 0.0 0.816 37.9 LOS D 12.7 90.3 0.97 0.95 1.10 31.8 11 T1 All MCs 555 1.9 555 1.9 *0.816 32.3 LOS C 12.7 90.3 0.97 0.95 1.10 16.4 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.95 1.10 16.4 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.95 1.10 16.4 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 Approach 603 2.1 603 2.1 603 2.1 603 2.1 603 2.1 603 2.1 603 2.1 603 2 | 1 | L2 | All MCs | 2 | 0.0 | 2 | 0.0 | 0.173 | 39.5 | LOS D | 1.0 | 7.0 | 0.90 | 0.71 | 0.90 | 18.0 |
| Approach 99 3.2 99 3.2 0.173 33.4 LOS C 1.0 7.3 0.90 0.72 0.90 12.3 East: Flagstonian Dr (E) 4 L2 All MCs 297 2.1 297 2.1 *0.676 23.8 LOS C 4.1 29.5 0.96 0.86 1.00 23.4 5 T1 All MCs 301 2.1 301 2.1 0.533 24.4 LOS C 5.8 41.1 0.89 0.75 0.89 28.6 6 R2 All MCs 29 3.6 29 3.6 *29 3.6 *29 3.6 *29 3.6 *29 3.6 *29 3.6 *29 3.6 *29 3.6 *29 3.6 *29 3.6 *29 3.6 *29 3.6 *29 3.6 *29 3.6 *29 3.6 *25.2 LOS C 5.8 41.1 0.93 0.80 0.95 25.3 North: Neighbourhood Connector (N) *20 *20 *20 *20 *20 * | 2 | T1 | All MCs | 7 | 0.0 | 7 | 0.0 | 0.173 | 30.1 | LOS C | 1.0 | 7.0 | 0.90 | 0.71 | 0.90 | 27.8 |
| East: Flagstonian Dr (E) 4 | 3 | R2 | All MCs | 89 | 3.5 | 89 | 3.5 | 0.173 | 33.5 | LOS C | 1.0 | 7.3 | 0.90 | 0.72 | 0.90 | 9.8 |
| 4 L2 All MCs 297 2.1 297 2.1 *0.676 23.8 LOS C 4.1 29.5 0.96 0.86 1.00 23.4 5 T1 All MCs 301 2.1 301 2.1 0.533 24.4 LOS C 5.8 41.1 0.89 0.75 0.89 28.6 6 R2 All MCs 29 3.6 29 3.6 *0.407 48.3 LOS D 0.7 5.3 1.00 0.71 1.00 25.7 Approach 627 2.2 627 2.2 0.676 25.2 LOS C 5.8 41.1 0.93 0.80 0.95 25.7 North: Neighbourhood Connector (N) 7 L2 All MCs 173 1.8 173 1.8 0.353 29.0 LOS C 3.2 22.7 0.85 0.78 0.85 28.3 T1 All MCs 27 0.0 27 0.0 *0.090 29.0 LOS C 0.6 4.3 0.86 0.67 0.86 29.4 9 R2 All MCs 5 0.0 5 0.0 0.090 32.2 LOS C 0.6 4.3 0.86 0.67 0.86 31.0 Approach 205 1.5 205 1.5 0.353 29.1 LOS C 3.2 22.7 0.85 0.76 0.85 28.3 West: Flagstonian Dr (W) 10 L2 All MCs 2 0.0 2 0.0 0.816 37.9 LOS D 12.7 90.3 0.97 0.95 1.10 31.8 11 T1 All MCs 555 1.9 555 1.9 *0.816 32.3 LOS C 12.7 90.3 0.97 0.95 1.10 16.4 12 R2 All MCs 46 4.5 46 4.5 0.215 43.5 LOS D 1.0 7.2 0.94 0.73 0.94 16.5 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.95 1.09 15.0 | Appro | ach | | 99 | 3.2 | 99 | 3.2 | 0.173 | 33.4 | LOS C | 1.0 | 7.3 | 0.90 | 0.72 | 0.90 | 12.3 |
| 5 T1 All MCs 301 2.1 301 2.1 0.533 24.4 LOS C 5.8 41.1 0.89 0.75 0.89 28.0 6 R2 All MCs 29 3.6 29 3.6 *0.407 48.3 LOS D 0.7 5.3 1.00 0.71 1.00 25.7 Approach 627 2.2 627 2.2 0.676 25.2 LOS C 5.8 41.1 0.93 0.80 0.95 25.7 North: Neighbourhood Connector (N) 7 L2 All MCs 173 1.8 173 1.8 0.353 29.0 LOS C 3.2 22.7 0.85 0.78 0.85 28.3 8 T1 All MCs 27 0.0 27 0.0 *0.090 29.0 LOS C 0.6 4.3 0.86 0.67 0.86 29.4 9 R2 All MCs 5 0.0 5 0.0 0.090 32.2 LOS C 0.6 4.3 0.86 0.67 0.86 31.0 Approach 205 1.5 205 1.5 0.353 29.1 LOS C 3.2 22.7 0.85 0.76 0.85 28.3 West: Flagstonian Dr (W) 10 L2 All MCs 2 0.0 2 0.0 0.816 37.9 LOS D 12.7 90.3 0.97 0.95 1.10 31.6 11 T1 All MCs 555 1.9 555 1.9 *0.816 32.3 LOS C 12.7 90.3 0.97 0.95 1.10 16.4 12 R2 All MCs 46 4.5 46 4.5 0.215 43.5 LOS D 1.0 7.2 0.94 0.73 0.94 16.7 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 | East: | Flagst | onian Dr | (E) | | | | | | | | | | | | |
| 6 R2 All MCs 29 3.6 29 3.6 *0.407 48.3 LOS D 0.7 5.3 1.00 0.71 1.00 25.7 Approach 627 2.2 627 2.2 0.676 25.2 LOS C 5.8 41.1 0.93 0.80 0.95 25.7 North: Neighbourhood Connector (N) 7 L2 All MCs 173 1.8 173 1.8 0.353 29.0 LOS C 3.2 22.7 0.85 0.78 0.85 28.7 8 T1 All MCs 27 0.0 27 0.0 *0.090 29.0 LOS C 0.6 4.3 0.86 0.67 0.86 29.4 9 R2 All MCs 5 0.0 5 0.0 0.090 32.2 LOS C 0.6 4.3 0.86 0.67 0.86 31.0 Approach 205 1.5 205 1.5 0.353 29.1 LOS C 3.2 22.7 0.85 0.76 0.85 28.3 West: Flagstonian Dr (W) 10 L2 All MCs 2 0.0 2 0.0 0.816 37.9 LOS D 12.7 90.3 0.97 0.95 1.10 31.8 11 T1 All MCs 555 1.9 555 1.9 *0.816 32.3 LOS C 12.7 90.3 0.97 0.95 1.10 16.4 12 R2 All MCs 46 4.5 46 4.5 0.215 43.5 LOS D 1.0 7.2 0.94 0.73 0.94 16.7 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 | 4 | L2 | All MCs | 297 | 2.1 | 297 | 2.1 | * 0.676 | 23.8 | LOS C | 4.1 | 29.5 | 0.96 | 0.86 | 1.00 | 23.4 |
| Approach 627 2.2 627 2.2 0.676 25.2 LOS C 5.8 41.1 0.93 0.80 0.95 25.7 North: Neighbourhood Connector (N) 7 L2 All MCs 173 1.8 173 1.8 0.353 29.0 LOS C 3.2 22.7 0.85 0.78 0.85 28.7 8 T1 All MCs 27 0.0 27 0.0 *0.090 29.0 LOS C 0.6 4.3 0.86 0.67 0.86 29.4 9 R2 All MCs 5 0.0 5 0.0 0.090 32.2 LOS C 0.6 4.3 0.86 0.67 0.86 31.0 Approach 205 1.5 205 1.5 0.353 29.1 LOS C 3.2 22.7 0.85 0.76 0.85 28.3 West: Flagstonian Dr (W) 10 L2 All MCs 2 0.0 2 0.0 0.816 37.9 LOS D </td <td>5</td> <td>T1</td> <td>All MCs</td> <td>301</td> <td>2.1</td> <td>301</td> <td>2.1</td> <td>0.533</td> <td>24.4</td> <td>LOS C</td> <td>5.8</td> <td>41.1</td> <td>0.89</td> <td>0.75</td> <td>0.89</td> <td>28.0</td> | 5 | T1 | All MCs | 301 | 2.1 | 301 | 2.1 | 0.533 | 24.4 | LOS C | 5.8 | 41.1 | 0.89 | 0.75 | 0.89 | 28.0 |
| North: Neighbourhood Connector (N) 7 | 6 | R2 | All MCs | 29 | 3.6 | 29 | 3.6 | * 0.407 | 48.3 | LOS D | 0.7 | 5.3 | 1.00 | 0.71 | 1.00 | 25.7 |
| 7 L2 All MCs 173 1.8 173 1.8 0.353 29.0 LOS C 3.2 22.7 0.85 0.78 0.85 28.7 8 T1 All MCs 27 0.0 27 0.0 *0.090 29.0 LOS C 0.6 4.3 0.86 0.67 0.86 29.4 9 R2 All MCs 5 0.0 5 0.0 0.090 32.2 LOS C 0.6 4.3 0.86 0.67 0.86 31.0 Approach 205 1.5 205 1.5 0.353 29.1 LOS C 3.2 22.7 0.85 0.76 0.85 28.3 West: Flagstonian Dr (W) 10 L2 All MCs 2 0.0 2 0.0 0.816 37.9 LOS D 12.7 90.3 0.97 0.95 1.10 31.8 11 T1 All MCs 555 1.9 555 1.9 *0.816 32.3 LOS C 12.7 90.3 0.97 0.95 1.10 16.4 12 R2 All MCs 46 4.5 46 4.5 0.215 43.5 LOS D 1.0 7.2 0.94 0.73 0.94 16.7 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 | Appro | ach | | 627 | 2.2 | 627 | 2.2 | 0.676 | 25.2 | LOS C | 5.8 | 41.1 | 0.93 | 0.80 | 0.95 | 25.7 |
| 8 T1 All MCs 27 0.0 27 0.0 *0.090 29.0 LOS C 0.6 4.3 0.86 0.67 0.86 29.4 9 R2 All MCs 5 0.0 5 0.0 0.090 32.2 LOS C 0.6 4.3 0.86 0.67 0.86 31.0 Approach 205 1.5 205 1.5 0.353 29.1 LOS C 3.2 22.7 0.85 0.76 0.85 28.3 West: Flagstonian Dr (W) 10 L2 All MCs 2 0.0 2 0.0 0.816 37.9 LOS D 12.7 90.3 0.97 0.95 1.10 31.6 11 T1 All MCs 555 1.9 *0.816 32.3 LOS C 12.7 90.3 0.97 0.95 1.10 16.4 12 R2 All MCs 46 4.5 46 4.5 0.215 43.5 LOS D 1.0 7.2 0.94 0.73 0.94 16.7 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 | North | : Neig | hbourhoo | d Conn | ecto | r (N) | | | | | | | | | | |
| 9 R2 All MCs 5 0.0 5 0.0 0.090 32.2 LOS C 0.6 4.3 0.86 0.67 0.86 31.0 Approach 205 1.5 205 1.5 0.353 29.1 LOS C 3.2 22.7 0.85 0.76 0.85 28.3 West: Flagstonian Dr (W) 10 L2 All MCs 2 0.0 2 0.0 0.816 37.9 LOS D 12.7 90.3 0.97 0.95 1.10 31.6 11 T1 All MCs 555 1.9 555 1.9 *0.816 32.3 LOS C 12.7 90.3 0.97 0.95 1.10 16.4 12 R2 All MCs 46 4.5 46 4.5 0.215 43.5 LOS D 1.0 7.2 0.94 0.73 0.94 16.7 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 | 7 | L2 | All MCs | 173 | 1.8 | 173 | 1.8 | 0.353 | 29.0 | LOS C | 3.2 | 22.7 | 0.85 | 0.78 | 0.85 | 28.1 |
| Approach 205 1.5 205 1.5 0.353 29.1 LOS C 3.2 22.7 0.85 0.76 0.85 28.3 West: Flagstonian Dr (W) 10 L2 All MCs 2 0.0 2 0.0 0.816 37.9 LOS D 12.7 90.3 0.97 0.95 1.10 31.8 11 T1 All MCs 555 1.9 \$555 1.9 *0.816 32.3 LOS C 12.7 90.3 0.97 0.95 1.10 16.4 12 R2 All MCs 46 4.5 46 4.5 0.215 43.5 LOS D 1.0 7.2 0.94 0.73 0.94 16.7 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 | 8 | T1 | All MCs | 27 | 0.0 | 27 | 0.0 | * 0.090 | 29.0 | LOS C | 0.6 | 4.3 | 0.86 | 0.67 | 0.86 | 29.4 |
| West: Flagstonian Dr (W) 10 L2 All MCs 2 0.0 2 0.0 0.816 37.9 LOS D 12.7 90.3 0.97 0.95 1.10 31.8 11 T1 All MCs 555 1.9 555 1.9 *0.816 32.3 LOS C 12.7 90.3 0.97 0.95 1.10 16.4 12 R2 All MCs 46 4.5 46 4.5 0.215 43.5 LOS D 1.0 7.2 0.94 0.73 0.94 16.7 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 | 9 | R2 | All MCs | 5 | 0.0 | 5 | 0.0 | 0.090 | 32.2 | LOS C | 0.6 | 4.3 | 0.86 | 0.67 | 0.86 | 31.0 |
| 10 L2 All MCs 2 0.0 2 0.0 0.816 37.9 LOS D 12.7 90.3 0.97 0.95 1.10 31.6 11 T1 All MCs 555 1.9 555 1.9 *0.816 32.3 LOS C 12.7 90.3 0.97 0.95 1.10 16.4 12 R2 All MCs 46 4.5 46 4.5 0.215 43.5 LOS D 1.0 7.2 0.94 0.73 0.94 16.7 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 | Appro | ach | | 205 | 1.5 | 205 | 1.5 | 0.353 | 29.1 | LOS C | 3.2 | 22.7 | 0.85 | 0.76 | 0.85 | 28.3 |
| 11 T1 All MCs 555 1.9 555 1.9 *0.816 32.3 LOS C 12.7 90.3 0.97 0.95 1.10 16.4 12 R2 All MCs 46 4.5 46 4.5 0.215 43.5 LOS D 1.0 7.2 0.94 0.73 0.94 16.7 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 | West: | Flags | tonian Dı | · (W) | | | | | | | | | | | | |
| 12 R2 All MCs 46 4.5 46 4.5 0.215 43.5 LOS D 1.0 7.2 0.94 0.73 0.94 16.7 Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 | 10 | L2 | All MCs | 2 | 0.0 | 2 | 0.0 | 0.816 | 37.9 | LOS D | 12.7 | 90.3 | 0.97 | 0.95 | 1.10 | 31.8 |
| Approach 603 2.1 603 2.1 0.816 33.2 LOS C 12.7 90.3 0.97 0.93 1.09 15.0 | 11 | T1 | All MCs | 555 | 1.9 | 555 | 1.9 | * 0.816 | 32.3 | LOS C | 12.7 | 90.3 | 0.97 | 0.95 | 1.10 | 16.4 |
| | 12 | R2 | All MCs | 46 | 4.5 | 46 | 4.5 | 0.215 | 43.5 | LOS D | 1.0 | 7.2 | 0.94 | 0.73 | 0.94 | 16.7 |
| All Vehicles 1535 2.1 1535 2.1 0.816 29.4 LOS C 12.7 90.3 0.93 0.84 0.99 21.5 | Appro | ach | | 603 | 2.1 | 603 | 2.1 | 0.816 | 33.2 | LOS C | 12.7 | 90.3 | 0.97 | 0.93 | 1.09 | 15.0 |
| | All Ve | hicles | | 1535 | 2.1 | 1535 | 2.1 | 0.816 | 29.4 | LOS C | 12.7 | 90.3 | 0.93 | 0.84 | 0.99 | 21.5 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

| Pedestrian Mo | ovement | Perforr | mance | | | | | | | |
|--------------------|--------------|----------------|---------------------|--------------|-------------|--------------|--------------|----------------|-----------------|----------------|
| Mov ID Crossing | Dem. Flow | Aver. Delay | Level of Service | | EUE | Prop. Que | Eff. Stop | Travel Time | Travel Dist. | Aver. Speed |
| | ped/h | sec | | [Ped ped | Dist] m | | Rate | sec | m | m/sec |
| South: District C | Centre (S) | | | | | | | | | |
| P1 Full | 50 | 31.8 | LOS D | 0.1 | 0.1 | 0.92 | 0.92 | 185.7 | 200.0 | 1.08 |

| East: Flagstonian | Dr (E) | | | | | | | | | |
|-------------------|---------|--------|-------|-----|-----|------|------|-------|-------|------|
| P2 Full | 50 | 31.8 | LOS D | 0.1 | 0.1 | 0.92 | 0.92 | 185.7 | 200.0 | 1.08 |
| North: Neighbourh | ood Cor | nector | (N) | | | | | | | |
| P3 Full | 50 | 31.8 | LOS D | 0.1 | 0.1 | 0.92 | 0.92 | 185.7 | 200.0 | 1.08 |
| West: Flagstonian | Dr (W) | | | | | | | | | |
| P4 Full | 50 | 31.8 | LOS D | 0.1 | 0.1 | 0.92 | 0.92 | 185.7 | 200.0 | 1.08 |
| All Pedestrians | 200 | 31.8 | LOS D | 0.1 | 0.1 | 0.92 | 0.92 | 185.7 | 200.0 | 1.08 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA INTERSECTION 9.1 | Copyright © 2000-2024 Akcelik and Associates Pty Ltd | sidrasolutions.com
Organisation: BITZIOS CONSULTING | Licence: PLUS / FLOATING | Processed: Friday, 15 March 2024 2:16:08 PM
Project: P:\P2300 Flagstone City Stages 2-5\Technical\Models\SIDRA\2024_02_16 CA3 Masterplan SIDRAs\P2300.001M 2066 CA3 South
Masterplan SIDRA Models.sip9

Site: 1 [1_2066 PM (Site Folder: 2066 PM)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: N101 [2066 District Centre PM (Network Folder: General)]

New Beith Road / Flagstonian Drive

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 160 seconds (Site Practical Cycle Time)

| Vehi | cle M | ovement | Perfo | rma | nce | | | | | | | | | | |
|--------|---------|-----------|-----------------|-------------|-------|-------------|----------------|-------|----------|------------|----------|------|--------------|---------------|-------|
| Mov | Turn | Mov | Dem | | | rival | Deg. | Aver. | Level of | Aver. Back | Of Queue | | Eff. | Aver. | Aver. |
| ID | | Class | ا- ا Total] | ows HV 1 | | ows HV 1 | Satn | Delay | Service | [Veh. | Dist] | Que | Stop Rate | No. of Cycles | Speed |
| | | | veh/h | | veh/h | % | v/c | sec | | veh | m m | | rtato | | km/h |
| South | n: New | Beith Ro | ad (S) | | | | | | | | | | | | |
| 1 | L2 | All MCs | 175 | 1.1 | 175 | 1.1 | 0.271 | 44.0 | LOS D | 3.7 | 25.9 | 0.76 | 0.76 | 0.76 | 37.3 |
| 2 | T1 | All MCs | 778 | 1.7 | 778 | 1.7 | 0.797 | 68.5 | LOS E | 18.9 | 134.2 | 1.00 | 0.90 | 1.03 | 40.3 |
| 3 | R2 | All MCs | 191 | 2.1 | 191 | 2.1 | 0.642 | 86.6 | LOS F | 4.6 | 32.9 | 1.00 | 0.81 | 1.04 | 25.2 |
| Appro | oach | | 1144 | 1.7 | 1144 | 1.7 | 0.797 | 67.8 | LOS E | 18.9 | 134.2 | 0.96 | 0.86 | 0.99 | 35.2 |
| East: | Flagst | tonian Dr | (E) | | | | | | | | | | | | |
| 4 | L2 | All MCs | 805 | 0.7 | 805 | 0.7 | * 1.058 | 169.8 | LOS F | 28.2 | 198.5 | 1.00 | 1.16 | 1.61 | 16.8 |
| 5 | T1 | All MCs | 360 | 1.7 | 360 | 1.7 | 0.574 | 47.7 | LOS D | 14.1 | 100.3 | 0.89 | 0.77 | 0.89 | 15.2 |
| 6 | R2 | All MCs | 368 | 2.7 | 368 | 2.7 | 0.622 | 55.4 | LOS E | 14.8 | 105.7 | 0.91 | 0.85 | 0.91 | 34.7 |
| Appro | oach | | 1533 | 1.4 | 1533 | 1.4 | 1.058 | 113.7 | LOS F | 28.2 | 198.5 | 0.95 | 1.00 | 1.27 | 19.0 |
| North | : New | Bieth Roa | ad (N) | | | | | | | | | | | | |
| 7 | L2 | All MCs | 171 | 1.2 | 171 | 1.2 | 0.143 | 37.4 | LOS D | 1.7 | 12.2 | 0.41 | 0.70 | 0.41 | 54.8 |
| 8 | T1 | All MCs | 1324 | 1.0 | 1324 | 1.0 | * 1.063 | 150.9 | LOS F | 44.6 | 314.8 | 1.00 | 1.37 | 1.51 | 27.8 |
| 9 | R2 | All MCs | 321 | 1.2 | 321 | 1.2 | 0.517 | 85.1 | LOS F | 7.3 | 51.4 | 0.96 | 0.81 | 0.96 | 25.6 |
| Appro | oach | | 1816 | 1.0 | 1816 | 1.0 | 1.063 | 128.6 | LOS F | 44.6 | 314.8 | 0.94 | 1.20 | 1.31 | 25.2 |
| West | : Flags | tonian Dr | (W) | | | | | | | | | | | | |
| 10 | L2 | All MCs | 167 | 2.4 | 167 | 2.4 | 0.311 | 52.3 | LOS D | 6.1 | 43.9 | 0.82 | 0.79 | 0.82 | 32.0 |
| 11 | T1 | All MCs | 280 | 3.2 | 280 | 3.2 | * 1.035 | 140.8 | LOS F | 10.5 | 75.3 | 1.00 | 1.18 | 1.64 | 9.6 |
| 12 | R2 | All MCs | 195 | 1.0 | 195 | 1.0 | 1.035 | 146.8 | LOS F | 10.4 | 74.4 | 1.00 | 1.17 | 1.65 | 16.4 |
| Appro | oach | | 642 | 2.3 | 642 | 2.3 | 1.035 | 119.6 | LOS F | 10.5 | 75.3 | 0.95 | 1.08 | 1.43 | 16.3 |
| All Ve | hicles | | 5135 | 1.5 | 5135 | 1.5 | 1.063 | 109.5 | LOS F | 44.6 | 314.8 | 0.95 | 1.05 | 1.24 | 24.2 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

| Pedestrian Mo | ovement | Perforr | nance | | | | | | | |
|--------------------|--------------|----------------|---------------------|-------------------------|-----|--------------|----------------------|----------------|-----------------|----------------|
| Mov ID Crossing | Dem. Flow | Aver. Delay | Level of Service | AVERAGE QUE [Ped | | Prop. Que | Eff. Stop Rate | Travel Time | Travel Dist. | Aver. Speed |
| South: New Beit | ped/h | sec | | ped | m m | | Rate | sec | m | m/sec |
| P1 Full | 50 | 73.3 | LOS F | 0.2 | 0.2 | 0.96 | 0.96 | 88.7 | 20.0 | 0.23 |

| East: Flagstonian | Dr (E) | | | | | | | | | |
|-------------------|--------|------|-------|-----|-----|------|------|-------|-------|------|
| P2 Full | 50 | 73.3 | LOS F | 0.2 | 0.2 | 0.96 | 0.96 | 88.7 | 20.0 | 0.23 |
| West: Flagstonian | Dr (W) | | | | | | | | | |
| P4 Full | 50 | 74.3 | LOS F | 0.2 | 0.2 | 0.96 | 0.96 | 228.1 | 200.0 | 0.88 |
| All Pedestrians | 150 | 73.6 | LOS F | 0.2 | 0.2 | 0.96 | 0.96 | 135.2 | 80.0 | 0.59 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Organisation: BITZIOS CONSULTING | Licence: PLUS / FLOATING | Processed: Friday, 15 March 2024 2:00:35 PM
Project: P:\P2300 Flagstone City Stages 2-5\Technical\Models\SIDRA\2024_02_16 CA3 Masterplan SIDRAs\P2300.001M 2066 CA3 South Masterplan SIDRA Models.sip9

Site: 2 [2_2066 PM (Site Folder: 2066 PM)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: N101 [2066 District Centre PM (Network Folder: General)]

New Site

Site Category: (None)

| Vehi | cle M | ovement | Perfo | rma | nce | | | | | | | | | | |
|--------|----------|------------|--------|--------------|-----------------|-------------|---------|-------|----------|------------|----------|------|--------------|---------------|-------|
| Mov | Turn | Mov | Dem | | | rival | Deg. | Aver. | Level of | Aver. Back | Of Queue | | Eff. | Aver. | Aver. |
| ID | | Class | | lows HV] | اء ا Total] | ows HV] | Satn | Delay | Service | [Veh. | Dist] | Que | Stop Rate | No. of Cycles | Speed |
| | | | veh/h | | veh/h | % | v/c | sec | | veh | m ¹ | | | - , | km/h |
| South | n: Distr | ict Centre | e (S) | | | | | | | | | | | | |
| 1 | L2 | All MCs | 13 | 0.0 | 13 | 0.0 | 0.599 | 44.6 | LOS D | 3.9 | 27.8 | 0.98 | 0.81 | 0.99 | 16.5 |
| 2 | T1 | All MCs | 27 | 0.0 | 27 | 0.0 | * 0.599 | 34.7 | LOS C | 3.9 | 27.8 | 0.98 | 0.81 | 0.99 | 26.3 |
| 3 | R2 | All MCs | 306 | 2.1 | 306 | 2.1 | 0.599 | 38.2 | LOS D | 4.1 | 29.5 | 0.98 | 0.81 | 0.99 | 8.8 |
| Appro | oach | | 346 | 1.8 | 346 | 1.8 | 0.599 | 38.1 | LOS D | 4.1 | 29.5 | 0.98 | 0.81 | 0.99 | 11.4 |
| East: | Flagst | onian Dr | (E) | | | | | | | | | | | | |
| 4 | L2 | All MCs | 147 | 2.1 | 147 | 2.1 | 0.230 | 15.9 | LOS B | 1.4 | 9.9 | 0.74 | 0.75 | 0.74 | 28.2 |
| 5 | T1 | All MCs | 608 | 1.6 | 608 | 1.6 | * 0.866 | 39.9 | LOS D | 15.9 | 113.0 | 0.99 | 1.02 | 1.19 | 23.5 |
| 6 | R2 | All MCs | 134 | 0.0 | 134 | 0.0 | 0.480 | 47.4 | LOS D | 3.1 | 21.4 | 0.96 | 0.79 | 0.96 | 27.9 |
| Appro | oach | | 889 | 1.4 | 889 | 1.4 | 0.866 | 37.0 | LOS D | 15.9 | 113.0 | 0.94 | 0.94 | 1.08 | 22.8 |
| North | : Neigl | hbourhoo | d Conn | ecto | r (N) | | | | | | | | | | |
| 7 | L2 | All MCs | 82 | 0.0 | 82 | 0.0 | 0.111 | 20.7 | LOS C | 1.2 | 8.6 | 0.66 | 0.70 | 0.66 | 32.1 |
| 8 | T1 | All MCs | 5 | 0.0 | 5 | 0.0 | 0.019 | 30.9 | LOS C | 0.1 | 0.9 | 0.85 | 0.61 | 0.85 | 28.7 |
| 9 | R2 | All MCs | 1 | 0.0 | 1 | 0.0 | 0.019 | 34.1 | LOS C | 0.1 | 0.9 | 0.85 | 0.61 | 0.85 | 30.2 |
| Appro | oach | | 88 | 0.0 | 88 | 0.0 | 0.111 | 21.5 | LOS C | 1.2 | 8.6 | 0.67 | 0.69 | 0.67 | 31.8 |
| West | : Flags | tonian Dr | (W) | | | | | | | | | | | | |
| 10 | L2 | All MCs | 13 | 8.3 | 13 | 8.3 | 0.646 | 35.3 | LOS D | 6.7 | 48.6 | 0.95 | 0.81 | 0.95 | 31.0 |
| 11 | T1 | All MCs | 295 | 3.2 | 295 | 3.2 | 0.646 | 29.6 | LOS C | 6.7 | 48.6 | 0.95 | 0.81 | 0.95 | 15.7 |
| 12 | R2 | All MCs | 27 | 3.8 | 27 | 3.8 | * 0.404 | 51.3 | LOS D | 0.7 | 5.2 | 1.00 | 0.71 | 1.00 | 13.9 |
| Appro | oach | | 335 | 3.5 | 335 | 3.5 | 0.646 | 31.6 | LOS C | 6.7 | 48.6 | 0.95 | 0.80 | 0.96 | 16.5 |
| All Ve | hicles | | 1659 | 1.8 | 1659 | 1.8 | 0.866 | 35.3 | LOS D | 15.9 | 113.0 | 0.94 | 0.87 | 1.01 | 20.1 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

| Ped | destrian Mo | vement | Perforr | nance | | | | | | | |
|-----------|-----------------|--------------|----------------|---------------------|----------------|----------------|--------------|--------------|----------------|-----------------|----------------|
| Mo\ ID | / Crossing | Dem. Flow | Aver. Delay | Level of Service | AVERAGE QUE | BACK OF EUE | Prop. Que | Eff. Stop | Travel Time | Travel Dist. | Aver. Speed |
| | | ped/h | sec | | [Ped ped | Dist] m | | Rate | sec | m | m/sec |
| Sou | th: District Co | | | | | | | | | | |
| P1 | Full | 50 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | 188.1 | 200.0 | 1.06 |

| East: Flagstonian | Dr (E) | | | | | | | | | |
|-------------------|----------|--------|-------|-----|-----|------|------|-------|-------|------|
| P2 Full | 50 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | 188.1 | 200.0 | 1.06 |
| North: Neighbourh | nood Cor | nector | (N) | | | | | | | |
| P3 Full | 50 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | 188.1 | 200.0 | 1.06 |
| West: Flagstonian | Dr (W) | | | | | | | | | |
| P4 Full | 50 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | 188.1 | 200.0 | 1.06 |
| All Pedestrians | 200 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | 188.1 | 200.0 | 1.06 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Project: P:\P2300 Flagstone City Stages 2-5\Technical\Models\SIDRA\2024_02_16 CA3 Masterplan SIDRAs\P2300.001M 2066 CA3 South
Masterplan SIDRA Models.sip9

Site: 1 [1_2066 PM_Sensitivity (Site Folder: 2066 PM)]
Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: N101 [2066 District Centre PM_Sensitivity (Network Folder: General)]

New Beith Road / Flagstonian Drive

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Coordinated Cycle Time = 160 seconds (Site Practical Cycle Time)

| Vehi | cle M | ovement | Perfo | rma | nce | | | | | | | | | | |
|-----------|---------|--------------|---------------------------------|------------|------|---------------------------|---------------------|-----------------------|---------------------|-----------------------------|-------------------------|--------------|----------------------|---------------------------|------------------------|
| Mov ID | Turn | Mov Class | Dem Fl [Total l veh/h | ows HV] | FI | rival ows HV] % | Deg. Satn v/c | Aver. Delay sec | Level of Service | Aver. Back [Veh. veh | Of Queue Dist] m | Prop. Que | Eff. Stop Rate | Aver. No. of Cycles | Aver. Speed km/h |
| South | n: New | Beith Ro | ad (S) | | | | | | | | | | | | |
| 1 | L2 | All MCs | 175 | 1.1 | 175 | 1.1 | 0.271 | 43.9 | LOS D | 3.4 | 23.9 | 0.76 | 0.76 | 0.76 | 37.7 |
| 2 | T1 | All MCs | 778 | 1.7 | 778 | 1.7 | 0.817 | 71.1 | LOS E | 19.4 | 137.5 | 1.00 | 0.92 | 1.06 | 39.6 |
| 3 | R2 | All MCs | 191 | 2.1 | 191 | 2.1 | 0.696 | 88.7 | LOS F | 4.7 | 33.5 | 1.00 | 0.83 | 1.08 | 24.9 |
| Appro | oach | | 1144 | 1.7 | 1144 | 1.7 | 0.817 | 69.9 | LOS E | 19.4 | 137.5 | 0.96 | 0.88 | 1.02 | 34.7 |
| East: | Flagst | tonian Dr | (E) | | | | | | | | | | | | |
| 4 | L2 | All MCs | 805 | 0.7 | 805 | 0.7 | * 0.995 | 128.2 | LOS F | 25.6 | 180.6 | 1.00 | 1.08 | 1.40 | 20.8 |
| 5 | T1 | All MCs | 360 | 1.7 | 360 | 1.7 | 0.635 | 54.2 | LOS D | 14.8 | 105.3 | 0.93 | 0.81 | 0.93 | 14.1 |
| 6 | R2 | All MCs | 368 | 2.7 | 368 | 2.7 | 0.717 | 62.5 | LOS E | 15.7 | 112.1 | 0.96 | 0.86 | 0.96 | 33.2 |
| Appro | oach | | 1533 | 1.4 | 1533 | 1.4 | 0.995 | 95.0 | LOS F | 25.6 | 180.6 | 0.97 | 0.96 | 1.19 | 21.5 |
| North | : New | Bieth Roa | ad (N) | | | | | | | | | | | | |
| 7 | L2 | All MCs | 171 | 1.2 | 171 | 1.2 | 0.143 | 32.1 | LOS C | 1.7 | 11.8 | 0.41 | 0.70 | 0.41 | 54.8 |
| 8 | T1 | All MCs | 1324 | 1.0 | 1324 | 1.0 | * 0.972 | 87.9 | LOS F | 36.0 | 253.9 | 1.00 | 1.08 | 1.21 | 38.7 |
| 9 | R2 | All MCs | 321 | 1.2 | 321 | 1.2 | 0.439 | 77.3 | LOS E | 7.0 | 49.2 | 0.92 | 0.80 | 0.92 | 26.7 |
| Appro | oach | | 1816 | 1.0 | 1816 | 1.0 | 0.972 | 80.8 | LOS F | 36.0 | 253.9 | 0.93 | 0.99 | 1.09 | 32.9 |
| West | : Flags | tonian Dr | (W) | | | | | | | | | | | | |
| 10 | L2 | All MCs | 167 | 2.4 | 167 | 2.4 | 0.276 | 47.3 | LOS D | 5.8 | 41.4 | 0.78 | 0.78 | 0.78 | 33.4 |
| 11 | T1 | All MCs | 280 | 3.2 | 280 | 3.2 | * 0.961 | 104.5 | LOS F | 9.3 | 66.6 | 1.00 | 1.08 | 1.44 | 12.2 |
| 12 | R2 | All MCs | 195 | 1.0 | 195 | 1.0 | 0.961 | 110.5 | LOS F | 9.2 | 65.7 | 1.00 | 1.07 | 1.44 | 19.9 |
| Appro | oach | | 642 | 2.3 | 642 | 2.3 | 0.961 | 91.5 | LOS F | 9.3 | 66.6 | 0.94 | 1.00 | 1.27 | 19.6 |
| All Ve | hicles | | 5135 | 1.5 | 5135 | 1.5 | 0.995 | 83.9 | LOS F | 36.0 | 253.9 | 0.95 | 0.96 | 1.12 | 28.4 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Override Site Data tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

| Ped | lestrian Mo | vement | Perforr | nance | | | | | | | |
|-----------|---------------|--------------|----------------|---------------------|----------------|----------------|--------------|--------------|----------------|-----------------|----------------|
| Mov ID | , Crossing | Dem. Flow | Aver. Delay | Level of Service | AVERAGE QUI | BACK OF EUE | Prop. Que | Eff. Stop | Travel Time | Travel Dist. | Aver. Speed |
| | | ped/h | sec | | [Ped ped | Dist] m | | Rate | sec | m | m/sec |
| Sou | th: New Beith | | 5) | | | | | | | | |
| P1 | Full | 20 | 73.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 | 88.6 | 20.0 | 0.23 |

| East: Flagstonian | Dr (E) | | | | | | | | | |
|-------------------|--------|------|-------|-----|-----|------|------|-------|-------|------|
| P2 Full | 50 | 73.3 | LOS F | 0.2 | 0.2 | 0.96 | 0.96 | 88.7 | 20.0 | 0.23 |
| West: Flagstonian | Dr (W) | | | | | | | | | |
| P4 Full | 50 | 74.3 | LOS F | 0.2 | 0.2 | 0.96 | 0.96 | 228.1 | 200.0 | 0.88 |
| All Pedestrians | 120 | 73.7 | LOS F | 0.2 | 0.2 | 0.96 | 0.96 | 146.8 | 95.0 | 0.65 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Project: P:\P2300 Flagstone City Stages 2-5\Technical\Models\SIDRA\2024_02_16 CA3 Masterplan SIDRAs\P2300.001M 2066 CA3 South
Masterplan SIDRA Models.sip9

Site: 2 [2_2066 PM_Sensitivity (Site Folder: 2066 PM)]
Output produced by SIDRA INTERSECTION Version: 9.1.6.228

Network: N101 [2066 District Centre PM_Sensitivity (Network Folder: General)]

New Site

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 80 seconds (Site Practical Cycle Time)

| Vehi | cle M | ovement | Perfo | rma | nce | | | | | | | | | | |
|-----------|----------|--------------|--------|------------|---------|--------------------------|---------------------|-----------------------|---------------------|-----------------------------|-------------------------|--------------|----------------------|---------------------------|------------------------|
| Mov ID | Turn | Mov Class | | ows HV] | | rival ows HV] % | Deg. Satn v/c | Aver. Delay sec | Level of Service | Aver. Back [Veh. veh | Of Queue Dist] m | Prop. Que | Eff. Stop Rate | Aver. No. of Cycles | Aver. Speed km/h |
| South | n: Distr | rict Centre | | 70 | VG11/11 | 70 | V/ O | | | ٧٥١١ | | | | | IXIII/II |
| 1 | L2 | All MCs | 13 | 0.0 | 13 | 0.0 | 0.599 | 44.6 | LOS D | 3.9 | 27.8 | 0.98 | 0.81 | 0.99 | 16.5 |
| 2 | T1 | All MCs | 27 | 0.0 | 27 | 0.0 | * 0.599 | 34.7 | LOS C | 3.9 | 27.8 | 0.98 | 0.81 | 0.99 | 26.3 |
| 3 | R2 | All MCs | 306 | 2.1 | 306 | 2.1 | 0.599 | 38.2 | LOS D | 4.1 | 29.5 | 0.98 | 0.81 | 0.99 | 8.8 |
| Appro | oach | | 346 | 1.8 | 346 | 1.8 | 0.599 | 38.1 | LOS D | 4.1 | 29.5 | 0.98 | 0.81 | 0.99 | 11.4 |
| East: | Flagst | tonian Dr | (E) | | | | | | | | | | | | |
| 4 | L2 | All MCs | 147 | 2.1 | 147 | 2.1 | 0.230 | 15.9 | LOS B | 1.4 | 9.9 | 0.74 | 0.75 | 0.74 | 28.2 |
| 5 | T1 | All MCs | 608 | 1.6 | 608 | 1.6 | * 0.866 | 39.9 | LOS D | 15.9 | 113.0 | 0.99 | 1.02 | 1.19 | 23.5 |
| 6 | R2 | All MCs | 134 | 0.0 | 134 | 0.0 | 0.480 | 47.4 | LOS D | 3.1 | 21.4 | 0.96 | 0.79 | 0.96 | 27.9 |
| Appro | oach | | 889 | 1.4 | 889 | 1.4 | 0.866 | 37.0 | LOS D | 15.9 | 113.0 | 0.94 | 0.94 | 1.08 | 22.8 |
| North | : Neig | hbourhoo | d Conn | ecto | r (N) | | | | | | | | | | |
| 7 | L2 | All MCs | 82 | 0.0 | 82 | 0.0 | 0.111 | 20.7 | LOS C | 1.2 | 8.6 | 0.66 | 0.70 | 0.66 | 32.1 |
| 8 | T1 | All MCs | 5 | 0.0 | 5 | 0.0 | 0.019 | 30.9 | LOS C | 0.1 | 0.9 | 0.85 | 0.61 | 0.85 | 28.7 |
| 9 | R2 | All MCs | 1 | 0.0 | 1 | 0.0 | 0.019 | 34.1 | LOS C | 0.1 | 0.9 | 0.85 | 0.61 | 0.85 | 30.2 |
| Appro | oach | | 88 | 0.0 | 88 | 0.0 | 0.111 | 21.5 | LOS C | 1.2 | 8.6 | 0.67 | 0.69 | 0.67 | 31.8 |
| West | Flags | tonian Dr | (W) | | | | | | | | | | | | |
| 10 | L2 | All MCs | 13 | 8.3 | 13 | 8.3 | 0.646 | 35.3 | LOS D | 6.7 | 48.6 | 0.95 | 0.81 | 0.95 | 31.0 |
| 11 | T1 | All MCs | 295 | 3.2 | 295 | 3.2 | 0.646 | 29.6 | LOS C | 6.7 | 48.6 | 0.95 | 0.81 | 0.95 | 15.7 |
| 12 | R2 | All MCs | 27 | 3.8 | 27 | 3.8 | * 0.404 | 51.3 | LOS D | 0.7 | 5.2 | 1.00 | 0.71 | 1.00 | 13.9 |
| Appro | oach | | 335 | 3.5 | 335 | 3.5 | 0.646 | 31.6 | LOS C | 6.7 | 48.6 | 0.95 | 0.80 | 0.96 | 16.5 |
| All Ve | hicles | | 1659 | 1.8 | 1659 | 1.8 | 0.866 | 35.3 | LOS D | 15.9 | 113.0 | 0.94 | 0.87 | 1.01 | 20.1 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Network Data dialog (Override Site Data tab)

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

| Pedestrian Mo | ovement | Perform | nance | | | | | | | |
|--------------------|--------------|----------------|---------------------|----------------|-------------|--------------|--------------|----------------|-----------------|----------------|
| Mov ID Crossing | Dem. Flow | Aver. Delay | Level of Service | AVERAGE QUE | UE | Prop. Que | Eff. Stop | Travel Time | Travel Dist. | Aver. Speed |
| | ped/h | sec | | [Ped ped | Dist] m | | Rate | sec | m | m/sec |
| South: District C | Centre (S) | | | | | | | | | |
| P1 Full | 50 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | 188.1 | 200.0 | 1.06 |

| East: Flagstonian | Dr (E) | | | | | | | | | |
|-------------------|----------|--------|-------|-----|-----|------|------|-------|-------|------|
| P2 Full | 50 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | 188.1 | 200.0 | 1.06 |
| North: Neighbourh | nood Cor | nector | (N) | | | | | | | |
| P3 Full | 50 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | 188.1 | 200.0 | 1.06 |
| West: Flagstonian | Dr (W) | | | | | | | | | |
| P4 Full | 50 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | 188.1 | 200.0 | 1.06 |
| All Pedestrians | 200 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | 188.1 | 200.0 | 1.06 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Masterplan SIDRA Models.sip9

SITE LAYOUT

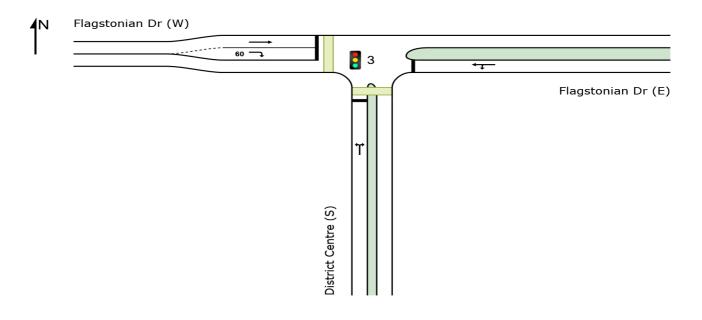
Site: 3 [3_2066 AM (Site Folder: 2066 AM)]

New Site

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



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Masterplan SIDRA Models.sip9

Site: 3 [3_2066 AM (Site Folder: 2066 AM)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

New Site

Site Category: (None)

| Vehic | cle M | ovemen | t Perfo | rma | nce | | | | | | | | | | |
|--------|---------|------------|------------------|------|---------------|-----------|---|-------|----------|---------------|-------------|-------|------|--------|----------|
| Mov | Turn | Mov | Dem | | | rival | Deg. | Aver. | Level of | 95% B | | Prop. | Eff. | Aver. | Aver. |
| ID | | Class | | lows | | lows | Satn | Delay | Service | Que | | Que | Stop | No. of | Speed |
| | | | ιοται ι veh/h | | [Total veh/h | HV] % | v/c | sec | | [Veh. veh | Dist] m | | Rate | Cycles | km/h |
| South | : Distr | ict Centre | | 70 | 7011/11 | | • | | | 7011 | | | | | 1(11)/11 |
| 1 | L2 | All MCs | 7 | 0.0 | 7 | 0.0 | 0.233 | 18.0 | LOS B | 1.1 | 8.2 | 0.87 | 0.73 | 0.87 | 34.2 |
| 3 | R2 | All MCs | 66 | 3.2 | 66 | 3.2 | 0.233 | 17.1 | LOS B | 1.1 | 8.2 | 0.87 | 0.73 | 0.87 | 24.7 |
| Appro | ach | | 74 | 2.9 | 74 | 2.9 | 0.233 | 17.2 | LOS B | 1.1 | 8.2 | 0.87 | 0.73 | 0.87 | 25.9 |
| East: | Flagst | tonian Dr | (E) | | | | | | | | | | | | |
| 4 | L2 | All MCs | 72 | 1.5 | 72 | 1.5 | * 0.636 | 20.9 | LOS C | 5.1 | 36.6 | 0.92 | 0.82 | 1.01 | 28.1 |
| 5 | T1 | All MCs | 240 | 2.2 | 240 | 2.2 | 0.636 | 12.5 | LOS B | 5.1 | 36.6 | 0.92 | 0.82 | 1.01 | 42.2 |
| Appro | ach | | 312 | 2.0 | 312 | 2.0 | 0.636 | 14.5 | LOS B | 5.1 | 36.6 | 0.92 | 0.82 | 1.01 | 39.8 |
| West: | Flags | tonian Dr | (W) | | | | | | | | | | | | |
| 11 | T1 | All MCs | 545 | 1.9 | 545 | 1.9 | 0.522 | 5.7 | LOSA | 6.1 | 43.6 | 0.68 | 0.60 | 0.68 | 51.5 |
| 12 | R2 | All MCs | 20 | 0.0 | 20 | 0.0 | * 0.377 | 28.1 | LOS C | 0.4 | 3.0 | 1.00 | 0.69 | 1.03 | 29.2 |
| Appro | ach | | 565 | 1.9 | 565 | 1.9 | 0.522 | 6.5 | LOSA | 6.1 | 43.6 | 0.69 | 0.60 | 0.69 | 50.3 |
| All Ve | hicles | | 951 | 2.0 | 951 | 2.0 | 0.636 | 9.9 | LOSA | 6.1 | 43.6 | 0.78 | 0.68 | 0.81 | 44.7 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

| Mov | Input | Dem. | Aver. | Level of / | AVERAGE | BACK OF | Prop. | Eff. | Travel | Travel | Aver. |
|--------------------|-----------|-------|-------|------------|--------------|---------------|-------|--------------|--------|---------|-------|
| ID Crossing | Vol. | Flow | Delay | Service | QUE [Ped | :UE Dist] | Que | Stop Rate | Time | Dist. S | Speed |
| | ped/h | ped/h | sec | | ped | m ¯ | | | sec | m | m/sec |
| South: District | Centre | (S) | | | | | | | | | |
| P1 Full | 50 | 50 | 12.0 | LOS B | 0.0 | 0.0 | 0.83 | 0.83 | 165.9 | 200.0 | 1.21 |
| West: Flagsto | nian Dr (| W) | | | | | | | | | |
| P4 Full | 50 | 50 | 12.0 | LOS B | 0.0 | 0.0 | 0.83 | 0.83 | 165.9 | 200.0 | 1.21 |
| All Pedestrians | 100 | 100 | 12.0 | LOS B | 0.0 | 0.0 | 0.83 | 0.83 | 165.9 | 200.0 | 1.21 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 3 [3_2066 PM (Site Folder: 2066 PM)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

New Site

Site Category: (None)

| Vehic | cle M | ovemen | t Perfo | rma | nce | | | | | | | | | | |
|--------|---------|------------|------------------|------|---------------|-----------|----------------|-------|----------|---------------|-------------|-------|------|--------|--------|
| Mov | Turn | Mov | Dem | | | rival | Deg. | Aver. | Level of | 95% B | | Prop. | Eff. | Aver. | Aver. |
| ID | | Class | | lows | | lows | Satn | Delay | Service | Que | | Que | Stop | No. of | Speed |
| | | | ιοται ι veh/h | | [Total veh/h | HV] % | v/c | sec | | [Veh. veh | Dist] m | | Rate | Cycles | km/h |
| South | : Distr | ict Centre | | 70 | 7011/11 | | •,,, | | | 7011 | | | | | 111711 |
| 1 | L2 | All MCs | 37 | 0.0 | 37 | 0.0 | 0.475 | 21.7 | LOS C | 2.4 | 17.7 | 0.94 | 0.77 | 0.94 | 32.0 |
| 3 | R2 | All MCs | 89 | 5.9 | 89 | 5.9 | * 0.475 | 20.7 | LOS C | 2.4 | 17.7 | 0.94 | 0.77 | 0.94 | 22.4 |
| Appro | ach | | 126 | 4.2 | 126 | 4.2 | 0.475 | 21.0 | LOS C | 2.4 | 17.7 | 0.94 | 0.77 | 0.94 | 25.8 |
| East: | Flagst | tonian Dr | (E) | | | | | | | | | | | | |
| 4 | L2 | All MCs | 43 | 0.0 | 43 | 0.0 | 0.732 | 16.7 | LOS B | 11.0 | 77.9 | 0.86 | 0.83 | 0.96 | 30.7 |
| 5 | T1 | All MCs | 576 | 1.6 | 576 | 1.6 | * 0.732 | 11.1 | LOS B | 11.0 | 77.9 | 0.86 | 0.83 | 0.96 | 44.9 |
| Appro | ach | | 619 | 1.5 | 619 | 1.5 | 0.732 | 11.5 | LOS B | 11.0 | 77.9 | 0.86 | 0.83 | 0.96 | 44.2 |
| West: | Flags | tonian Dr | (W) | | | | | | | | | | | | |
| 11 | T1 | All MCs | 243 | 2.6 | 243 | 2.6 | 0.231 | 5.1 | LOSA | 2.5 | 17.8 | 0.55 | 0.46 | 0.55 | 52.3 |
| 12 | R2 | All MCs | 5 | 20.0 | 5 | 20.0 | * 0.130 | 31.2 | LOS C | 0.1 | 1.0 | 1.00 | 0.63 | 1.00 | 27.9 |
| Appro | ach | | 248 | 3.0 | 248 | 3.0 | 0.231 | 5.6 | LOSA | 2.5 | 17.8 | 0.56 | 0.46 | 0.56 | 51.4 |
| All Ve | hicles | | 994 | 2.2 | 994 | 2.2 | 0.732 | 11.2 | LOS B | 11.0 | 77.9 | 0.80 | 0.73 | 0.86 | 43.3 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

| Pedestrian I | Noveme | ent Perf | ormano | e | | | | | | | |
|--------------------|---------------|--------------|----------------|---------------------|-------------------------|-----|--------------|----------------------|----------------|-------------------|----------------|
| Mov ID Crossing | Input Vol. | Dem. Flow | Aver. Delay | Level of Service | AVERAGE QUE [Ped | | Prop. Que | Eff. Stop Rate | Travel Time | Travel Dist. S | Aver. Speed |
| | ped/h | ped/h | sec | | ped | m Î | | | sec | m | m/sec |
| South: District | Centre | (S) | | | | | | | | | |
| P1 Full | 50 | 50 | 14.5 | LOS B | 0.0 | 0.0 | 0.85 | 0.85 | 168.3 | 200.0 | 1.19 |
| West: Flagsto | nian Dr (| W) | | | | | | | | | |
| P4 Full | 50 | 50 | 14.5 | LOS B | 0.0 | 0.0 | 0.85 | 0.85 | 168.3 | 200.0 | 1.19 |
| All Pedestrians | 100 | 100 | 14.5 | LOS B | 0.0 | 0.0 | 0.85 | 0.85 | 168.3 | 200.0 | 1.19 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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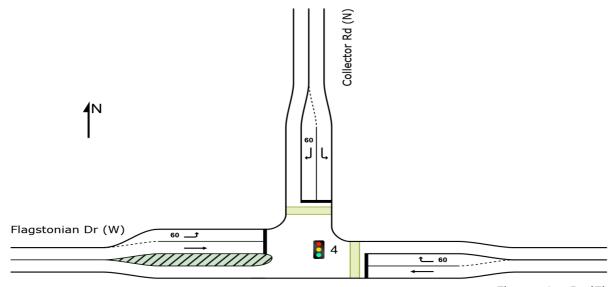
SITE LAYOUT

Site: 4 [4_2066 AM (Site Folder: 2066 AM)]

New Site

Site Category: (None)
Signals - EQUISAT (Fixed-Time/SCATS) Isolated

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



Flagstonian Dr (E)

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Masterplan SIDRA Models.sip9

Site: 4 [4_2066 AM (Site Folder: 2066 AM)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

New Site

Site Category: (None)

| Vehi | cle Mo | ovement | Perfo | rma | nce | | | | | | | | | | |
|--------|---------|------------|--------------------|------|---------------|-----------|----------------|-------|----------|---------------|-------------|-------|------|--------|---------|
| Mov | Turn | Mov | Dem | nand | Ar | rival | Deg. | Aver. | Level of | 95% Ba | ack Of | Prop. | Eff. | Aver. | Aver. |
| ID | | Class | | lows | | lows | Satn | Delay | Service | Que | | Que | Stop | No. of | Speed |
| | | | [lotal veh/h | | [Total veh/h | HV J % | v/c | sec | | [Veh. veh | Dist] m | | Rate | Cycles | km/h |
| East: | Flagst | tonian Dr | | 70 | VG11/11 | 70 | V/C | 366 | | Ven | - ''' | | | | KIII/II |
| 5 | T1 | All MCs | 154 | 3.4 | 154 | 3.4 | 0.142 | 3.4 | LOSA | 1.1 | 7.8 | 0.50 | 0.40 | 0.50 | 56.5 |
| 6 | R2 | All MCs | 28 | 0.0 | 28 | 0.0 | * 0.230 | 22.2 | LOS C | 0.5 | 3.2 | 0.98 | 0.70 | 0.98 | 38.7 |
| Appro | oach | | 182 | 2.9 | 182 | 2.9 | 0.230 | 6.3 | LOSA | 1.1 | 7.8 | 0.57 | 0.45 | 0.57 | 52.7 |
| North | : Colle | ctor Rd (I | N) | | | | | | | | | | | | |
| 7 | L2 | All MCs | 100 | 0.0 | 100 | 0.0 | 0.538 | 20.7 | LOS C | 1.6 | 11.3 | 0.99 | 0.80 | 1.10 | 39.0 |
| 9 | R2 | All MCs | 17 | 0.0 | 17 | 0.0 | * 0.054 | 16.7 | LOS B | 0.2 | 1.6 | 0.87 | 0.67 | 0.87 | 42.3 |
| Appro | oach | | 117 | 0.0 | 117 | 0.0 | 0.538 | 20.1 | LOS C | 1.6 | 11.3 | 0.97 | 0.78 | 1.07 | 39.5 |
| West | : Flags | tonian Dr | (W) | | | | | | | | | | | | |
| 10 | L2 | All MCs | 26 | 8.0 | 26 | 8.0 | 0.024 | 7.9 | LOSA | 0.1 | 1.1 | 0.39 | 0.64 | 0.39 | 47.6 |
| 11 | T1 | All MCs | 458 | 2.3 | 458 | 2.3 | * 0.511 | 6.3 | LOS A | 4.9 | 35.1 | 0.75 | 0.64 | 0.75 | 53.8 |
| Appro | oach | | 484 | 2.6 | 484 | 2.6 | 0.511 | 6.4 | LOSA | 4.9 | 35.1 | 0.73 | 0.64 | 0.73 | 53.4 |
| All Ve | hicles | | 783 | 2.3 | 783 | 2.3 | 0.538 | 8.4 | LOSA | 4.9 | 35.1 | 0.73 | 0.62 | 0.74 | 50.6 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

| Mov | Input | Dem. | Aver. | Level of A | AVERAGE | BACK OF | Prop. | Eff. | Travel | Travel | Aver. |
|--------------------|-----------|-------|-------|------------|--------------|--------------|-------|--------------|--------|---------|-------|
| ID Crossing | Vol. | Flow | Delay | Service | QUE [Ped | UE Dist] | Que | Stop Rate | Time | Dist. S | Speed |
| | ped/h | ped/h | sec | | ped | m ¯ | | | sec | m | m/sec |
| East: Flagston | ian Dr (E | Ξ) | | | | | | | | | |
| P2 Full | 50 | 50 | 9.6 | LOS A | 0.0 | 0.0 | 0.80 | 0.80 | 163.5 | 200.0 | 1.22 |
| North: Collecte | or Rd (N |) | | | | | | | | | |
| P3 Full | 50 | 50 | 9.6 | LOS A | 0.0 | 0.0 | 0.80 | 0.80 | 163.5 | 200.0 | 1.22 |
| All Pedestrians | 100 | 100 | 9.6 | LOSA | 0.0 | 0.0 | 0.80 | 0.80 | 163.5 | 200.0 | 1.22 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 4 [4_2066 PM (Site Folder: 2066 PM)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

New Site

Site Category: (None)

| Vehic | cle M | ovemen | t Perfo | rma | nce | | | | | | | | | | |
|-----------|---------|--------------|---------|-------------|-----|---------------------------|---------------------|-----------------------|---------------------|-------------------------------|------------------------------|--------------|----------------------|---------------------------|------------------------|
| Mov ID | Turn | Mov Class | | lows HV] | | rival lows HV] % | Deg. Satn v/c | Aver. Delay sec | Level of Service | 95% B Que [Veh. veh | ack Of eue Dist] m | Prop. Que | Eff. Stop Rate | Aver. No. of Cycles | Aver. Speed km/h |
| East: | Flags | tonian Dr | (E) | | | | | | | | | | | | |
| 5 | T1 | All MCs | 516 | 1.0 | 516 | 1.0 | 0.444 | 3.1 | LOSA | 3.6 | 25.3 | 0.58 | 0.50 | 0.58 | 56.8 |
| 6 | R2 | All MCs | 75 | 0.0 | 75 | 0.0 | * 0.252 | 16.5 | LOS B | 0.9 | 6.2 | 0.91 | 0.74 | 0.91 | 41.4 |
| Appro | ach | | 591 | 0.9 | 591 | 0.9 | 0.444 | 4.8 | LOSA | 3.6 | 25.3 | 0.62 | 0.53 | 0.62 | 54.3 |
| North | : Colle | ctor Rd (| N) | | | | | | | | | | | | |
| 7 | L2 | All MCs | 43 | 2.4 | 43 | 2.4 | 0.148 | 15.4 | LOS B | 0.5 | 3.6 | 0.89 | 0.71 | 0.89 | 41.6 |
| 9 | R2 | All MCs | 27 | 0.0 | 27 | 0.0 | * 0.092 | 15.1 | LOS B | 0.3 | 2.2 | 0.88 | 0.69 | 0.88 | 43.1 |
| Appro | ach | | 71 | 1.5 | 71 | 1.5 | 0.148 | 15.3 | LOS B | 0.5 | 3.6 | 0.89 | 0.70 | 0.89 | 42.2 |
| West | Flags | tonian Dı | r (W) | | | | | | | | | | | | |
| 10 | L2 | All MCs | 14 | 0.0 | 14 | 0.0 | 0.017 | 9.9 | LOSA | 0.1 | 0.7 | 0.58 | 0.64 | 0.58 | 46.5 |
| 11 | T1 | All MCs | 160 | 3.9 | 160 | 3.9 | * 0.234 | 6.4 | LOS A | 1.4 | 10.5 | 0.73 | 0.59 | 0.73 | 53.7 |
| Appro | ach | | 174 | 3.6 | 174 | 3.6 | 0.234 | 6.7 | LOSA | 1.4 | 10.5 | 0.72 | 0.59 | 0.72 | 53.0 |
| All Ve | hicles | | 835 | 1.5 | 835 | 1.5 | 0.444 | 6.1 | LOSA | 3.6 | 25.3 | 0.67 | 0.56 | 0.67 | 52.7 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

| Pedestrian N | loveme | ent Perf | ormano | e | | | | | | | |
|--------------------|---------------|--------------|----------------|---------------------|-------------------------|-----|--------------|----------------------|----------------|-------------------|----------------|
| Mov ID Crossing | Input Vol. | Dem. Flow | Aver. Delay | Level of Service | AVERAGE QUE [Ped | | Prop. Que | Eff. Stop Rate | Travel Time | Travel Dist. S | Aver. Speed |
| | ped/h | ped/h | sec | | ped | m Î | | | sec | m | m/sec |
| East: Flagston | ian Dr (E | Ξ) | | | | | | | | | |
| P2 Full | 50 | 50 | 7.2 | LOSA | 0.0 | 0.0 | 0.76 | 0.76 | 161.1 | 200.0 | 1.24 |
| North: Collecto | or Rd (N |) | | | | | | | | | |
| P3 Full | 50 | 50 | 7.2 | LOSA | 0.0 | 0.0 | 0.76 | 0.76 | 161.1 | 200.0 | 1.24 |
| All Pedestrians | 100 | 100 | 7.2 | LOSA | 0.0 | 0.0 | 0.76 | 0.76 | 161.1 | 200.0 | 1.24 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Project: P:\P2300 Flagstone City Stages 2-5\Technical\Models\SIDRA\2024_02_16 CA3 Masterplan SIDRAs\P2300.001M 2066 CA3 South

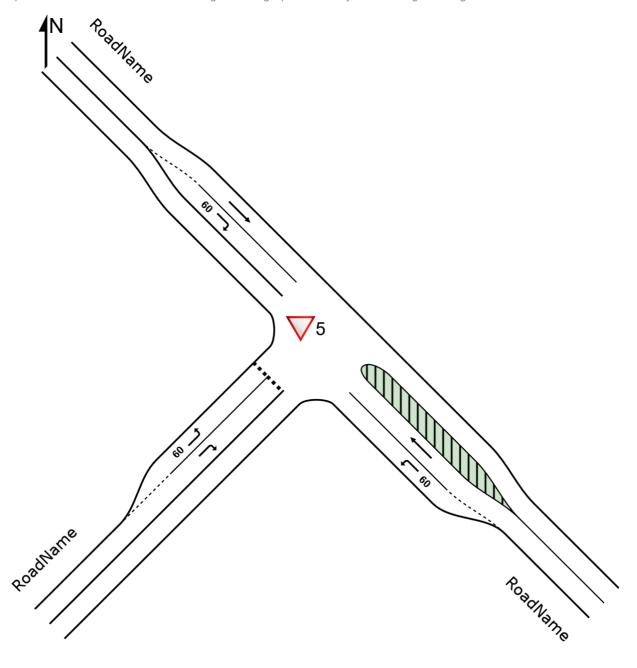
Masterplan SIDRA Models.sip9

SITE LAYOUT

▽ Site: 5 [5_2066 AM (Site Folder: 2066 AM)]

CA3 Collector / Ulladulla N Connection Site Category: (None) Give-Way (Two-Way)

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



▽ Site: 5 [5_2066 AM (Site Folder: 2066 AM)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

CA3 Collector / Ulladulla N Connection

Site Category: (None) Give-Way (Two-Way)

| Vehic | cle M | ovemen | t Perfo | rma | nce | | | | | | | | | | |
|-----------|--------|--------------|---------|-------------|-----|---------------------------|---------------------|-----------------------|---------------------|-----|--------------------------------|--------------|----------------------|---------------------------|------------------------|
| Mov ID | Turn | Mov Class | | lows HV] | | rival lows HV] % | Deg. Satn v/c | Aver. Delay sec | Level of Service | | Back Of leue Dist] m | Prop. Que | Eff. Stop Rate | Aver. No. of Cycles | Aver. Speed km/h |
| South | East: | RoadNar | me | | | | | | | | | | | | |
| 4 | L2 | All MCs | 113 | 2.7 | 113 | 2.7 | 0.062 | 5.6 | LOSA | 0.0 | 0.0 | 0.00 | 0.58 | 0.00 | 52.8 |
| 5 | T1 | All MCs | 50 | 4.0 | 50 | 4.0 | 0.026 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 60.0 |
| Appro | ach | | 163 | 3.1 | 163 | 3.1 | 0.062 | 3.9 | NA | 0.0 | 0.0 | 0.00 | 0.40 | 0.00 | 54.8 |
| North | West: | RoadNa | me | | | | | | | | | | | | |
| 11 | T1 | All MCs | 32 | 3.1 | 32 | 3.1 | 0.017 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 60.0 |
| 12 | R2 | All MCs | 50 | 0.0 | 50 | 0.0 | 0.041 | 6.1 | LOSA | 0.2 | 1.1 | 0.27 | 0.56 | 0.27 | 52.0 |
| Appro | ach | | 82 | 1.2 | 82 | 1.2 | 0.041 | 3.7 | NA | 0.2 | 1.1 | 0.16 | 0.34 | 0.16 | 54.9 |
| South | West: | RoadNa | me | | | | | | | | | | | | |
| 1 | L2 | All MCs | 230 | 1.7 | 230 | 1.7 | 0.182 | 5.8 | LOSA | 0.7 | 5.3 | 0.15 | 0.55 | 0.15 | 52.4 |
| 3 | R2 | All MCs | 406 | 2.2 | 406 | 2.2 | 0.475 | 8.3 | LOSA | 3.4 | 24.4 | 0.49 | 0.66 | 0.56 | 50.9 |
| Appro | ach | | 636 | 2.0 | 636 | 2.0 | 0.475 | 7.4 | LOSA | 3.4 | 24.4 | 0.36 | 0.62 | 0.41 | 51.4 |
| All Ve | hicles | | 881 | 2.2 | 881 | 2.2 | 0.475 | 6.4 | NA | 3.4 | 24.4 | 0.28 | 0.55 | 0.31 | 52.3 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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Project: P:\P2300 Flagstone City Stages 2-5\Technical\Models\SIDRA\2024_02_16 CA3 Masterplan SIDRAs\P2300.001M 2066 CA3 South Masterplan SIDRA Models.sip9

▽ Site: 5 [5_2066 PM (Site Folder: 2066 PM)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

CA3 Collector / Ulladulla N Connection

Site Category: (None) Give-Way (Two-Way)

| Vehic | cle M | ovemen | t Perfo | rma | nce | | | | | | | | | | |
|-----------|--------|--------------|---------|-------------|-----|--------------------------|---------------------|-----------------------|---------------------|-----|--------------------------------|--------------|----------------------|---------------------------|------------------------|
| Mov ID | Turn | Mov Class | | lows HV] | | rival ows HV] % | Deg. Satn v/c | Aver. Delay sec | Level of Service | | Back Of Jeue Dist] m | Prop. Que | Eff. Stop Rate | Aver. No. of Cycles | Aver. Speed km/h |
| South | East: | RoadNar | ne | | | | | | | | | | | | |
| 4 | L2 | All MCs | 390 | 1.0 | 390 | 1.0 | 0.212 | 5.6 | LOSA | 0.0 | 0.0 | 0.00 | 0.58 | 0.00 | 52.8 |
| 5 | T1 | All MCs | 95 | 1.1 | 95 | 1.1 | 0.049 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 60.0 |
| Appro | ach | | 485 | 1.0 | 485 | 1.0 | 0.212 | 4.5 | NA | 0.0 | 0.0 | 0.00 | 0.46 | 0.00 | 54.0 |
| North | West: | RoadNar | ne | | | | | | | | | | | | |
| 11 | T1 | All MCs | 46 | 13.0 | 46 | 13.0 | 0.025 | 0.0 | LOSA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 60.0 |
| 12 | R2 | All MCs | 84 | 0.0 | 84 | 0.0 | 0.098 | 8.0 | LOSA | 0.4 | 2.6 | 0.50 | 0.71 | 0.50 | 51.0 |
| Appro | ach | | 130 | 4.6 | 130 | 4.6 | 0.098 | 5.1 | NA | 0.4 | 2.6 | 0.32 | 0.46 | 0.32 | 53.9 |
| South | West: | RoadNa | me | | | | | | | | | | | | |
| 1 | L2 | All MCs | 69 | 1.4 | 69 | 1.4 | 0.057 | 5.9 | LOSA | 0.2 | 1.5 | 0.19 | 0.55 | 0.19 | 52.3 |
| 3 | R2 | All MCs | 106 | 0.0 | 106 | 0.0 | 0.167 | 9.2 | LOSA | 0.7 | 4.6 | 0.53 | 0.73 | 0.53 | 50.3 |
| Appro | ach | | 175 | 0.6 | 175 | 0.6 | 0.167 | 7.9 | LOSA | 0.7 | 4.6 | 0.39 | 0.66 | 0.39 | 51.1 |
| All Ve | hicles | | 790 | 1.5 | 790 | 1.5 | 0.212 | 5.4 | NA | 0.7 | 4.6 | 0.14 | 0.51 | 0.14 | 53.3 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA (TWSC): Level of Service is not defined for major road approaches or the intersection as a whole for Two-Way Sign Control (HCM LOS rule).

Two-Way Sign Control Capacity Model: SIDRA Standard.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Gap.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

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SITE LAYOUT

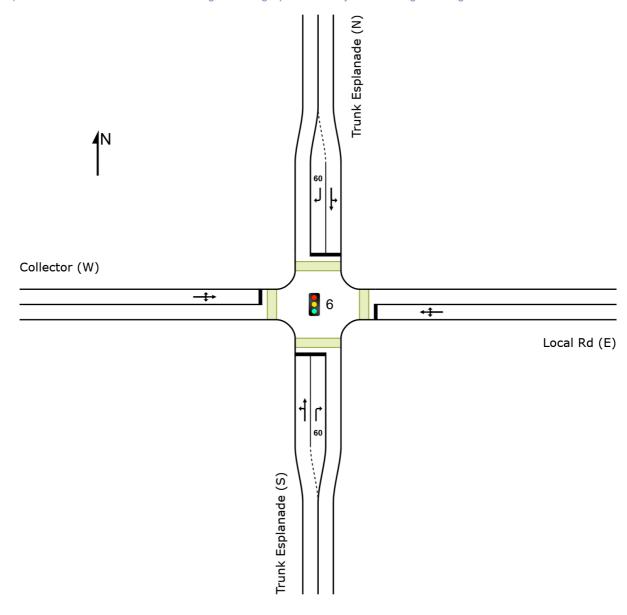
Site: 6 [6_2066 AM (Site Folder: 2066 AM)]

NA

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



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Masterplan SIDRA Models.sip9

Site: 6 [6_2066 AM (Site Folder: 2066 AM)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA

Site Category: (None)

| Vehi | cle Mo | ovemen | t Perfo | rma | nce | | | | | | | | | | |
|--------|---------|-----------|---------|-----|---------------|-------------|---------|-------|----------|--------------|---------------|-------|--------------|------------------|-------|
| Mov | Turn | Mov | Dem | | | rival | Deg. | Aver. | Level of | | ack Of | Prop. | Eff. | Aver. | Aver. |
| ID | | Class | | ows | FI Total [| ows HV/1 | Satn | Delay | Service | Qu [Veh. | eue Dist] | Que | Stop Rate | No. of Cycles | Speed |
| | | | veh/h | | veh/h | % | v/c | sec | | veh | m m | | rtate | Oyolos | km/h |
| South | n: Trun | k Esplana | ade (S) | | | | | | | | | | | | |
| 1 | L2 | All MCs | 26 | 8.0 | 26 | 8.0 | * 0.329 | 26.1 | LOS C | 4.1 | 29.1 | 0.67 | 0.58 | 0.67 | 47.0 |
| 2 | T1 | All MCs | 262 | 1.6 | 262 | 1.6 | 0.329 | 7.1 | LOSA | 4.1 | 29.1 | 0.67 | 0.58 | 0.67 | 52.4 |
| 3 | R2 | All MCs | 3 | 0.0 | 3 | 0.0 | 0.077 | 33.3 | LOS C | 0.1 | 0.6 | 1.00 | 0.61 | 1.00 | 35.6 |
| Appro | oach | | 292 | 2.2 | 292 | 2.2 | 0.329 | 9.1 | LOSA | 4.1 | 29.1 | 0.67 | 0.58 | 0.67 | 51.6 |
| East: | Local | Rd (E) | | | | | | | | | | | | | |
| 4 | L2 | All MCs | 1 | 0.0 | 1 | 0.0 | * 0.044 | 27.8 | LOS C | 0.1 | 0.8 | 0.96 | 0.63 | 0.96 | 38.2 |
| 5 | T1 | All MCs | 2 | 0.0 | 2 | 0.0 | 0.044 | 23.2 | LOS C | 0.1 | 0.8 | 0.96 | 0.63 | 0.96 | 36.9 |
| 6 | R2 | All MCs | 2 | 0.0 | 2 | 0.0 | 0.044 | 27.8 | LOS C | 0.1 | 0.8 | 0.96 | 0.63 | 0.96 | 38.1 |
| Appro | oach | | 5 | 0.0 | 5 | 0.0 | 0.044 | 26.0 | LOS C | 0.1 | 8.0 | 0.96 | 0.63 | 0.96 | 37.6 |
| North | : Trunl | k Esplana | ade (N) | | | | | | | | | | | | |
| 7 | L2 | All MCs | 4 | 0.0 | 4 | 0.0 | 0.083 | 13.8 | LOS B | 0.9 | 6.4 | 0.49 | 0.40 | 0.49 | 49.3 |
| 8 | T1 | All MCs | 83 | 1.3 | 83 | 1.3 | 0.083 | 4.9 | LOSA | 0.9 | 6.4 | 0.49 | 0.40 | 0.49 | 55.2 |
| 9 | R2 | All MCs | 6 | 0.0 | 6 | 0.0 | * 0.153 | 33.8 | LOS C | 0.2 | 1.2 | 1.00 | 0.63 | 1.00 | 35.4 |
| Appro | oach | | 94 | 1.1 | 94 | 1.1 | 0.153 | 7.2 | LOSA | 0.9 | 6.4 | 0.52 | 0.42 | 0.52 | 52.9 |
| West | : Colle | ctor (W) | | | | | | | | | | | | | |
| 10 | L2 | All MCs | 2 | 0.0 | 2 | 0.0 | * 0.064 | 26.8 | LOS C | 0.3 | 2.0 | 0.90 | 0.66 | 0.90 | 39.5 |
| 11 | T1 | All MCs | 4 | 0.0 | 4 | 0.0 | 0.064 | 19.2 | LOS B | 0.3 | 2.0 | 0.90 | 0.66 | 0.90 | 38.1 |
| 12 | R2 | All MCs | 7 | 0.0 | 7 | 0.0 | 0.064 | 23.7 | LOS C | 0.3 | 2.0 | 0.90 | 0.66 | 0.90 | 39.4 |
| Appro | oach | | 14 | 0.0 | 14 | 0.0 | 0.064 | 22.8 | LOS C | 0.3 | 2.0 | 0.90 | 0.66 | 0.90 | 39.0 |
| All Ve | hicles | | 404 | 1.8 | 404 | 1.8 | 0.329 | 9.3 | LOSA | 4.1 | 29.1 | 0.65 | 0.55 | 0.65 | 51.1 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

| Pedestrian | Moveme | ent Perf | ormano | e | | | | | | | |
|--------------------|---------------|--------------|----------------|---------------------|-------------------------|-----|--------------|----------------------|----------------|-----------------|----------------|
| Mov ID Crossing | Input Vol. | Dem. Flow | Aver. Delay | Level of Service | AVERAGE QUE [Ped | | Prop. Que | Eff. Stop Rate | Travel Time | Travel Dist. | Aver. Speed |
| | ped/h | ped/h | sec | | ped | m | | | sec | m | m/sec |
| South: Trunk | Esplanad | de (S) | | | | | | | | | |
| P1 Full | 20 | 20 | 16.9 | LOS B | 0.0 | 0.0 | 0.87 | 0.87 | 170.8 | 200.0 | 1.17 |
| East: Local R | d (E) | | | | | | | | | | |
| P2 Full | 20 | 20 | 16.9 | LOS B | 0.0 | 0.0 | 0.87 | 0.87 | 170.8 | 200.0 | 1.17 |

| North: Trunk Esplanade (N) | | | | | | | | | | | |
|----------------------------|-----|-----|------|-------|-----|-----|------|------|-------|-------|------|
| P3 Full | 20 | 20 | 16.9 | LOS B | 0.0 | 0.0 | 0.87 | 0.87 | 170.8 | 200.0 | 1.17 |
| West: Collector (W) | | | | | | | | | | | |
| P4 Full | 50 | 50 | 16.9 | LOS B | 0.1 | 0.1 | 0.87 | 0.87 | 170.8 | 200.0 | 1.17 |
| All Pedestrians | 110 | 110 | 16.9 | LOS B | 0.1 | 0.1 | 0.87 | 0.87 | 170.8 | 200.0 | 1.17 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Masterplan SIDRA Models.sip9

Site: 6 [6_2066 PM (Site Folder: 2066 PM)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA

Site Category: (None)

| Vehicle Movement Performance | | | | | | | | | | | | | | | |
|------------------------------|----------------------------|-----------|--------|--------------|---------------|-------------|----------------|-------|----------|---------------|----------------|-------|--------------|------------------|-------|
| Mov | Turn | Mov | Den | | | rival | Deg. | Aver. | Level of | | Back Of | Prop. | Eff. | Aver. | Aver. |
| ID | | Class | | lows HV 1 | FI Total [| ows HV/1 | Satn | Delay | Service | ્રા [Veh. | ueue Dist] | Que | Stop Rate | No. of Cycles | Speed |
| | | | veh/h | | veh/h | % | v/c | sec | | veh | m | | riato | | km/h |
| South | South: Trunk Esplanade (S) | | | | | | | | | | | | | | |
| 1 | L2 | All MCs | 73 | 1.4 | 73 | 1.4 | * 0.431 | 32.0 | LOS C | 3.5 | 24.7 | 0.90 | 0.75 | 0.90 | 41.4 |
| 2 | T1 | All MCs | 102 | 1.0 | 102 | 1.0 | 0.431 | 10.6 | LOS B | 3.5 | 24.7 | 0.90 | 0.75 | 0.90 | 45.4 |
| 3 | R2 | All MCs | 1 | 0.0 | 1 | 0.0 | 0.026 | 32.6 | LOS C | 0.0 | 0.2 | 0.99 | 0.57 | 0.99 | 35.8 |
| Appro | ach | | 176 | 1.2 | 176 | 1.2 | 0.431 | 19.6 | LOS B | 3.5 | 24.7 | 0.91 | 0.75 | 0.91 | 43.6 |
| East: | Local | Rd (E) | | | | | | | | | | | | | |
| 4 | L2 | All MCs | 2 | 0.0 | 2 | 0.0 | * 0.110 | 28.5 | LOS C | 0.5 | 3.3 | 0.93 | 0.66 | 0.93 | 39.9 |
| 5 | T1 | All MCs | 17 | 0.0 | 17 | 0.0 | 0.110 | 20.7 | LOS C | 0.5 | 3.3 | 0.93 | 0.66 | 0.93 | 38.5 |
| 6 | R2 | All MCs | 2 | 50.0 | 2 | 50.0 | 0.110 | 25.7 | LOS C | 0.5 | 3.3 | 0.93 | 0.66 | 0.93 | 38.7 |
| Appro | ach | | 21 | 5.0 | 21 | 5.0 | 0.110 | 22.0 | LOS C | 0.5 | 3.3 | 0.93 | 0.66 | 0.93 | 38.7 |
| North | Trunk | k Esplana | de (N) | | | | | | | | | | | | |
| 7 | L2 | All MCs | 11 | 0.0 | 11 | 0.0 | 0.166 | 16.9 | LOS B | 1.8 | 13.2 | 0.61 | 0.51 | 0.61 | 47.5 |
| 8 | T1 | All MCs | 131 | 4.0 | 131 | 4.0 | 0.166 | 7.3 | LOSA | 1.8 | 13.2 | 0.61 | 0.51 | 0.61 | 53.0 |
| 9 | R2 | All MCs | 15 | 0.0 | 15 | 0.0 | * 0.357 | 34.6 | LOS C | 0.4 | 2.8 | 1.00 | 0.67 | 1.01 | 35.2 |
| Appro | ach | | 156 | 3.4 | 156 | 3.4 | 0.357 | 10.5 | LOS B | 1.8 | 13.2 | 0.65 | 0.53 | 0.65 | 50.2 |
| West: | Collec | ctor (W) | | | | | | | | | | | | | |
| 10 | L2 | All MCs | 6 | 16.7 | 6 | 16.7 | * 0.118 | 31.4 | LOS C | 0.4 | 2.8 | 0.95 | 0.68 | 0.95 | 37.6 |
| 11 | T1 | All MCs | 2 | 0.0 | 2 | 0.0 | 0.118 | 19.9 | LOS B | 0.4 | 2.8 | 0.95 | 0.68 | 0.95 | 36.7 |
| 12 | R2 | All MCs | 8 | 0.0 | 8 | 0.0 | 0.118 | 24.5 | LOS C | 0.4 | 2.8 | 0.95 | 0.68 | 0.95 | 37.9 |
| Appro | ach | | 17 | 6.3 | 17 | 6.3 | 0.118 | 26.5 | LOS C | 0.4 | 2.8 | 0.95 | 0.68 | 0.95 | 37.7 |
| All Ve | hicles | | 369 | 2.6 | 369 | 2.6 | 0.431 | 16.2 | LOS B | 3.5 | 24.7 | 0.80 | 0.65 | 0.80 | 45.5 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

| Pedestrian Movement Performance | | | | | | | | | | | | |
|---------------------------------|---------------|--------------|----------------|---|-----|-----|--------------|----------------------|----------------|-----------------|----------------|--|
| Mov ID Crossing | Input Vol. | Dem. Flow | Aver. Delay | Level of AVERAGE BACK OF Service QUEUE [Ped Dist] | | | Prop. Que | Eff. Stop Rate | Travel Time | Travel Dist. | Aver. Speed | |
| | ped/h | ped/h | sec | | ped | m | | | sec | m | m/sec | |
| South: Trunk Esplanade (S) | | | | | | | | | | | | |
| P1 Full | 20 | 20 | 16.9 | LOS B | 0.0 | 0.0 | 0.87 | 0.87 | 170.8 | 200.0 | 1.17 | |
| East: Local R | d (E) | | | | | | | | | | | |
| P2 Full | 20 | 20 | 16.9 | LOS B | 0.0 | 0.0 | 0.87 | 0.87 | 170.8 | 200.0 | 1.17 | |

| North: Trunk E | splanade | (N) | | | | | | | | | |
|--------------------|----------|-----|------|-------|-----|-----|------|------|-------|-------|------|
| P3 Full | 20 | 20 | 16.9 | LOS B | 0.0 | 0.0 | 0.87 | 0.87 | 170.8 | 200.0 | 1.17 |
| West: Collecto | or (W) | | | | | | | | | | |
| P4 Full | 50 | 50 | 16.9 | LOS B | 0.1 | 0.1 | 0.87 | 0.87 | 170.8 | 200.0 | 1.17 |
| All Pedestrians | 110 | 110 | 16.9 | LOS B | 0.1 | 0.1 | 0.87 | 0.87 | 170.8 | 200.0 | 1.17 |

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Masterplan SIDRA Models.sip9

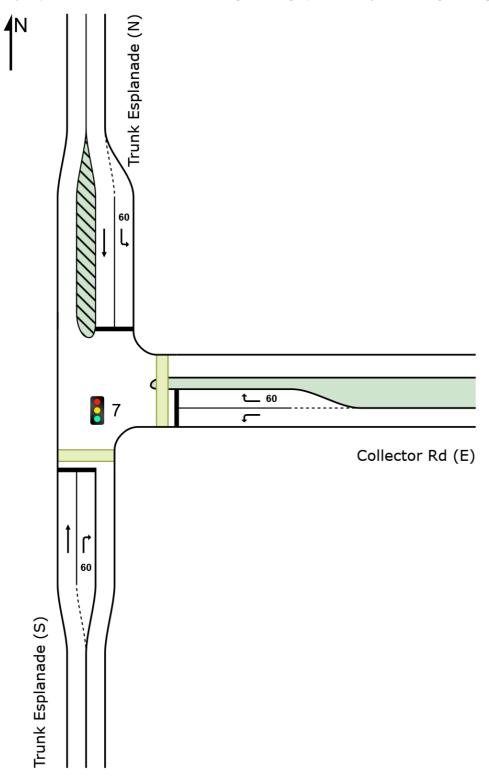
SITE LAYOUT

Site: 7 [7_2066 AM (Site Folder: 2066 AM)]

NA

Site Category: (None) Signals - EQUISAT (Fixed-Time/SCATS) Isolated

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



Site: 7 [7_2066 AM (Site Folder: 2066 AM)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA

Site Category: (None)

| Vehic | cle M | ovemen | t Perfo | rma | nce | | | | | | | | | | |
|--------|---------|------------|---------|------|---------|-------|---------|-------|----------|--------|--------|-------|------|--------|----------|
| Mov | Turn | Mov | Den | nand | Ar | rival | Deg. | Aver. | Level of | 95% Ba | ack Of | Prop. | Eff. | Aver. | Aver. |
| ID | | Class | | lows | | ows | Satn | Delay | Service | Que | | Que | Stop | No. of | Speed |
| | | | | | [Total | | | | | [Veh. | Dist] | | Rate | Cycles | Lean /In |
| South | ı: Trun | k Esplana | veh/h | % | veh/h | % | v/c | sec | | veh | m | _ | _ | | km/h |
| 2 | T1 | All MCs | . , | 2.6 | 164 | 2.6 | 0.122 | 1.1 | LOSA | 0.5 | 3.9 | 0.35 | 0.23 | 0.35 | 58.4 |
| 3 | R2 | All MCs | | 0.0 | 106 | | * 0.143 | 9.9 | LOSA | 0.7 | 4.9 | 0.67 | 0.71 | 0.67 | 46.5 |
| Appro | | 7 111.00 | 271 | | 271 | 1.6 | 0.143 | 4.6 | LOSA | 0.7 | 4.9 | 0.47 | 0.42 | 0.47 | 53.1 |
| East: | Collec | ctor Rd (E | Ξ) | | | | | | | | | | | | |
| 4 | L2 | All MCs | 61 | 1.7 | 61 | 1.7 | 0.111 | 10.5 | LOS B | 0.5 | 3.3 | 0.75 | 0.70 | 0.75 | 45.5 |
| 6 | R2 | All MCs | 11 | 0.0 | 11 | 0.0 | * 0.038 | 13.2 | LOS B | 0.1 | 0.7 | 0.88 | 0.65 | 0.88 | 44.2 |
| Appro | oach | | 72 | 1.5 | 72 | 1.5 | 0.111 | 10.9 | LOS B | 0.5 | 3.3 | 0.77 | 0.69 | 0.77 | 45.3 |
| North | : Trunl | k Esplana | de (N) | | | | | | | | | | | | |
| 7 | L2 | All MCs | 33 | 0.0 | 33 | 0.0 | 0.117 | 14.5 | LOS B | 0.3 | 2.2 | 0.89 | 0.70 | 0.89 | 43.9 |
| 8 | T1 | All MCs | 33 | 0.0 | 33 | 0.0 | * 0.084 | 7.7 | LOS A | 0.3 | 2.0 | 0.84 | 0.60 | 0.84 | 53.3 |
| Appro | oach | | 65 | 0.0 | 65 | 0.0 | 0.117 | 11.1 | LOS B | 0.3 | 2.2 | 0.87 | 0.65 | 0.87 | 48.1 |
| All Ve | hicles | | 407 | 1.3 | 407 | 1.3 | 0.143 | 6.7 | LOSA | 0.7 | 4.9 | 0.59 | 0.50 | 0.59 | 50.7 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

| Pedestrian I | Moveme | ent Perf | ormano | e | | | | | | | |
|--------------------|---------------|--------------|----------------|---------------------|----------------|-------------|--------------|--------------|----------------|-----------------|----------------|
| Mov ID Crossing | Input Vol. | Dem. Flow | Aver. Delay | Level of Service | AVERAGE QUE | UE | Prop. Que | Eff. Stop | Travel Time | Travel Dist. | Aver. Speed |
| | ped/h | ped/h | sec | | [Ped ped | Dist] m | | Rate | sec | m | m/sec |
| South: Trunk E | Esplanad | de (S) | | | | | | | | | |
| P1 Full | 50 | 50 | 4.9 | LOSA | 0.0 | 0.0 | 0.70 | 0.70 | 158.8 | 200.0 | 1.26 |
| East: Collecto | r Rd (E) | | | | | | | | | | |
| P2 Full | 50 | 50 | 4.9 | LOSA | 0.0 | 0.0 | 0.70 | 0.70 | 158.8 | 200.0 | 1.26 |
| All Pedestrians | 100 | 100 | 4.9 | LOSA | 0.0 | 0.0 | 0.70 | 0.70 | 158.8 | 200.0 | 1.26 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Project: P:\P2300 Flagstone City Stages 2-5\Technical\Models\SIDRA\2024_02_16 CA3 Masterplan SIDRAs\P2300.001M 2066 CA3 South Masterplan SIDRA Models.sip9

Site: 7 [7_2066 PM (Site Folder: 2066 PM)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA

Site Category: (None)

| Vehic | cle Mo | ovement | t Perfo | rma | nce | | | | | | | | | | |
|-----------|---------|--------------|------------|-------------|-----|---------------------------|---------------------|-----------------------|---------------------|--------------------------------|-----|--------------|----------------------|---------------------------|------------------------|
| Mov ID | Turn | Mov Class | | lows HV] | | rival lows HV] % | Deg. Satn v/c | Aver. Delay sec | Level of Service | 95% Ba Que [Veh. veh | | Prop. Que | Eff. Stop Rate | Aver. No. of Cycles | Aver. Speed km/h |
| South | : Trun | k Esplana | ade (S) | | | | | | | | | | | | |
| 2 | T1 | All MCs | 67 | 1.6 | 67 | 1.6 | 0.063 | 2.3 | LOSA | 0.3 | 2.3 | 0.49 | 0.37 | 0.49 | 57.8 |
| 3 | R2 | All MCs | 47 | 2.2 | 47 | 2.2 | 0.086 | 11.4 | LOS B | 0.4 | 2.6 | 0.75 | 0.70 | 0.75 | 45.6 |
| Appro | ach | | 115 | 1.8 | 115 | 1.8 | 0.086 | 6.1 | LOSA | 0.4 | 2.6 | 0.59 | 0.51 | 0.59 | 52.0 |
| East: | Collec | tor Rd (E | :) | | | | | | | | | | | | |
| 4 | L2 | All MCs | 124 | 3.4 | 124 | 3.4 | * 0.196 | 9.9 | LOSA | 0.9 | 6.6 | 0.73 | 0.72 | 0.73 | 45.8 |
| 6 | R2 | All MCs | 41 | 0.0 | 41 | 0.0 | 0.088 | 11.3 | LOS B | 0.3 | 2.3 | 0.79 | 0.69 | 0.79 | 45.2 |
| Appro | ach | | 165 | 2.5 | 165 | 2.5 | 0.196 | 10.2 | LOS B | 0.9 | 6.6 | 0.75 | 0.71 | 0.75 | 45.7 |
| North | : Trunl | k Esplana | ide (N) | | | | | | | | | | | | |
| 7 | L2 | All MCs | 17 | 0.0 | 17 | 0.0 | 0.036 | 12.1 | LOS B | 0.1 | 0.9 | 0.78 | 0.67 | 0.78 | 45.2 |
| 8 | T1 | All MCs | 34 | 3.1 | 34 | 3.1 | * 0.088 | 7.7 | LOSA | 0.3 | 2.1 | 0.84 | 0.60 | 0.84 | 53.3 |
| Appro | ach | | 51 | 2.1 | 51 | 2.1 | 0.088 | 9.2 | LOSA | 0.3 | 2.1 | 0.82 | 0.62 | 0.82 | 50.3 |
| All Ve | hicles | | 331 | 2.2 | 331 | 2.2 | 0.196 | 8.6 | LOSA | 0.9 | 6.6 | 0.70 | 0.63 | 0.70 | 48.4 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

* Critical Movement (Signal Timing)

| Mov ID Crossing | Input | Dem. | Aver. | | AVERAGE | | Prop. | Eff. | Travel | Travel | Aver. |
|--------------------|----------|--------|-------|---------|--------------|---------------|-------|--------------|--------|---------|-------|
| ID Crossing | Vol. | Flow | Delay | Service | QUE [Ped | :UE Dist] | Que | Stop Rate | Time | Dist. S | Speed |
| | ped/h | ped/h | sec | | ped | m | | | sec | m | m/sec |
| South: Trunk E | Esplanac | de (S) | | | | | | | | | |
| P1 Full | 50 | 50 | 4.9 | LOSA | 0.0 | 0.0 | 0.70 | 0.70 | 158.8 | 200.0 | 1.26 |
| East: Collecto | r Rd (E) | | | | | | | | | | |
| P2 Full | 50 | 50 | 4.9 | LOSA | 0.0 | 0.0 | 0.70 | 0.70 | 158.8 | 200.0 | 1.26 |
| All Pedestrians | 100 | 100 | 4.9 | LOSA | 0.0 | 0.0 | 0.70 | 0.70 | 158.8 | 200.0 | 1.26 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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SITE LAYOUT

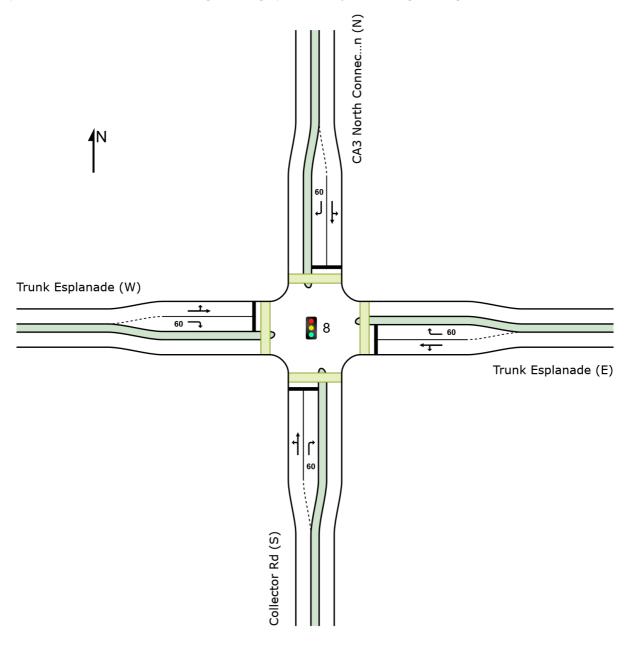
Site: 8 [8_2066 AM (Site Folder: 2066 AM)]

NA

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



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Masterplan SIDRA Models.sip9

Site: 8 [8_2066 AM (Site Folder: 2066 AM)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 70 seconds (Site Optimum Cycle Time - Minimum

Delay)

| Vehic | cle Mo | ovement | Perfo | rma | nce | | | | | | | | | | |
|-----------|---------|--------------|---------|-------|-------|--------------|----------------|----------------|---------------------|------------------------|-------|--------------|----------------------|---------------------------|----------------|
| Mov ID | Turn | Mov Class | | lows | | rival ows | Deg. Satn | Aver. Delay | Level of Service | 95% B Que [Veh. | | Prop. Que | Eff. Stop Rate | Aver. No. of Cycles | Aver. Speed |
| | | | veh/h | | veh/h | % | v/c | sec | | veh | m m | | rtate | Сусісз | km/h |
| South | : Colle | ector Rd (| S) | | | | | | | | | | | | |
| 1 | L2 | All MCs | 1 | 0.0 | 1 | 0.0 | 0.115 | 39.5 | LOS D | 1.0 | 7.2 | 0.89 | 0.65 | 0.89 | 39.8 |
| 2 | T1 | All MCs | 33 | 0.0 | 33 | 0.0 | 0.115 | 28.0 | LOS C | 1.0 | 7.2 | 0.89 | 0.65 | 0.89 | 41.0 |
| 3 | R2 | All MCs | 87 | 1.1 | 87 | 1.1 | * 0.551 | 41.6 | LOS D | 3.1 | 22.0 | 1.00 | 0.78 | 1.04 | 34.8 |
| Appro | ach | | 121 | 8.0 | 121 | 8.0 | 0.551 | 37.9 | LOS D | 3.1 | 22.0 | 0.97 | 0.75 | 1.00 | 36.3 |
| East: | Trunk | Esplanad | le (E) | | | | | | | | | | | | |
| 4 | L2 | All MCs | 128 | 8.0 | 128 | 8.0 | 0.220 | 19.7 | LOS B | 3.8 | 27.0 | 0.65 | 0.68 | 0.65 | 45.4 |
| 5 | T1 | All MCs | 53 | 1.9 | 53 | 1.9 | 0.220 | 10.2 | LOS B | 3.8 | 27.0 | 0.65 | 0.68 | 0.65 | 47.0 |
| 6 | R2 | All MCs | 3 | 0.0 | 3 | 0.0 | * 0.113 | 49.6 | LOS D | 0.1 | 0.9 | 1.00 | 0.61 | 1.00 | 32.3 |
| Appro | ach | | 184 | 1.1 | 184 | 1.1 | 0.220 | 17.5 | LOS B | 3.8 | 27.0 | 0.66 | 0.68 | 0.66 | 45.5 |
| North | CA3 | North Co | nnectio | n (N) | | | | | | | | | | | |
| 7 | L2 | All MCs | 27 | 3.7 | 27 | 3.7 | 0.291 | 42.3 | LOS D | 1.7 | 12.3 | 0.96 | 0.73 | 0.96 | 36.4 |
| 8 | T1 | All MCs | 24 | 0.0 | 24 | 0.0 | * 0.291 | 30.2 | LOS C | 1.7 | 12.3 | 0.96 | 0.73 | 0.96 | 37.5 |
| 9 | R2 | All MCs | 7 | 0.0 | 7 | 0.0 | 0.044 | 38.7 | LOS D | 0.2 | 1.6 | 0.94 | 0.65 | 0.94 | 35.7 |
| Appro | ach | | 58 | 1.7 | 58 | 1.7 | 0.291 | 36.9 | LOS D | 1.7 | 12.3 | 0.96 | 0.72 | 0.96 | 36.8 |
| West: | Trunk | Esplana | de (W) | | | | | | | | | | | | |
| 10 | L2 | All MCs | 80 | 0.0 | 80 | 0.0 | * 0.705 | 40.6 | LOS D | 15.8 | 111.4 | 0.89 | 0.80 | 0.90 | 43.9 |
| 11 | T1 | All MCs | 464 | 1.3 | 464 | 1.3 | 0.705 | 16.7 | LOS B | 15.8 | 111.4 | 0.89 | 0.80 | 0.90 | 45.4 |
| 12 | R2 | All MCs | 1 | 0.0 | 1 | 0.0 | 0.038 | 49.0 | LOS D | 0.0 | 0.3 | 1.00 | 0.57 | 1.00 | 32.6 |
| Appro | ach | | 545 | 1.1 | 545 | 1.1 | 0.705 | 20.2 | LOS C | 15.8 | 111.4 | 0.89 | 0.80 | 0.90 | 44.8 |
| All Ve | hicles | | 908 | 1.1 | 908 | 1.1 | 0.705 | 23.1 | LOS C | 15.8 | 111.4 | 0.86 | 0.76 | 0.87 | 43.0 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

| | | ormanc | <u> </u> | | | | | | | |
|--------|---------------------------|--------------------------------------|--|--|---|--|--|--|--|---|
| nput | Dem. | Aver. | Level of A\ | VERAGE B | ACK OF | Prop. | Eff. | Travel | Travel | Aver. |
| Vol. | Flow | Delay | Service | QUEU | E | Que | Stop | Time | Dist. | Speed |
| | | | | [Ped | Dist] | | Rate | | | |
| ed/h | ped/h | sec | | ped | m | | | sec | m | m/sec |
| Rd (S) | | | | | | | | | | |
| 20 | 20 | 28.4 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 | 182.2 | 200.0 | 1.10 |
| | Vol. ped/h r Rd (S) | vol. Flow ped/h ped/h r Rd (S) | Vol. Flow Delay ped/h ped/h sec r Rd (S) | Vol. Flow Delay Service ped/h sec r Rd (S) | Vol. Flow Delay Service QUEU [Ped ped/h ped/h sec ped r Rd (S) | Vol. Flow Delay Service QUEUE [Ped Dist] ped/h ped/h sec ped m | Vol. Flow Delay Service QUEUE Que [Ped Dist] ped/h ped/h sec ped m | Vol. Flow Delay Service QUEUE Que Stop [Ped Dist] Rate ped/h ped/h sec ped m r Rd (S) | Vol. Flow Delay Service QUEUE Que Stop Time [Ped Dist] Rate ped/h ped/h sec ped m sec r Rd (S) | Vol. Flow Delay Service QUEUE Que Stop Time Dist. [Ped Dist] Rate ped/h ped/h sec ped m sec m Rd (S) |

| P2 Full | 20 | 20 | 28.4 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 | 182.2 | 200.0 | 1.10 |
|--------------------|-------------|----------|------|-------|-----|-----|------|------|-------|-------|------|
| North: CA3 No | orth Connec | ction (N | I) | | | | | | | | |
| P3 Full | 20 | 20 | 28.4 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 | 182.2 | 200.0 | 1.10 |
| West: Trunk E | splanade (\ | N) | | | | | | | | | |
| P4 Full | 20 | 20 | 28.4 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 | 182.2 | 200.0 | 1.10 |
| All Pedestrians | 80 | 80 | 28.4 | LOSC | 0.0 | 0.0 | 0.90 | 0.90 | 182.2 | 200.0 | 1.10 |

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Masterplan SIDRA Models.sip9

Site: 8 [8_2066 PM (Site Folder: 2066 PM)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

NA

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 60 seconds (Site Optimum Cycle Time - Minimum

Delay)

| Vehic | cle Mo | ovement | Perfo | rma | nce | | | | | | | | | | |
|-----------|---------|--------------|--------|-------|-------|--------------|--------------|----------------|---------------------|------|-------------------------|--------------|----------------------|---------------------------|----------------|
| Mov ID | Turn | Mov Class | | ows | | rival ows | Deg. Satn | Aver. Delay | Level of Service | | ack Of eue Dist] | Prop. Que | Eff. Stop Rate | Aver. No. of Cycles | Aver. Speed |
| | | | veh/h | | veh/h | % | v/c | sec | | veh | m m | | rtate | Сусісз | km/h |
| South | : Colle | ector Road | d (S) | | | | | | | | | | | | |
| 1 | L2 | All MCs | 1 | 0.0 | 1 | 0.0 | 0.076 | 35.9 | LOS D | 0.5 | 3.8 | 0.89 | 0.63 | 0.89 | 41.3 |
| 2 | T1 | All MCs | 19 | 5.3 | 19 | 5.3 | 0.076 | 24.3 | LOS C | 0.5 | 3.8 | 0.89 | 0.63 | 0.89 | 42.6 |
| 3 | R2 | All MCs | 53 | 1.9 | 53 | 1.9 | * 0.434 | 37.8 | LOS D | 1.7 | 11.8 | 1.00 | 0.74 | 1.00 | 36.1 |
| Appro | ach | | 73 | 2.7 | 73 | 2.7 | 0.434 | 34.2 | LOS C | 1.7 | 11.8 | 0.97 | 0.71 | 0.97 | 37.6 |
| East: | Trunk | Esplanad | e (E) | | | | | | | | | | | | |
| 4 | L2 | All MCs | 234 | 0.9 | 234 | 0.9 | * 0.622 | 24.6 | LOS C | 10.7 | 75.5 | 0.86 | 0.79 | 0.86 | 44.0 |
| 5 | T1 | All MCs | 209 | 0.0 | 209 | 0.0 | 0.622 | 13.6 | LOS B | 10.7 | 75.5 | 0.86 | 0.79 | 0.86 | 45.5 |
| 6 | R2 | All MCs | 37 | 0.0 | 37 | 0.0 | * 0.398 | 39.2 | LOS D | 1.2 | 8.3 | 1.00 | 0.72 | 1.00 | 35.6 |
| Appro | ach | | 480 | 0.4 | 480 | 0.4 | 0.622 | 20.9 | LOS C | 10.7 | 75.5 | 0.87 | 0.79 | 0.87 | 43.8 |
| North | : CA3 | North Co | nectio | n (N) | | | | | | | | | | | |
| 7 | L2 | All MCs | 16 | 0.0 | 16 | 0.0 | 0.199 | 38.3 | LOS D | 8.0 | 5.4 | 0.96 | 0.70 | 0.96 | 37.6 |
| 8 | T1 | All MCs | 10 | 0.0 | 10 | 0.0 | * 0.199 | 26.2 | LOS C | 8.0 | 5.4 | 0.96 | 0.70 | 0.96 | 38.6 |
| 9 | R2 | All MCs | 8 | 12.5 | 8 | 12.5 | 0.070 | 36.4 | LOS D | 0.2 | 1.9 | 0.96 | 0.66 | 0.96 | 36.4 |
| Appro | ach | | 34 | 2.9 | 34 | 2.9 | 0.199 | 34.3 | LOS C | 8.0 | 5.4 | 0.96 | 0.69 | 0.96 | 37.6 |
| West: | Trunk | Esplana | de (W) | | | | | | | | | | | | |
| 10 | L2 | All MCs | 14 | 0.0 | 14 | 0.0 | 0.176 | 35.2 | LOS D | 2.4 | 16.8 | 0.74 | 0.61 | 0.74 | 45.8 |
| 11 | T1 | All MCs | 99 | 1.0 | 99 | 1.0 | 0.176 | 13.5 | LOS B | 2.4 | 16.8 | 0.74 | 0.61 | 0.74 | 47.4 |
| 12 | R2 | All MCs | 1 | 0.0 | 1 | 0.0 | 0.011 | 36.7 | LOS D | 0.0 | 0.2 | 0.97 | 0.58 | 0.97 | 36.5 |
| Appro | ach | | 114 | 0.9 | 114 | 0.9 | 0.176 | 16.3 | LOS B | 2.4 | 16.8 | 0.75 | 0.61 | 0.75 | 47.1 |
| All Ve | hicles | | 701 | 0.9 | 701 | 0.9 | 0.622 | 22.2 | LOS C | 10.7 | 75.5 | 0.87 | 0.75 | 0.87 | 43.2 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

| Mov | Input | Dem. | Aver. | Level of A | AVERAGE | BACK OF | Prop. | Eff. | Travel | Travel | Aver. |
|----------------|----------|-------|-------|------------|--------------|---------------|-------|--------------|--------|---------|-------|
| ID Crossing | Vol. | Flow | Delay | Service | QUE [Ped | EUE Dist] | Que | Stop Rate | Time | Dist. S | Speed |
| | ped/h | ped/h | sec | | ped | m - | | | sec | m i | m/sec |
| South: Collect | or Road | (S) | | | | | | | | | |
| P1 Full | 20 | 20 | 23.4 | LOS C | 0.0 | 0.0 | 0.88 | 0.88 | 177.3 | 200.0 | 1.13 |
| East: Trunk Es | splanade | (E) | | | | | | | | | |

| P2 Full | 20 | 20 | 23.4 | LOS C | 0.0 | 0.0 | 0.88 | 0.88 | 177.3 | 200.0 | 1.13 |
|--------------------|-------------|----------|------|-------|-----|-----|------|------|-------|-------|------|
| North: CA3 No | rth Connec | ction (N | I) | | | | | | | | |
| P3 Full | 20 | 20 | 23.4 | LOS C | 0.0 | 0.0 | 0.88 | 0.88 | 177.3 | 200.0 | 1.13 |
| West: Trunk E | splanade (\ | N) | | | | | | | | | |
| P4 Full | 20 | 20 | 23.4 | LOS C | 0.0 | 0.0 | 0.88 | 0.88 | 177.3 | 200.0 | 1.13 |
| All Pedestrians | 80 | 80 | 23.4 | LOS C | 0.0 | 0.0 | 0.88 | 0.88 | 177.3 | 200.0 | 1.13 |

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Masterplan SIDRA Models.sip9

SITE LAYOUT

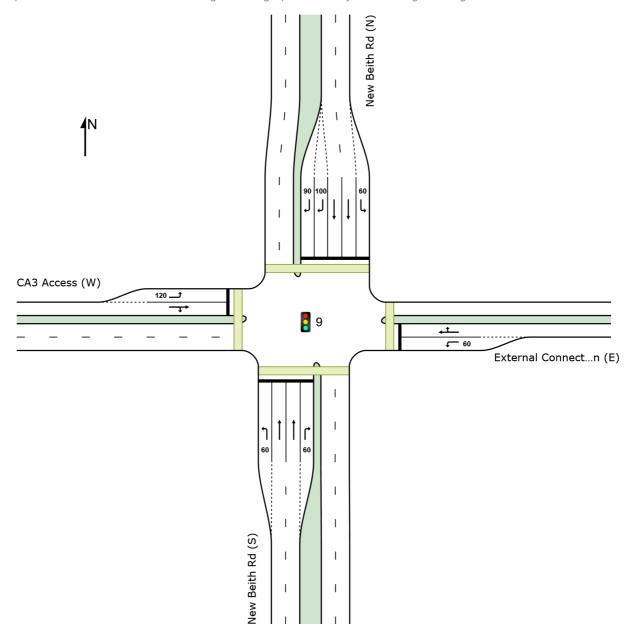
Site: 9 [9_2066 AM (Site Folder: 2066 AM)]

New Beith Road / CA3 Central Access

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated

Layout pictures are schematic functional drawings reflecting input data. They are not design drawings.



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Masterplan SIDRA Models.sip9

Site: 9 [9_2066 AM (Site Folder: 2066 AM)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

New Beith Road / CA3 Central Access

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 110 seconds (Site Practical Cycle Time)

| Vehi | cle Mo | ovement | Perfo | rma | nce | | | | | | | | | | |
|-----------|--------|--------------|------------|-------------|------|---------------------------|---------------------|-----------------------|---------------------|------|------------------------------|--------------|----------------------|---------------------------|------------------------|
| Mov ID | Turn | Mov Class | | lows HV] | | rival lows HV] % | Deg. Satn v/c | Aver. Delay sec | Level of Service | | ack Of eue Dist] m | Prop. Que | Eff. Stop Rate | Aver. No. of Cycles | Aver. Speed km/h |
| South | ı: New | Beith Rd | (S) | | | | | | | | | | | | |
| 1 | L2 | All MCs | 48 | 0.0 | 48 | 0.0 | 0.049 | 34.8 | LOS C | 1.2 | 8.5 | 0.51 | 0.70 | 0.51 | 53.6 |
| 2 | T1 | All MCs | 1751 | 2.6 | 1751 | 2.6 | * 0.883 | 47.2 | LOS D | 46.9 | 335.2 | 0.96 | 0.96 | 1.06 | 57.3 |
| 3 | R2 | All MCs | 30 | 13.3 | 30 | 13.3 | 0.072 | 56.3 | LOS E | 1.2 | 9.6 | 0.80 | 0.72 | 0.80 | 43.4 |
| Appro | oach | | 1829 | 2.7 | 1829 | 2.7 | 0.883 | 47.0 | LOS D | 46.9 | 335.2 | 0.94 | 0.95 | 1.04 | 50.5 |
| East: | Exterr | nal Conne | ction (E | Ξ) | | | | | | | | | | | |
| 4 | L2 | All MCs | 64 | 0.0 | 64 | 0.0 | 0.111 | 34.6 | LOS C | 2.4 | 17.1 | 0.75 | 0.73 | 0.75 | 45.6 |
| 5 | T1 | All MCs | 3 | 0.0 | 3 | 0.0 | * 0.406 | 55.5 | LOS E | 3.0 | 20.9 | 0.99 | 0.75 | 0.99 | 30.2 |
| 6 | R2 | All MCs | 52 | 0.0 | 52 | 0.0 | 0.406 | 61.1 | LOS E | 3.0 | 20.9 | 0.99 | 0.75 | 0.99 | 38.3 |
| Appro | oach | | 119 | 0.0 | 119 | 0.0 | 0.406 | 46.7 | LOS D | 3.0 | 20.9 | 0.86 | 0.74 | 0.86 | 41.6 |
| North | : New | Beith Rd | (N) | | | | | | | | | | | | |
| 7 | L2 | All MCs | 12 | 0.0 | 12 | 0.0 | 0.017 | 36.0 | LOS D | 0.2 | 1.7 | 0.65 | 0.67 | 0.65 | 55.4 |
| 8 | T1 | All MCs | 1164 | 5.5 | 1164 | 5.5 | 0.876 | 56.1 | LOS E | 33.4 | 244.6 | 1.00 | 0.99 | 1.15 | 51.7 |
| 9 | R2 | All MCs | 136 | 0.7 | 136 | 0.7 | * 0.675 | 70.2 | LOS E | 3.9 | 27.6 | 1.00 | 0.81 | 1.14 | 36.8 |
| Appro | oach | | 1312 | 5.0 | 1312 | 5.0 | 0.876 | 57.3 | LOS E | 33.4 | 244.6 | 1.00 | 0.97 | 1.14 | 46.2 |
| West | CA3 / | Access (V | V) | | | | | | | | | | | | |
| 10 | L2 | All MCs | 418 | 0.5 | 418 | 0.5 | 0.887 | 59.5 | LOS E | 25.1 | 176.7 | 1.00 | 0.99 | 1.23 | 38.5 |
| 11 | T1 | All MCs | 3 | 0.0 | 3 | 0.0 | * 0.489 | 47.9 | LOS D | 6.5 | 47.4 | 0.97 | 0.79 | 0.97 | 32.1 |
| 12 | R2 | All MCs | 125 | 4.8 | 125 | 4.8 | 0.489 | 53.6 | LOS D | 6.5 | 47.4 | 0.97 | 0.79 | 0.97 | 38.8 |
| Appro | ach | | 546 | 1.5 | 546 | 1.5 | 0.887 | 58.1 | LOS E | 25.1 | 176.7 | 0.99 | 0.94 | 1.17 | 38.5 |
| All Ve | hicles | | 3806 | 3.2 | 3806 | 3.2 | 0.887 | 52.1 | LOS D | 46.9 | 335.2 | 0.97 | 0.95 | 1.09 | 46.9 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

| Pedestrian Movement Performance | | | | | | | | | | | | |
|---------------------------------|---------------|--------------|----------------|---------------------|-------------------------|--------------------------|--------------|----------------------|----------------|-----------------|----------------|--|
| Mov ID Crossing | Input Vol. | Dem. Flow | Aver. Delay | Level of Service | AVERAGE QUE [Ped | BACK OF EUE Dist] | Prop. Que | Eff. Stop Rate | Travel Time | Travel Dist. | Aver. Speed | |
| | ped/h | ped/h | sec | | ped | m | | | sec | m | m/sec | |
| South: New Beith Rd (S) | | | | | | | | | | | | |
| P1 Full | 5 | 5 | 48.2 | LOS E | 0.0 | 0.0 | 0.94 | 0.94 | 202.1 | 200.0 | 0.99 | |
| East: External Connection (E) | | | | | | | | | | | | |
| P2 Full | 30 | 30 | 48.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | 202.1 | 200.0 | 0.99 | |

| North: New Beith Rd (N) | | | | | | | | | | | |
|-------------------------|----------|-----|------|-------|-----|-----|------|------|-------|-------|------|
| P3 Full | 20 | 20 | 48.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | 202.1 | 200.0 | 0.99 |
| West: CA3 Acc | cess (W) | | | | | | | | | | |
| P4 Full | 50 | 50 | 48.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | 202.2 | 200.0 | 0.99 |
| All Pedestrians | 105 | 105 | 48.3 | LOSE | 0.1 | 0.1 | 0.94 | 0.94 | 202.1 | 200.0 | 0.99 |

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Project: P:\P2300 Flagstone City Stages 2-5\Technical\Models\SIDRA\2024_02_16 CA3 Masterplan SIDRAs\P2300.001M 2066 CA3 South
Masterplan SIDRA Models.sip9

Site: 9 [9_2066 PM (Site Folder: 2066 PM)]

Output produced by SIDRA INTERSECTION Version: 9.1.6.228

New Beith Road / CA3 Central Access

Site Category: (None)

Signals - EQUISAT (Fixed-Time/SCATS) Isolated Cycle Time = 120 seconds (Site Practical Cycle Time)

| Vehicle Movement Performance | | | | | | | | | | | | | | | |
|------------------------------|--------|-----------|----------|------|---------------|-----------|----------------|-------|----------|--------------|---------------|-------|--------------|------------------|-------|
| Mov | Turn | Mov | | nand | | rival | Deg. | Aver. | Level of | | ack Of | Prop. | Eff. | Aver. | Aver. |
| ID | | Class | | OWS | FI Total] | OWS | Satn | Delay | Service | Qu [Veh. | eue Dist] | Que | Stop Rate | No. of Cycles | Speed |
| | | | veh/h | | veh/h | пv ј % | v/c | sec | | veh | m m | | Nate | Cycles | km/h |
| South | ı: New | Beith Rd | (S) | | | | | | | | | | | | |
| 1 | L2 | All MCs | 121 | 8.0 | 121 | 8.0 | 0.179 | 51.8 | LOS D | 4.7 | 33.5 | 0.72 | 0.76 | 0.72 | 46.4 |
| 2 | T1 | All MCs | 1150 | 2.3 | 1150 | 2.3 | 0.875 | 65.2 | LOS E | 36.2 | 258.2 | 1.00 | 0.98 | 1.13 | 50.8 |
| 3 | R2 | All MCs | 55 | 0.0 | 55 | 0.0 | * 0.711 | 93.9 | LOS F | 3.5 | 24.6 | 1.00 | 0.81 | 1.19 | 33.6 |
| Appro | oach | | 1326 | 2.0 | 1326 | 2.0 | 0.875 | 65.1 | LOS E | 36.2 | 258.2 | 0.97 | 0.96 | 1.10 | 43.5 |
| East: | Exterr | nal Conne | ction (E | Ξ) | | | | | | | | | | | |
| 4 | L2 | All MCs | 30 | 0.0 | 30 | 0.0 | 0.162 | 60.2 | LOS E | 1.7 | 11.6 | 0.95 | 0.72 | 0.95 | 36.9 |
| 5 | T1 | All MCs | 3 | 0.0 | 3 | 0.0 | 0.241 | 60.0 | LOS E | 1.7 | 12.2 | 0.98 | 0.72 | 0.98 | 29.2 |
| 6 | R2 | All MCs | 27 | 0.0 | 27 | 0.0 | 0.241 | 65.6 | LOS E | 1.7 | 12.2 | 0.98 | 0.72 | 0.98 | 37.1 |
| Appro | oach | | 60 | 0.0 | 60 | 0.0 | 0.241 | 62.6 | LOS E | 1.7 | 12.2 | 0.97 | 0.72 | 0.97 | 36.6 |
| North | : New | Beith Rd | (N) | | | | | | | | | | | | |
| 7 | L2 | All MCs | 54 | 0.0 | 54 | 0.0 | * 0.048 | 27.6 | LOS C | 8.0 | 5.4 | 0.43 | 0.69 | 0.43 | 59.6 |
| 8 | T1 | All MCs | 1922 | 1.2 | 1922 | 1.2 | * 0.896 | 44.1 | LOS D | 55.7 | 393.7 | 0.95 | 0.94 | 1.03 | 57.7 |
| 9 | R2 | All MCs | 358 | 0.3 | 358 | 0.3 | 0.386 | 52.3 | LOS D | 8.7 | 61.3 | 0.88 | 0.80 | 0.88 | 42.8 |
| Appro | oach | | 2334 | 1.0 | 2334 | 1.0 | 0.896 | 45.0 | LOS D | 55.7 | 393.7 | 0.93 | 0.91 | 1.00 | 50.3 |
| West | CA3 / | Access (V | V) | | | | | | | | | | | | |
| 10 | L2 | All MCs | 125 | 1.6 | 125 | 1.6 | 0.157 | 27.5 | LOS C | 4.4 | 31.1 | 0.64 | 0.73 | 0.64 | 50.1 |
| 11 | T1 | All MCs | 4 | 0.0 | 4 | 0.0 | * 0.181 | 50.6 | LOS D | 2.4 | 16.7 | 0.92 | 0.73 | 0.92 | 31.5 |
| 12 | R2 | All MCs | 41 | 0.0 | 41 | 0.0 | 0.181 | 56.2 | LOS E | 2.4 | 16.7 | 0.92 | 0.73 | 0.92 | 38.3 |
| Appro | oach | | 170 | 1.2 | 170 | 1.2 | 0.181 | 34.9 | LOS C | 4.4 | 31.1 | 0.72 | 0.73 | 0.72 | 46.3 |
| All Ve | hicles | | 3890 | 1.4 | 3890 | 1.4 | 0.896 | 51.7 | LOS D | 55.7 | 393.7 | 0.93 | 0.92 | 1.02 | 47.4 |

Site Level of Service (LOS) Method: Delay (SIDRA). Site LOS Method is specified in the Parameter Settings dialog (Options tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Delay Model: SIDRA Standard (Control Delay: Geometric Delay is included).

Queue Model: SIDRA queue estimation methods are used for Back of Queue and Queue at Start of Green.

Gap-Acceptance Capacity Formula: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Arrival Flows used in performance calculations are adjusted to include any Initial Queued Demand and Upstream Capacity Constraint effects.

| Pedestrian Movement Performance | | | | | | | | | | | | |
|---------------------------------|---------------|--------------|----------------|---------------------|-------------------------|-----|--------------|----------------------|----------------|-------------------|----------------|--|
| Mov ID Crossing | Input Vol. | Dem. Flow | Aver. Delay | Level of Service | AVERAGE QUE [Ped | | Prop. Que | Eff. Stop Rate | Travel Time | Travel Dist. S | Aver. Speed | |
| | ped/h | ped/h | sec | | ped | m - | | | sec | m | m/sec | |
| South: New E | eith Rd (| S) | | | | | | | | | | |
| P1 Full | 5 | 5 | 53.2 | LOS E | 0.0 | 0.0 | 0.94 | 0.94 | 207.1 | 200.0 | 0.97 | |
| East: Externa | l Connec | tion (E) | | | | | | | | | | |
| P2 Full | 30 | 30 | 53.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | 207.1 | 200.0 | 0.97 | |

| North: New Beith Rd (N) | | | | | | | | | | | |
|-------------------------|----------|-----|------|-------|-----|-----|------|------|-------|-------|------|
| P3 Full | 20 | 20 | 53.2 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | 207.1 | 200.0 | 0.97 |
| West: CA3 Acc | cess (W) | | | | | | | | | | |
| P4 Full | 50 | 50 | 53.3 | LOS E | 0.2 | 0.2 | 0.94 | 0.94 | 207.2 | 200.0 | 0.97 |
| All Pedestrians | 105 | 105 | 53.3 | LOSE | 0.2 | 0.2 | 0.94 | 0.94 | 207.1 | 200.0 | 0.97 |

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Project: P:\P2300 Flagstone City Stages 2-5\Technical\Models\SIDRA\2024_02_16 CA3 Masterplan SIDRAs\P2300.001M 2066 CA3 South
Masterplan SIDRA Models.sip9



Appendix D: Active Transport Provisions



